
***FINAL TECHNICAL APPENDICES
VOLUME III***

***NORTH VINEYARD STATION
SPECIFIC PLAN
ENVIRONMENTAL IMPACT REPORT***

- ***SUPPLEMENTAL TRANSPORTATION ANALYSIS***
- ***TRANSPORTATION ANALYSIS***

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*Sacramento County
Department of Environmental
Review and Assessment*

FEBRUARY 1998



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VOLUME III
FINAL TECHNICAL APPENDICES
NORTH VINEYARD STATION SPECIFIC PLAN

Technical Appendices Volume III for the North Vineyard Station Specific Plan EIR contains the following technical studies: Final Transportation Analysis and the Final Supplemental Transportation Analysis.

These technical studies were prepared by consultants with expertise in the appropriate area. The Law Offices of George E. Phillips served as the coordinator for this work. All phases of the technical study work have been monitored by the Sacramento County Planning Department. Scoping of all technical issues and report review has been conducted by county staff of the Planning Department, the Transportation Division and the Department of Environmental Review and Assessment.

Technical issues were identified during the early scoping process. Administrative drafts of each technical study was prepared by consultants and reviewed by County staff. Revisions were made as needed. The technical studies contained herein are a result of this process.

Please note: Participating property owners have changed since the technical studies were originally conducted. These studies contain technical data for some properties that are no longer considered participating properties however, the technical data is not affected by these changes. The Specific Plan Proponents for the project now include: Winncrest Homes, US Home, Florin Investors, and Cal Maple Development. The Saca Properties, Morvai and East Bradshaw Gerber Associates are no longer participating owners. For a map of the current participating properties see Plate PRF-A in the Preface of the Final EIR.

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- Appendix A-2** **Final Report-Transportation Analysis *October 2, 1996***
with
Revised Pages 31/32 (*February 6, 1998*)

APPENDIX A-1

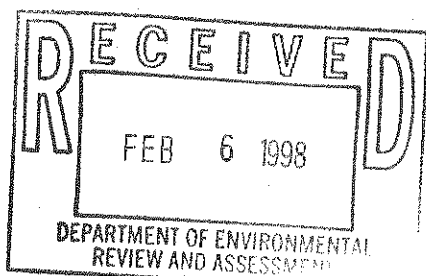
NVSSP FEIR: Final Technical Appendices Vol. III

FINAL REPORT

SUPPLEMENTAL TRANSPORTATION ANALYSIS FOR THE NORTH VINEYARD STATION SPECIFIC PLAN: Alternatives to Relocate Commercial Core

**Prepared for:
North Vineyard Station Property Owners**

February 6, 1998



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Final Report

**SUPPLEMENTAL TRANSPORTATION ANALYSIS
FOR THE
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I. INTRODUCTION

BACKGROUND

The *Transportation Analysis for the North Vineyard Station Specific Plan* was originally prepared by Fehr & Peers Associates on October 2, 1996. That document presented the traffic impacts of the preferred project and presented a trip generation comparison of two reduced-density alternatives. Since that time, two additional alternatives have been prepared, both of which involve the transfer of uses from the original commercial core area to other locations within the specific plan area. The two new alternatives are known as the Alternate Core and Dispersed Core alternatives. Detailed descriptions of these alternatives are presented below.

STUDY PURPOSE

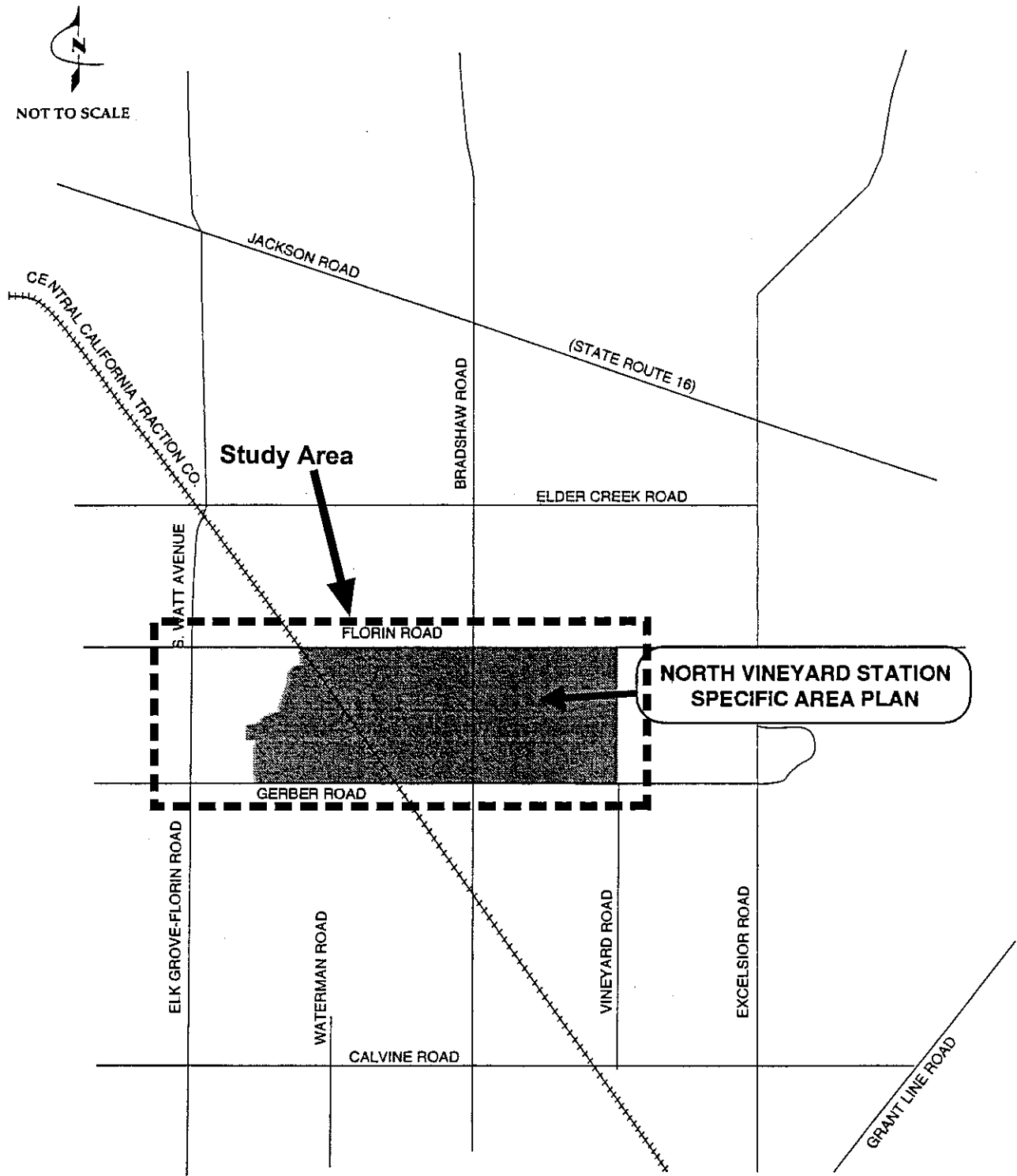
The purpose of this study is to analyze the impacts of the two new North Vineyard Station Specific Plan land use alternatives on the adjacent transportation system. This study assesses the short- and long-term impacts of the alternatives on the local circulation network and describes the improvements required to mitigate operational deficiencies of the transportation system. The results of this alternatives analysis are also compared to the impacts and mitigation measures of the preferred plan.

DESCRIPTION OF PROJECT ALTERNATIVES

The North Vineyard Station Specific Plan area is located in southeastern Sacramento County and is bounded by Florin Road to the north, Elder Creek to the west, the planned Vineyard Road extension to the east, and Gerber Road to the south. Figure 1 shows the location of the plan area and the adjacent roadway network.

The plan area is a 1,595-acre residential development with a total of 6,339 dwelling units and approximately 38 acres of retail and office uses. Public facilities include neighborhood parks, a community park, a library, and elementary schools. Under the preferred project, the commercial core is located near the intersection of Waterman Road and Gerber Road. The two alternatives to the preferred project analyzed in this document are described below:

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<p>Figure 1</p> <p>812-18-01</p>	<p>STUDY AREA</p>	
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Alternate Core Alternative - The commercial core is completely relocated from the Waterman Road/Gerber Road intersection to the Bradshaw Road/Florin Road intersection. This alternative involves the relocation of all of the retail commercial and business-professional uses to the southwest and southeast quadrants of the Bradshaw Road/Florin Road intersection, as well as the transfer of approximately 300 multi-family dwelling units to Florin Road west of Bradshaw Road. Under this alternative, the amount of business-professional uses is approximately 50 percent of the amount planned under the preferred project. The relocated uses will be replaced by single-family residential dwelling units. The total number of dwelling units (6,339) is the same as the preferred project.

Dispersed Core Alternative - This alternative involves the relocation of 13.4 acres of retail commercial uses from the Waterman Road/Gerber Road intersection to the southwest quadrant of the Bradshaw Road/Florin Road intersection. The relocated retail commercial uses will be replaced by single-family residential dwelling units. All other land uses are essentially similar to the preferred project including the total number of dwelling units (6,339).

The proposed on-site street system for the preferred project and all alternatives consists of a network of two-lane collector and local streets interconnected by several major collector roadways that will provide access to the existing and future arterial roadways: Florin Road, Gerber Road, Bradshaw Road and the extensions of Vineyard Road and Waterman Road. As part of the, improvements to the existing transit, bicycle, and pedestrian facilities, as well as new facilities, will be implemented to encourage the use of alternative travel modes.

Both alternatives including the elimination of a collector roadway that was proposed under the preferred project. This roadway extended from Waterman Road (south of the proposed new Central Traction Railroad crossing) to Gerber Road (approximately halfway between Waterman Road and Bradshaw Road) and included a crossing of Gerber Creek.

STUDY AREA

Since the proposed alternatives result in a negligible change in net land use intensity, the study area for the alternatives analysis is focused on transportation facilities immediately adjacent to the project site. In consultation with Sacramento County Public Works staff, the following intersections selected for analysis:

1. Elk-Grove Florin Road/Florin Road
2. Florin Road/Waterman Road
3. Florin Road/Bradshaw Road
4. Florin Road/Vineyard Road
5. Elk-Grove Florin Road/Gerber Road
6. Gerber Road/Waterman Road

7. Gerber Road/Bradshaw Road
8. Gerber Road/Vineyard Road

These intersections are shown on Figure 1. In addition, the roadway segments immediately adjacent to the study intersections were analyzed according to Sacramento County traffic impact analysis guidelines.

IMPACT EVALUATION CRITERIA

Criteria for evaluating impacts were established for roadways (including segments and intersections), transit facilities, and bicycle/pedestrian facilities as documented in the original transportation analysis for the North Vineyard Station Specific Plan. These criteria were maintained for this alternatives analysis and are described below.

Arterial Roadway Segments

For purposes of this study, roadway segments were analyzed by comparing average daily traffic volumes to capacity thresholds that were developed for the Sacramento County General Plan Update. These thresholds are used as guidelines by the County to master plan the transportation network by projecting the need for new or upgraded facilities.

The effect of substandard roadway cross-sections was also used to identify deficiencies. Some of the study roadways, including Elk Grove-Florin Road, Jackson Road, and Bradshaw Road, include 12-foot travel lanes and minimum 6-foot usable shoulders. Other roadways such as Gerber Road and Excelsior Road provide 10- or 11-foot travel lanes and usable shoulders that are less than two-feet wide.

Operational Deficiencies

According to the Sacramento County General Plan, the streets within the study area are ultimately planned to be medium-access arterial roadways, with two to four intersections per mile, a limited number of driveways, and a speed limit between 35 and 55 miles per hour. For this type of facility, the daily roadway capacity is approximately 18,000, 36,000, and 54,000 VPD for streets with two lanes, four lanes and six lanes, respectively. These capacities were applied to all facilities under existing and cumulative conditions.

An alternative results in an operational deficiency if the addition of project-generated traffic causes the demand on a facility to exceed its ultimate capacity, thus requiring additional travel lanes beyond those delineated in the Transportation Plan of the *General Plan Circulation Element* dated December 15, 1993.

Physical Deficiencies

As noted above, several study roadways include substandard cross-sections. According to standards set forth in *A Policy on Geometric Design of Highways and Streets* (1994) published by the American Association of State Highway and Transportation Officials (AASHTO), the minimum lane width and shoulder width is 12 feet and 6 feet, respectively, for rural arterials serving more than 400 VPD. Thus, a physical deficiency is defined to occur when a substandard roadway segment serves more than 400 VPD.

Signalized Intersections

Level of service (LOS) is a term which qualitatively describes operating conditions for intersections. There are six levels of service, A through F, which represent driving conditions from best to worst, respectively. In general, LOS A represents free-flow conditions with no congestion and LOS F represents severe congestion or delay under stop-and-go conditions.

The County of Sacramento has defined the level of service standard for urban area roadways to be LOS E (i.e., LOS F is considered unacceptable). For intersections of rural collector roadways, the minimum acceptable LOS is D. These standards have been set forth in Policy CI-22 in the County's *General Plan Circulation Element*. As set forth in Policy CI-23, if implementation of a project results in a level of service worse than those specified in Policy CI-22, traffic impacts must be mitigated by enhancing the capacity of the roadway and transit system or reducing the demand generated by an alternative.

Analysis of signalized intersections was completed using the methods described in *Interim Materials on Highway Capacity* (Circular No. 212, Transportation Research Board, January 1980) and the interim *Traffic Impact Guidelines* (County of Sacramento Public Works, July 30, 1996). The characteristics of traffic operations for each LOS with respect to signalized intersections are shown in Table 1. Corresponding to each LOS is a volume-to-capacity (V/C) ratio. This is the ratio of the existing or projected volume to the theoretical capacity of the intersection. An intersection is defined to be "at capacity" at LOS E when the V/C ratio is 1.00.

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Table 1 Signalized Intersection Level of Service Descriptions		
<i>Level of Service</i>	<i>Interpretation</i>	<i>Volume-to-Capacity Ratio</i>
A	Uncongested operations; all queues clear in a single cycle.	Less than 0.61
B	Very light congestion; an occasional phase is fully utilized.	0.61-0.70
C	Light congestion; occasional queues on approaches.	0.71-0.80
D	Significant congestion on critical approaches, but intersection is functional. Cars required to wait through more than one cycle during short peaks. No longstanding queues formed.	0.81-0.90
E	Severe congestion with some longstanding queues on critical approaches. Traffic queue may block nearby intersection(s) upstream of critical approach(es).	0.91-1.00
F	Total breakdown, stop-and-go operation.	Greater than 1.00

Source: *Interim Materials on Highway Capacity* (Circular 212, Transportation Research Board, 1980).

A land use alternative results in an operational deficiency when the addition of traffic from the plan area changes the intersection LOS from E or better to LOS F. For intersections operating at LOS F without a specific plan alternative, an operational deficiency is identified if the addition of plan area traffic increases the V/C ratio by 0.05 or more.

Unsignalized Intersections

For those intersections that are controlled by stop signs, two methodologies were used to analyze operating conditions. For those intersections with stop sign control on the minor street only (two-way stops), the methods described in Chapter 10 of the *Highway Capacity Manual* (Special Report 209, Transportation Research Board, 1985) were applied. Level of service is calculated for each movement based on the reserve capacity available. Reserve capacity is a function of demand and the critical gaps in traffic on the major street. The criteria for LOS at two-way stop controlled intersections is shown in Table 2.

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Table 2 Intersection Level of Service Definitions for Two-Way Stop Controlled Intersections		
<i>Level of Service</i>	<i>Interpretation</i>	<i>Reserve Capacity (vehicles)</i>
A	Little or no delay.	≥ 400
B	Short traffic delays.	300 to 399
C	Average traffic delays.	200 to 299
D	Long traffic delays.	100 to 199
E	Very long traffic delays.	0 to 99
F	Stop-and-go conditions.	< 0

Source: *Highway Capacity Manual* (Special Report 209, Transportation Research Board, 1985).

At intersections with all-way stop control (i.e., all traffic stops), procedures described in Interim Materials on Unsignalized Intersection Capacity (Circular No. 373, Transportation Research Board, 1991) were used. This method determines an overall LOS based on the estimated delay for traffic on each of the approaches. The ranges of delay for each level of service is presented in Table 3.

Table 3 Intersection Level of Service Definitions for All-Way Stop Controlled Intersections		
<i>Level of Service</i>	<i>Interpretation</i>	<i>Average Vehicle Delay (in seconds)</i>
A	Little or no delay.	≤ 5
B	Short traffic delays.	>5 and ≤ 10
C	Average traffic delays.	>10 and ≤ 20
D	Long traffic delays.	>20 and ≤ 30
E	Very long traffic delays.	>30 and ≤ 45
F	Stop-and-go conditions.	>45

Source: *Interim Materials on Unsignalized Intersection Capacity* (Circular No. 373, Transportation Research Board, 1991).

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An alternative results in an operational deficiency when the addition of traffic from the plan area changes the LOS from E or better to LOS F. For those intersections that are affected, an analysis of peak hour volumes was conducted to determine if a traffic signal would be warranted.

Since the proposed alternatives generally involve the relocation of land uses within the plan area and do not alter the overall character of the plan, the change in impacts to transit system and bicycle/pedestrian systems is considered negligible. Thus, these modes were not evaluated in this supplemental analysis.

REPORT ORGANIZATION

Chapter II presents the operating characteristics of the transportation system under existing conditions with and without the preferred project, the Alternate Core and the Dispersed Core Alternatives. The impact of the preferred project and the alternatives under cumulative conditions is presented in Chapter III, and Chapter IV presents the projected traffic volumes on internal roadways for these scenarios. Chapter V summarizes the difference in mitigation measures between the various project alternatives.

II. IMPACTS UNDER EXISTING CONDITIONS

The existing condition of the transportation system within the study area was described in detail in Chapter II of the original transportation analysis prepared for the plan area in October 1996. Chapter III of that report described the traffic impacts of the preferred project and the improvements required to mitigate impacts. This chapter describes the effect that trips generated by the proposed alternatives would have on the operation of the existing local transportation system and its users. The impacts and mitigation measures for the preferred project and the Alternate Core and Dispersed Core alternatives are presented in this chapter.

VEHICLE TRIPS FROM PROJECT ALTERNATIVES

The three-step process of trip generation, distribution, and assignment used in the original transportation analysis for the North Vineyard Station Specific Plan was employed for this study. This process is described below.

Project Trip Generation

Traffic from the North Vineyard Station Specific Plan area was estimated using the following process:

- Estimate vehicle trip ends for various land uses using standard trip generation rates;
- Adjust the total number of trips to account for internal and pass-by trips;
- Estimate transit utilization; and
- Determine net project trip generation.

Each step is explained in detail below.

Estimation Of Vehicle Trip Ends

The number of trips generated for each land use was estimated by applying trip rates from *Trip Generation*, Institute of Transportation Engineers, 5th Edition, 1991. These rates were applied to the proposed land uses to yield the total number of trips generated by each land use. The daily, a.m. peak hour, and p.m. peak hour trip rates are shown in Table 4 for each land use.

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Land Use	Units	Daily	AM Peak Hour	PM Peak Hour
Low Density Residential	d.u.	9.55	0.74	1.01
Medium Density Residential	d.u.	9.45	0.74	1.01
High-Density (Multi-Family) Residential	d.u.	6.47	0.56	0.69
Elementary School	students	1.09	0.28	0.25
Business Park	ksf GFA	Depends on Size of Parcel		
Commercial	ksf GLA	Depends on Size of Parcel		

Internalization and Pass-By Adjustments

The total number of trips generated by each land use was adjusted to account for internal and pass-by trips. Internal trips are those which begin and end within the project boundary. An example is a trip from home to the neighborhood supermarket. Since these trips do not leave the project area, they do not affect operation of the external street system. The following set of internalization rates were developed for the North Vineyard Station Specific Plan:

- Residential 14%
- Neighborhood Commercial 40%
- Community Commercial 30%
- Business Park 15%
- Elementary School 90%

For residential trips, the internalization rate was determined by assuming 80 percent of all non-residential internal trips would be home-based. This equates to a 14 percent internalization rate for residential areas of the project.

Pass-by trips reflect those trips that are attracted from passing traffic already on the external street system. These types of trips occur for retail and commercial land uses. According to *Trip Generation* (Fifth Edition, Institute of Transportation Engineers, 1991), pass-by trip percentages vary based on the size of a retail or commercial land use and the daily traffic volume on the adjacent street. Given the types of retail and commercial land use proposed for the North

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Vineyard Station Specific Plan and the expected daily traffic volumes on the adjacent streets, pass-by factors of 20 percent for neighborhood commercial and 30 percent for community commercial were applied to the gross trip generation. These factors are consistent with values used in other traffic studies in the Sacramento region.

Transit Utilization

Transit utilization for the project was estimated using a two-step process. First, the number of vehicle-trips was converted to person-trips using a ratio of 1.15 persons per vehicle. The second step involved projecting the percentage of person-trips that would utilize transit. For previous studies in this part of the County, up to five percent of all person-trips would be completed using transit. This same percentage is considered applicable for the North Vineyard Station Specific Plan.

Net Project Trip Generation

Tables A-1 through A-3 in Appendix A summarize the trip generation under daily, AM peak hour, and PM peak hour conditions for the preferred project. Tables A-4 through A-6 and Tables A-7 through A-9 illustrate the trip generation for the Alternate Core and Dispersed Core alternatives, respectively. Trip generation is presented in the following categories:

- Total vehicle-trips;
- External and internal vehicle-trips;
- External person-trips; and
- Total transit trips.

Table 5 shows the trip generation for the preferred project and the two alternatives.

Table 5			
Comparison of Total Trip Generation by Land Use Alternative			
Land Use Alternative	Net External Vehicle Trips		
	Daily	AM Peak Hour	PM Peak Hour
Preferred Project	62,989	4,359	6,523
Alternate Core Alternative	63,389	4,407	6,584
Dispersed Core Alternative	64,075	4,511	6,669
Source: Fehr & Peers Associates, Inc. based on land use plans prepared by Donn C. Reiners, Inc. (1998)			

As shown in Table 5, all of the land use alternatives result in total daily and peak hour trips that are within three (3) percent of one another. These variations are caused by minor changes to the size of commercial and business-professional parcels and differences in total intensity. For, example, the Alternate Core alternative includes only half as much acreage designated for business-professional uses as compared to the other two alternatives.

In general, residential trips represent just over 70 percent of the total gross daily vehicle trips with neighborhood and community commercial uses representing 29 percent. Because of internalization and pass-by trips, commercial uses represent only 20 percent of the net external vehicle-trips, whereas, residential, school, and park uses comprise 80 percent of this total.

Project Traffic Distribution

The directional distribution of plan area traffic under existing conditions presented in the October 1996 transportation analysis was used to distribute trips generated by the proposed alternatives. The distribution favored areas to the north and west of the project site given the current location of residential and employment centers in Sacramento County.

Project Traffic Assignment

Based on the plan area trip distribution, trips generated by were added to the existing traffic counts at the study intersections listed in Chapter II. Figure 2 presents daily traffic volumes for existing conditions with and without the preferred project and the Alternate Core and Dispersed Core alternatives. Figures 3 and 4 present the peak hour intersection volumes for the Alternate and Dispersed Core alternatives, respectively. The resulting existing plus project volumes were analyzed to determine the impact of adding plan area traffic to the roadway network. Impacts to each component of the transportation system by the alternatives and how these results compare to the preferred project are presented below.

ROADWAY SYSTEM OPERATIONS

Arterial Roadway Segments

Daily traffic volumes shown on Figure 2 were compared to the capacity criteria for arterial roadway segments presented in Chapter I. The result of this comparison shows that the operation of several study roadway segments would be affected with the addition of traffic from the preferred project and from the plan alternatives.

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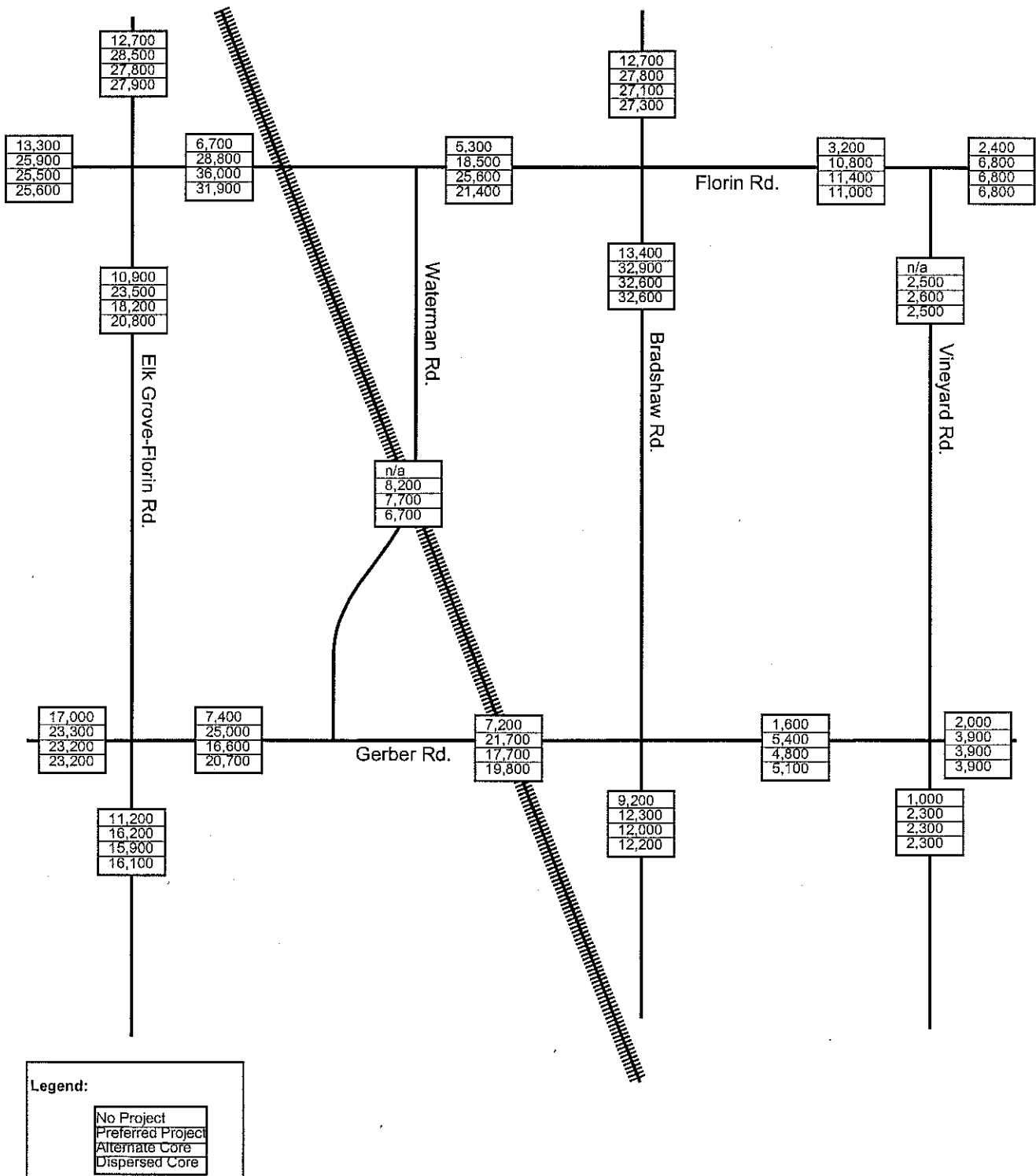
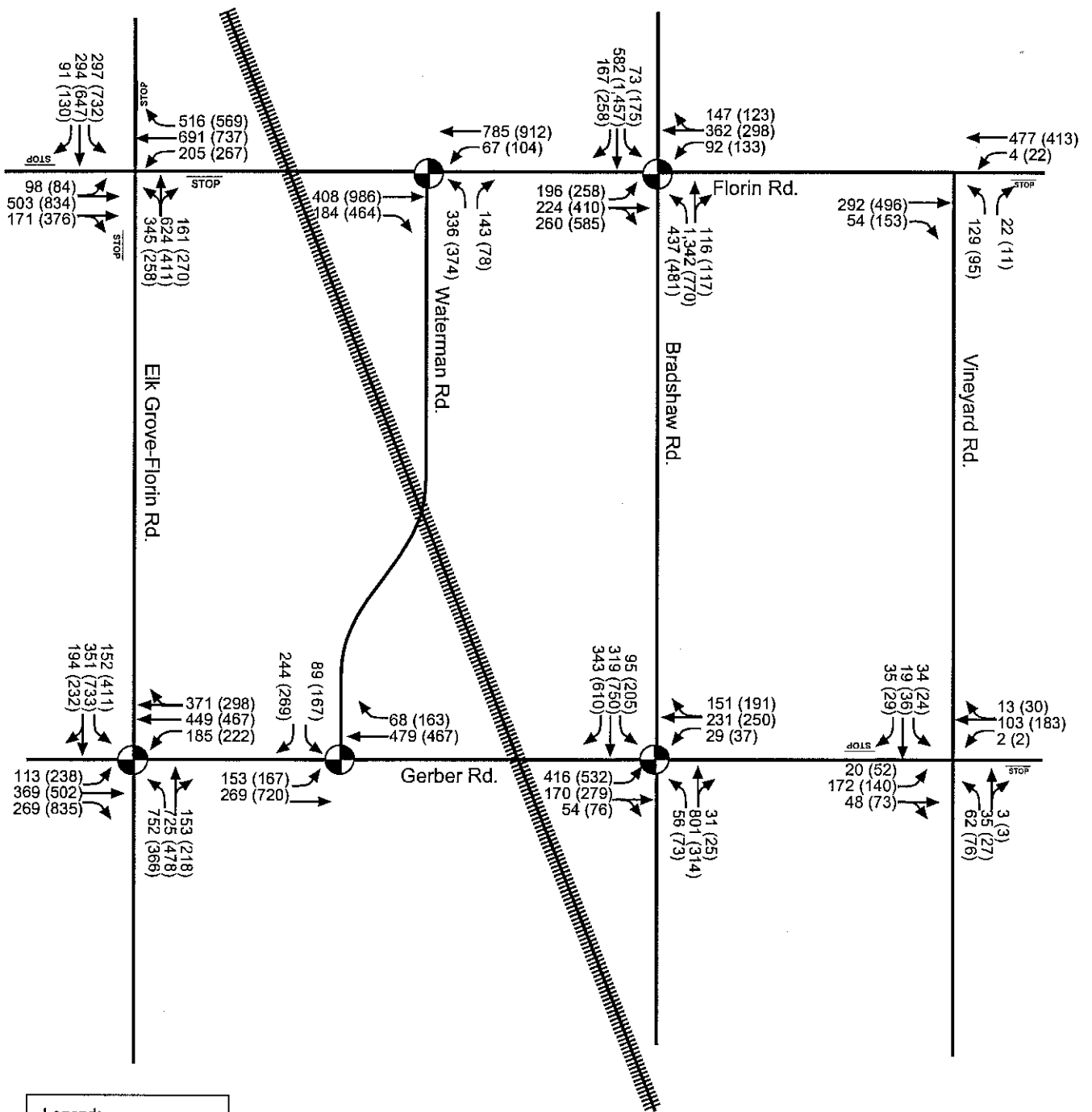


Figure 2 EXISTING PLUS ALTERNATIVES-DAILY TRAFFIC VOLUMES



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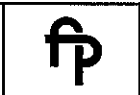
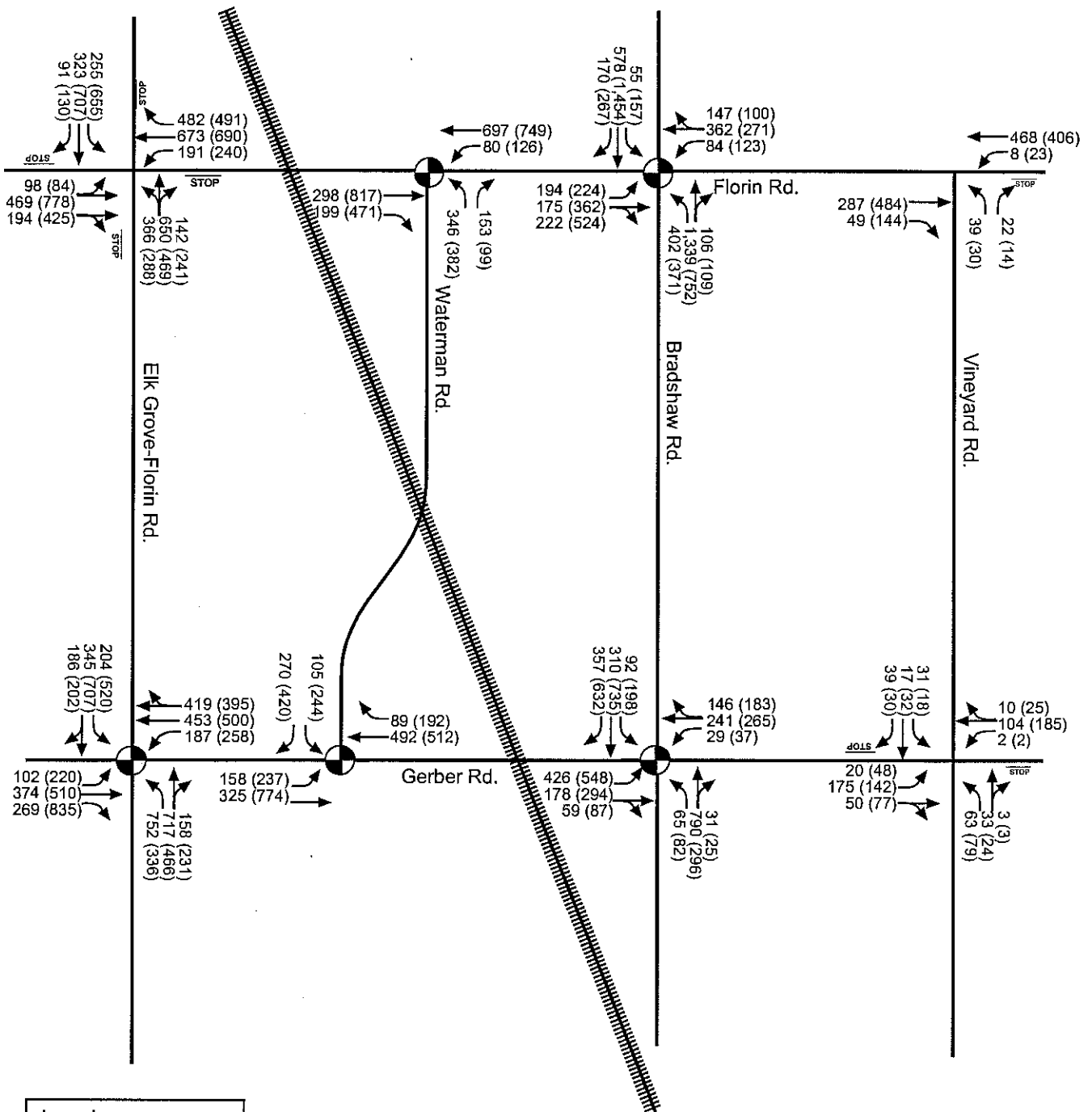
Legend:
 Traffic Signal

Figure 3
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**EXISTING PLUS ALTERNATE CORE
PEAK HOUR TRAFFIC VOLUMES**



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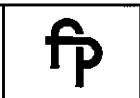
Legend:
 STOP Stop Sign
 Traffic Signal

Figure 4
 812-18-01

EXISTING PLUS DISPERSED CORE PEAK HOUR TRAFFIC VOLUMES



Except for one roadway segment, all of the operational and physical deficiencies identified for the preferred project would occur under both of the proposed alternatives. However, implementation of the Alternate Core alternative would not result in Impact EP-4, which requires the widening of Gerber Road between Elk Grove-Florin Road and Bradshaw Road from two to four lanes. The projected daily volumes on this segment range between 16,600 and 17,700 VPD under the Alternate Core alternative and are slightly less than the 18,000 VPD capacity for urban two lane roadways.

Study Intersections

Existing plus project traffic volumes shown on Figure 3 and 4 were used to calculate peak hour levels of service at the study intersections for the Alternate Core and Dispersed Core alternatives, respectively. Intersection levels of service for existing conditions with and without the preferred project and these alternatives is presented in Table 6. The corresponding calculation worksheets are included in Appendices B and C.

With one exception, implementation of either alternative will not change the intersection impacts identified for the preferred project. The only additional impact would occur at the Florin Road/Waterman Road intersection under the Alternate Core alternative (identified as Impact EP-20A). This intersection is projected to operate at LOS F during the PM peak hour with complete relocation of the commercial core. Under the preferred project and Dispersed Core alternative, this intersection would operate at an acceptable level according to County standards.

Each of the unsignalized intersections operating at LOS C or worse were analyzed to determine if traffic signals would be warranted under existing conditions with development of the plan area. Based on the Caltrans Peak Hour Signal Warrant criteria, this analysis showed that traffic signals would be warranted at the Elk Grove-Florin Road/Florin Road intersection. This location was already determined to warrant signals under existing conditions and the addition of plan area traffic would further degrade operations.

PROPOSED IMPROVEMENTS

Improvements required to eliminate roadway deficiencies with the preferred project were identified in the October 1996 transportation analysis. Changes to these improvements or the need for additional improvements under each plan alternative are presented below.

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Table 6 Existing Plus Plan Area Alternative Intersection Levels of Service								
Intersection	Existing AM (PM)		Existing Plus Preferred Project AM (PM)		Existing Plus Alternate Core Alt. AM (PM)		Existing Plus Dispersed Core Alt. AM (PM)	
	V/C, Delay or RC ¹	LOS ²	V/C, Delay or RC	LOS	V/C, Delay or RC	LOS	V/C, Delay or RC	LOS
Elk Grove-Florin Rd./ Florin Road ³	36.0 s (66.0 s)	E (F)	See Note 4	F (F)	See Note 4	F (F)	See Note 4	F (F)
Florin Rd./ Waterman Rd.	N/A	N/A	0.75 (0.92)	C (E)	0.79 (1.03)	C (F)	0.73 (0.93)	C (E)
Bradshaw Rd./ Florin Rd.	0.89 (0.82)	D (D)	1.61 (2.02)	F (F)	1.63 (2.23)	F (F)	1.60 (2.06)	F (F)
Vineyard Rd./ Florin Rd. ⁵	N/A	N/A	209 (215)	C (C)	217 (191)	C (D)	240 (223)	C (C)
Elk Grove-Florin Rd./ Gerber Rd.	1.05 (1.30)	F (F)	1.36 (1.58)	F (F)	1.35 (1.53)	F (F)	1.34 (1.52)	F (F)
Gerber Rd./ Waterman Rd.	N/A	N/A	0.66 (0.86)	B (D)	0.56 (0.64)	A (B)	0.59 (0.75)	A (C)
Bradshaw Rd./ Gerber Rd.	0.82 (0.66)	D (B)	1.26 (1.37)	F (F)	1.25 (1.31)	F (F)	1.26 (1.26)	F (F)
Vineyard Rd./ Gerber Rd. ⁵	567 (596)	A (A)	501 (406)	A (A)	592 (516)	A (A)	599 (520)	A (A)

Notes: ¹ V/C Ratio = Volume-to-Capacity Ratio (0.00 applies to signalized control); Delay = Average Total Delay (0.0 seconds applies to all-way stop control); and RC= Reserve Capacity (000 vehicles applies to two-way stop control).
² LOS = Level of Service.
³ All-way stop controlled intersection.
⁴ Intersection is substantially over capacity and delay calculation is meaningless.
⁵ Two-way stop controlled intersection.
N/A = Not applicable.

Alternate Core Alternative

Improvements EP-1 through EP-3, EP-5 through EP-14, and EP-16 through EP-19 (identified for the preferred project) would all be required for this alternative. Improvement EP-15A would replace EP-15, and EP-20A is a new required improvement.

Improvement EP-15A: Modify the Bradshaw Road/Florin Road intersection to include the following lane configurations:

- Dual left turn lanes, two through lanes, and a separate right turn lane on the northbound approach;
- Dual left turn lanes, *three* through lanes, and a separate right turn lane on the southbound approach; and
- One left turn lane, two through lanes, and a separate right-turn lane on the eastbound and westbound approaches.

The provision of southbound dual left turn lanes will require widening of the eastbound departure legs on Florin Road to two lanes before merging to a single travel lane. The length and merging distance for these lanes will be approved by Sacramento County Transportation Division staff. The westbound departure leg will be widened as part of Improvement EP-3.

Provision of three southbound lanes will require widening of Bradshaw Road to its ultimate width immediately south of Florin Road. The third southbound lane could be converted to an exclusive right-turn lane at the next downstream intersection or it could be designed to merge into the second through lane. The final configuration will be approved by Sacramento County Transportation Division staff.

These improvements would improve operations at this intersection to LOS C and LOS E in the a.m. and p.m. peak hours, respectively. Improvements should be implemented when the intersection operates unacceptably based on Sacramento County standards.

Improvement EP-20A: Modify the Florin Road/Waterman Road intersection to include the following lane configurations:

- Two through lanes and a separate right-turn lane on the eastbound approach; and
- Two through lanes and a separate left-turn lane on the westbound approach.

The provision of two through lanes in the east-west direction is consistent with Improvement EP-3 (widening of Florin Road between Elk Grove-Florin Road and Bradshaw Road from two to four lanes).

These improvements would improve operations at this intersection to LOS A and LOS B in the a.m. and p.m. peak hours, respectively. Improvements should be implemented when the intersection operates unacceptably based on Sacramento County standards.

Dispersed Core Alternative

Improvements EP-1 through EP-14, and EP-17 through EP-19 (identified for the preferred project) would all be required for this alternative. Improvements EP-15D and EP-16D would replace EP-15 and EP-16, respectively.

Improvement EP-15D: Modify the Bradshaw Road/Florin Road intersection to include the following lane configurations:

- Dual left turn lanes, two through lanes, and a separate right turn lane on the northbound approach;
- Dual left turn lanes, *three* through lanes, and a separate right turn lane on the southbound approach; and
- One left turn lane, two through lanes, and a separate right-turn lane on the eastbound and westbound approaches.

The provision of southbound dual left turn lanes will require widening of the eastbound departure legs on Florin Road to two lanes before merging to a single travel lane. The length and merging distance for these lanes will be approved by Sacramento County Transportation Division staff. The westbound departure leg will be widened as part of Improvement EP-3.

Provision of three southbound lanes will require widening of Bradshaw Road to its ultimate width immediately south of Florin

Road. The third southbound lane could be converted to an exclusive right-turn lane at the next downstream intersection or it could be designed to merge into the second through lane. The final configuration will be approved by Sacramento County Transportation Division staff. (This is the same as Improvement EP-15A).

These modifications would improve operations at this intersection to LOS C and LOS D in the a.m. and p.m. peak hours, respectively. Improvements should be implemented when the intersection operates unacceptably based on Sacramento County standards.

Improvement EP-16: Modify the Elk Grove-Florin Road/Gerber Road intersection to include the following lane configurations:

- Dual left turn lanes, two through lanes, and a separate right turn lane on the northbound and southbound approaches;
- One left turn lane, one through lane, and *dual right-turn lanes* on the eastbound approach; and
- One left turn lane, one through lane, and a shared through/right-turn lane on the westbound approach.

The provision of northbound dual left turn lanes and two westbound through lanes will require widening of the westbound departure leg on Gerber Road to two lanes before merging to a single travel lane in each direction. The length and merging distance for these lanes will be approved by Sacramento County Transportation Division staff.

These improvements would improve operations at this intersection to LOS D in both the a.m. and p.m. peak hours. Improvements should be implemented when the intersection operates unacceptably based on Sacramento County standards.

OPERATIONS WITH PROPOSED IMPROVEMENTS

Following implementation of the proposed improvements, the associated operational deficiencies would be eliminated. Table 7 shows the LOS at the study intersections after implementation of the proposed improvements.

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Table 7 Mitigated Existing Plus Plan Area Alternative Intersection Levels of Service						
Intersection	Existing Plus Preferred Project AM (PM)		Existing Plus Alternate Core Alt. AM (PM)		Existing Plus Dispersed Core Alt. AM (PM)	
	V/C, Delay or RC ¹	LOS ²	V/C, Delay or RC	LOS	V/C, Delay or RC	LOS
Elk Grove-Florin Rd./ Florin Road	0.67 (0.89)	B (D)	0.68 (0.94)	B (E)	0.66 (0.89)	B (D)
Florin Rd./ Waterman Rd.	0.51 (0.56)	A (A)	0.51 (0.68)	A (B)	0.73 (0.93)	C (E)
Bradshaw Rd./ Florin Rd.	0.78 (0.99)	C (E)	0.79 (0.94)	C (E)	0.78 (0.86)	C (D)
Vineyard Rd./ Florin Rd. ³	209 (215)	C (C)	217 (191)	C (D)	240 (223)	C (C)
Elk Grove-Florin Rd./ Gerber Rd.	0.85 (1.00)	D (E)	0.83 (0.99)	D (E)	0.83 (0.95)	D (E)
Gerber Rd./ Waterman Rd.	0.66 (0.86)	B (D)	0.56 (0.64)	A (B)	0.59 (0.75)	A (C)
Bradshaw Rd./ Gerber Rd.	0.85 (0.72)	D (C)	0.70 (0.76)	B (C)	0.70 (0.73)	B (C)
Vineyard Rd./ Gerber Rd. ³	501 (406)	A (A)	592 (516)	A (A)	599 (520)	A (A)

Notes: ¹ V/C Ratio = Volume-to-Capacity Ratio (0.00 applies to signalized control); Delay = Average Total Delay (0.0 seconds applies to all-way stop control); and RC= Reserve Capacity (000 vehicles applies to two-way stop control).
² LOS = Level of Service.
³ Two-way stop controlled intersection.

IV. CUMULATIVE OPERATING CONDITIONS

The purpose of the cumulative analysis is to determine if implementation of the proposed plan alternatives, in addition to planned cumulative growth, will adversely affect the planned transportation system.

PLANNED TRANSPORTATION IMPROVEMENTS

Figure 5 illustrates the planned number of lanes on each roadway segment under cumulative conditions. All of the study intersections are assumed to be signalized under this future scenario.

ROADWAY SYSTEM OPERATIONS

Cumulative Traffic Volumes

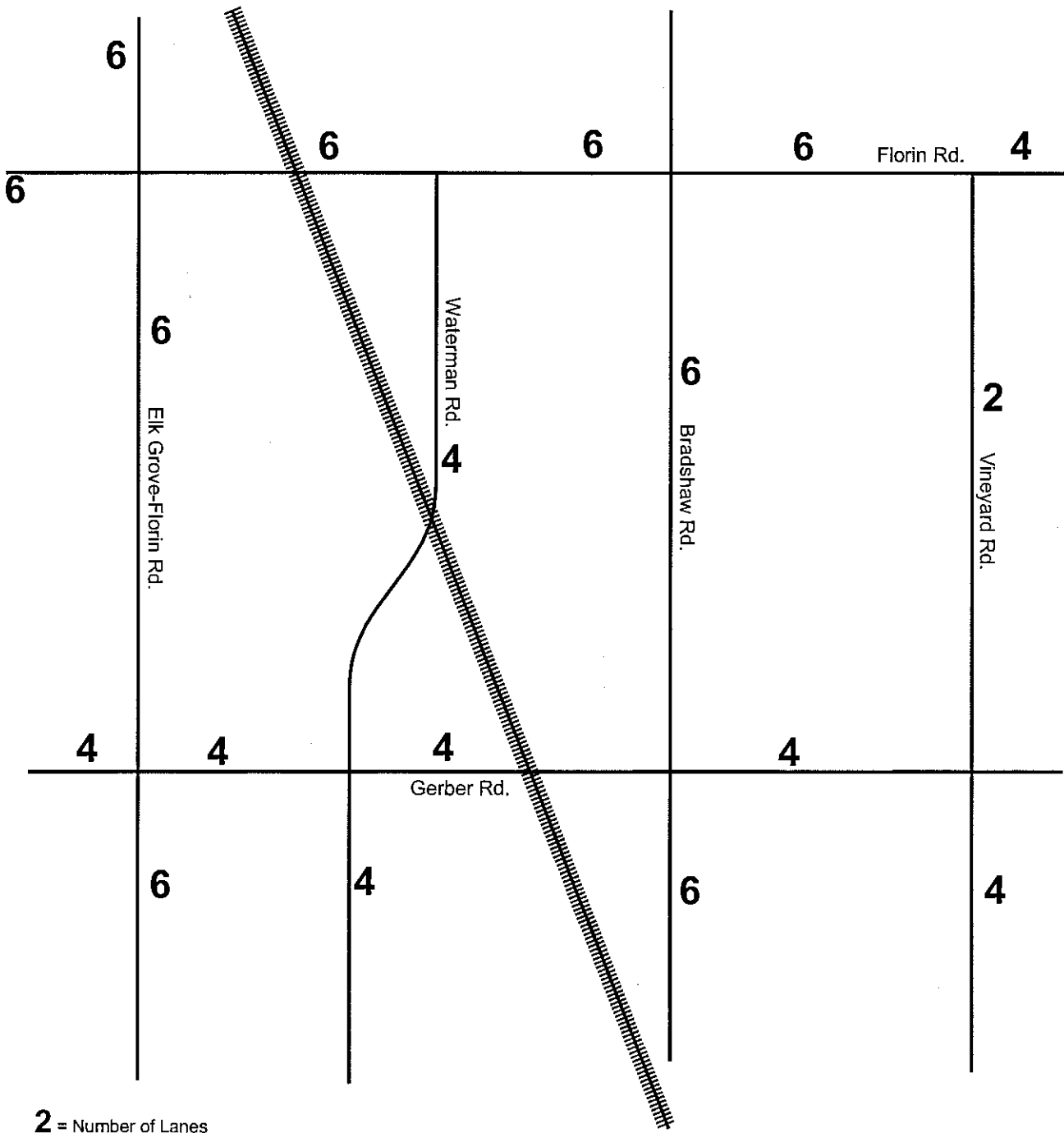
The Sacramento County General Plan traffic model was used to generate daily and peak hour traffic volumes for both of the plan area alternatives under cumulative conditions. Volumes were developed by adding the proposed land uses for each alternative to the traffic model and preparing a new set of traffic forecasts. The same peak hour factors used to develop the cumulative forecasts with and without the preferred project were applied for the alternatives analysis.

At the direction of County Transportation Division staff, the daily and peak hour trips generated by each alternative were factored to match the trip generation information presented in Tables B-4 through B-9. This process provided a conservative trip generation estimate for purposes of identifying project-related impacts and is more consistent with standard trip rates.

The trip distribution of project-generated trips on the study roadway network was obtained from the traffic model output. The distribution used in the October 1996 transportation analysis can also be applied to the alternatives analysis. This information will be used primarily to identify project impacts for roadway segments.

Figure 6 presents the daily traffic volumes for the Alternate Core and Dispersed Core Alternatives. Data for cumulative conditions with and without the preferred project are also shown on this figure for comparison purposes. Figures 7 and 8 present the peak hour turning movement volumes for the Alternate and Dispersed Core alternatives, respectively. Impacts to each element of the study roadway system are presented below.

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January 1998

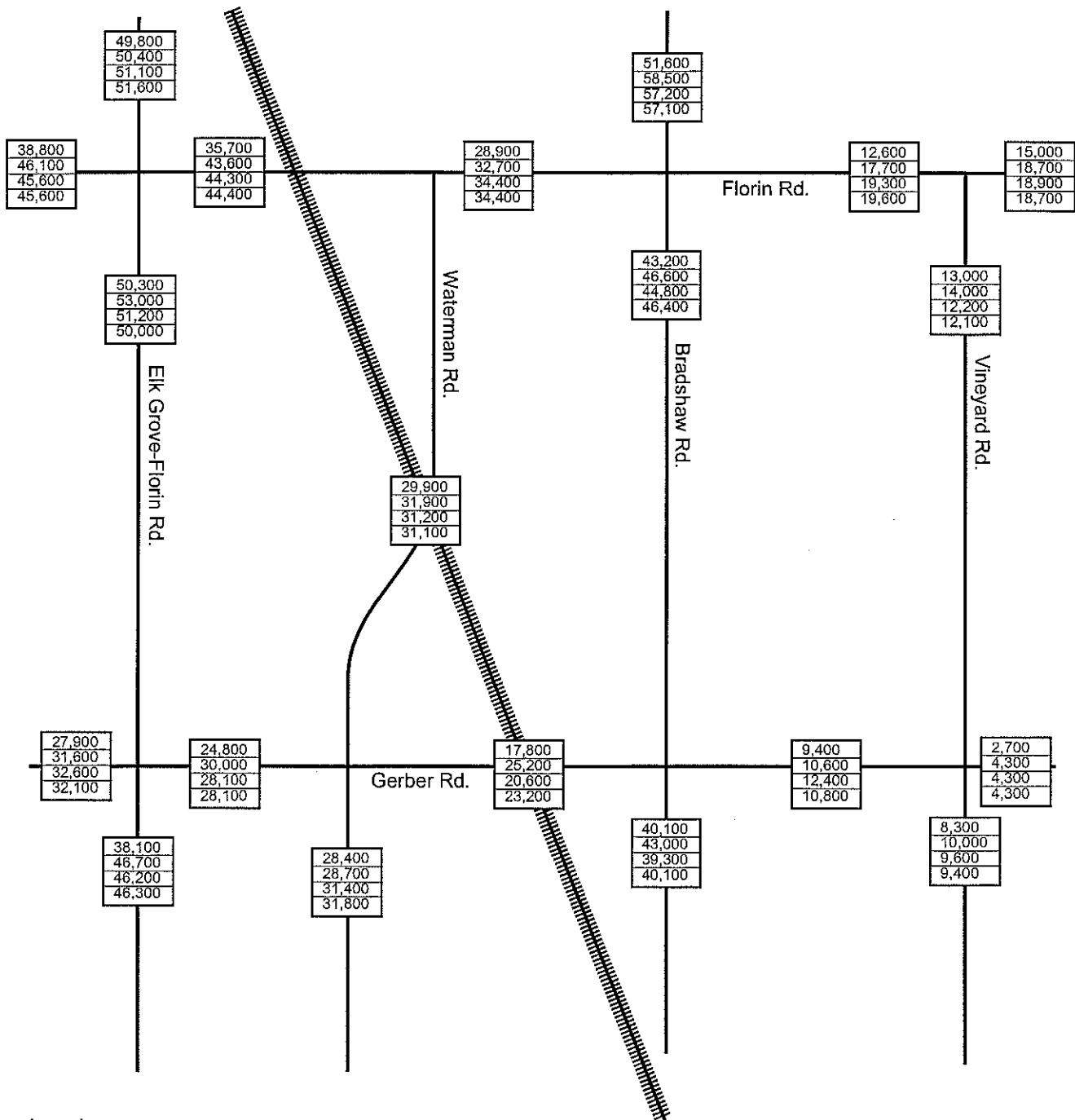


2 = Number of Lanes

Note : All Intersections assumed to be signalized
Source: Sacramento County General Plan - Transportation Plan Map (December 15, 1993).

<p>Figure 5 812-1B-01</p>	<p>PLANNED CUMULATIVE ROADWAY NETWORK</p>	
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North Vineyard Station Specific Plan
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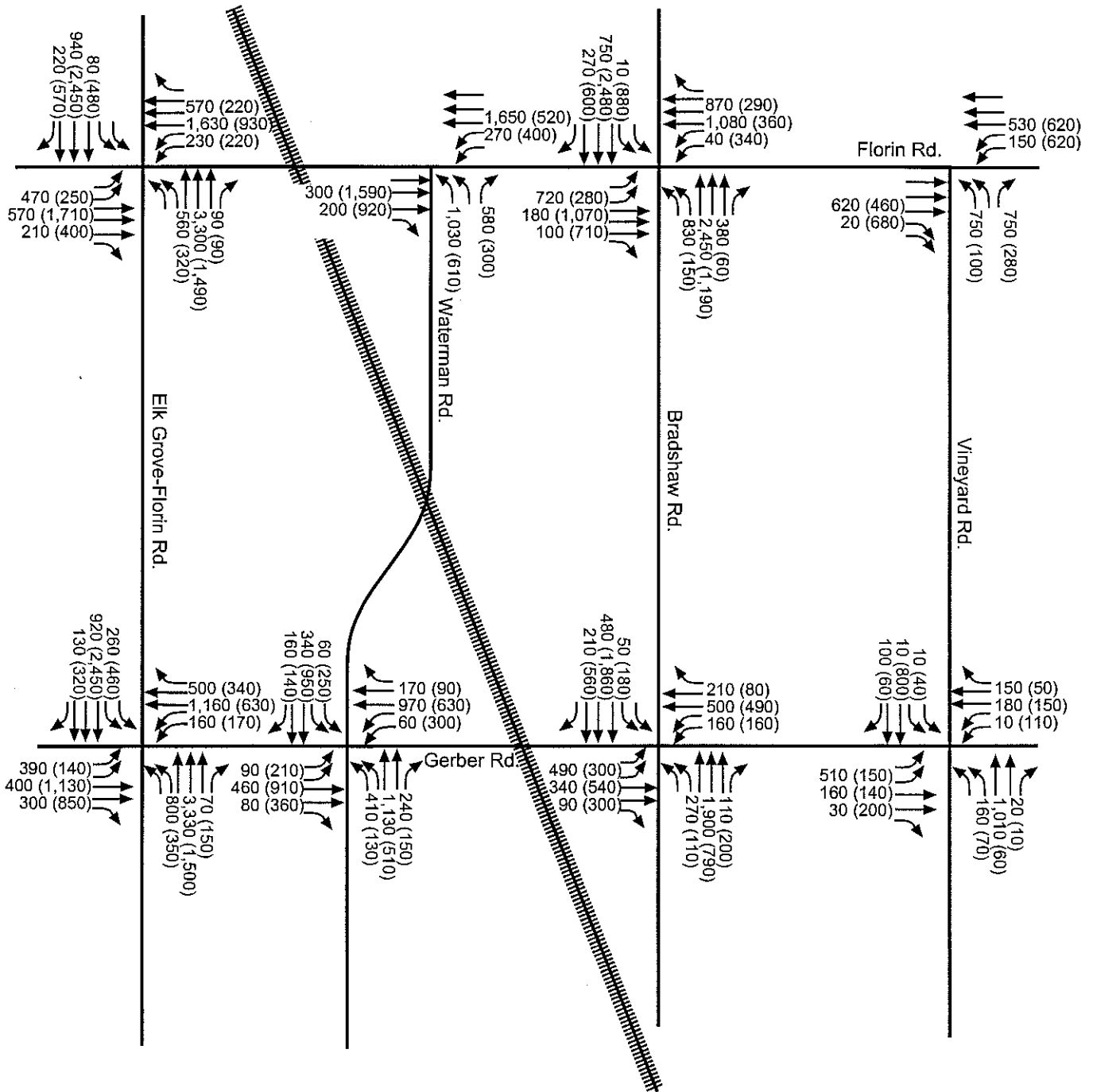


Legend:

- No Project
- Preferred Project
- Alternate Core
- Dispersed Core

<p>Figure 6</p> <p>812-12-01</p>	<p>CUMULATIVE PLUS PLAN AREA ALTERNATIVES-DAILY TRAFFIC VOLUMES</p>	
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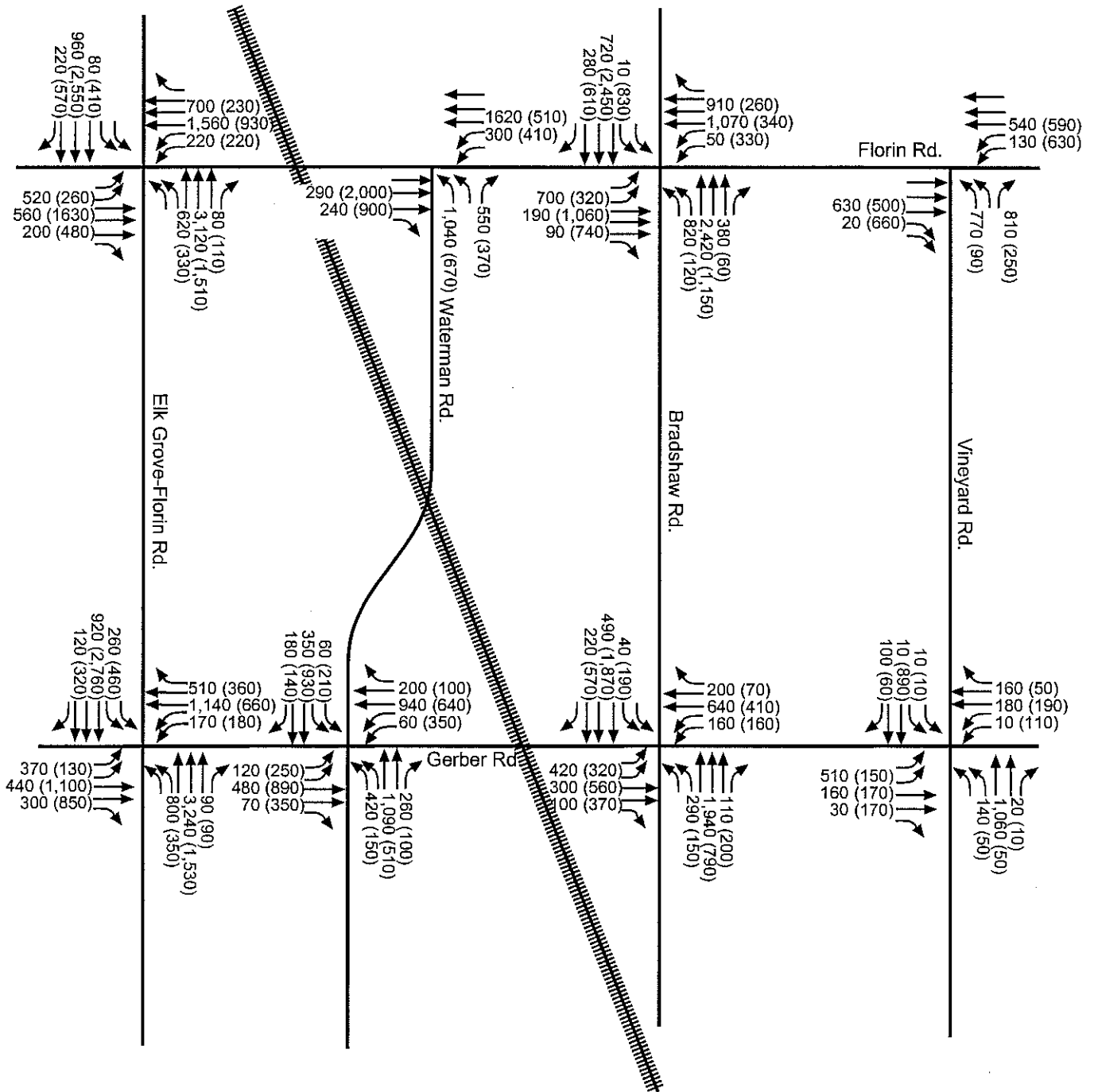
North Vineyard Station Specific Plan
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Note: All Intersections assumed to be signalized

<p>Figure 7</p> <p>812-18-01</p>	<p>CUMULATIVE PLUS ALTERNATE CORE PEAK HOUR TRAFFIC VOLUMES</p>	
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Note: All Intersections assumed to be signalized

Figure 8 CUMULATIVE PLUS DISPERSED CORE PEAK HOUR TRAFFIC VOLUMES

812-18-01



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Arterial Roadway Segments

The volumes on Figure 6 were compared to the arterial roadway capacities presented in Chapter I. Figure 6 shows that all of the plan alternatives would increase traffic on most roadway segments in the study area. All of the operational and physical roadway segment deficiencies identified for the preferred project would also occur with either of the proposed alternatives.

Study Intersections

The peak hour volumes presented on Figures 7 and 8 were used to determine the level of service for the Alternate and Dispersed Core alternatives under cumulative conditions. The results of the peak hour analyses are shown in Table 8 and the corresponding technical calculations are contained in Appendices D and E.

Intersection	Cumulative No Project AM (PM)		Cumulative Plus Preferred Project AM (PM)		Cumulative Plus Alternate Core AM (PM)		Cumulative Plus Dispersed Core AM (PM)	
	V/C Ratio ¹	LOS ²	V/C Ratio	LOS	V/C Ratio	LOS	V/C Ratio	LOS
Elk Grove-Florin Rd./ Florin Rd.	1.28 (1.04)	F (F)	1.40 (1.25)	F (F)	1.42 (1.22)	F (F)	1.37 (1.23)	F (F)
Florin Rd./ Waterman Rd.	0.62 (0.89)	B (D)	0.83 (0.99)	D (E)	0.78 (0.84)	C (D)	0.78 (0.89)	C (D)
Bradshaw Rd./ Florin Rd.	1.17 (1.10)	F (F)	1.27 (1.16)	F (F)	1.33 (1.16)	F (F)	1.33 (1.15)	F (F)
Florin Rd./ Vineyard Rd.	0.59 (0.53)	A (A)	0.59 (0.69)	A (B)	0.57 (0.71)	A (C)	0.60 (0.69)	A (B)
Elk Grove-Florin Rd./ Gerber Rd.	1.31 (1.20)	F (F)	1.47 (1.29)	F (F)	1.49 (1.24)	F (F)	1.45 (1.31)	F (F)
Waterman Rd./ Gerber Rd.	0.87 (0.81)	D (D)	0.70 (0.87)	B (D)	0.82 (0.85)	D (D)	0.81 (0.86)	D (D)
Bradshaw Rd./ Gerber Rd.	0.84 (0.88)	D (D)	0.80 (0.82)	C (D)	0.86 (0.79)	D (C)	0.89 (0.79)	D (C)
Vineyard Rd./ Gerber Rd.	0.72 (0.32)	C (A)	0.69 (0.41)	B (A)	0.65 (0.47)	B (A)	0.68 (0.45)	B (A)

Notes: ¹ V/C Ratio = Volume-to-Capacity Ratio.
² LOS = Level of Service.

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Under cumulative conditions, implementation of either the Alternate Core or Dispersed Core alternative is projected to cause the same impacts that were identified with the preferred project. Although the alternatives result in slightly different V/C ratios, operations at each of the study intersections are expected to be similar under any of the three development scenarios.

PROPOSED IMPROVEMENTS

Based on the projected roadway segment and intersection operations, all of the proposed improvements recommended for the preferred project will be required for both the Alternate Core and Dispersed Core alternatives. No modifications to the proposed improvements are required for either alternative.

OPERATIONS WITH PROPOSED IMPROVEMENTS

Table 9 shows the LOS at the study intersections after implementation of the proposed improvements included in the October 1996 transportation analysis of the preferred project. With implementation of the proposed improvements, the following three intersections would continue to operate at LOS F under cumulative conditions regardless of the plan alternative:

- Elk Grove-Florin Rd./Florin Rd.
- Bradshaw Rd./Florin Rd.
- Elk Grove-Florin Rd./Gerber Rd.

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Table 9 Mitigated Cumulative Intersection Levels of Service								
Intersection	Cumulative No Project AM (PM)		Cumulative Plus Preferred Project AM (PM)		Cumulative Plus Alternate Core AM (PM)		Cumulative Plus Dispersed Core AM (PM)	
	V/C Ratio ¹	LOS ²	V/C Ratio	LOS	V/C Ratio	LOS	V/C Ratio	LOS
Elk Grove-Florin Rd./ Florin Rd.	1.28 (1.04)	F (F)	1.40 (1.25)	F (F)	1.42 (1.22)	F (F)	1.37 (1.23)	F (F)
Florin Rd./ Waterman Rd.	0.62 (0.89)	B (D)	0.83 (0.99)	D (E)	0.78 (0.84)	C (D)	0.78 (0.89)	C (D)
Bradshaw Rd./ Florin Rd.	1.17 (1.10)	F (F)	1.15 (1.16)	F (F)	1.15 (1.16)	F (F)	1.13 (1.15)	F (F)
Florin Rd./ Vineyard Rd.	0.59 (0.53)	A (A)	0.59 (0.69)	A (B)	0.57 (0.71)	A (C)	0.60 (0.69)	A (B)
Elk Grove-Florin Rd./ Gerber Rd.	1.31 (1.20)	F (F)	1.47 (1.29)	F (F)	1.49 (1.24)	F (F)	1.45 (1.31)	F (F)
Waterman Rd./ Gerber Rd.	0.87 (0.81)	D (D)	0.70 (0.87)	B (D)	0.82 (0.85)	D (D)	0.81 (0.86)	D (D)
Bradshaw Rd./ Gerber Rd.	0.84 (0.88)	D (D)	0.80 (0.82)	C (D)	0.86 (0.79)	D (C)	0.89 (0.79)	D (C)
Vineyard Rd./ Gerber Rd.	0.72 (0.32)	C (A)	0.69 (0.41)	B (A)	0.65 (0.47)	B (A)	0.68 (0.45)	B (A)

Notes: ¹ V/C Ratio = Volume-to-Capacity Ratio.
² LOS = Level of Service.

IV. INTERNAL ROADWAY SYSTEM

As part of the specific plan alternatives analysis, the projected traffic volumes on the internal roadway system were also estimated to evaluate the effect of relocating part or all of the commercial core within the plan area. This chapter presents cumulative traffic volumes on the internal street system for the Alternate Core and Dispersed Core alternatives.

PROPOSED CIRCULATION SYSTEM

The proposed circulation system includes a variety of transportation facilities designed to encourage a balanced use of travel modes serving automobile, bicycle, pedestrian, and transit trips. The layout of these facilities is designed to evenly distribute trips to the arterials along the boundaries of the plan area and to limit the need for major roadways within the project.

Figures 9 and 10 show the projected daily traffic volumes on the internal roadway network under cumulative conditions for the Alternate Core and Dispersed Core alternatives, respectively. Based on these volumes and Sacramento County guidelines, the only facilities within the plan area with more than two travel lanes will be the sections of Waterman Road and Bradshaw Road between Florin Road and Gerber Road with four and six lanes, respectively.

Waterman Road is expected to serve approximately 31,000 vehicles per day (VPD), while Bradshaw Road is projected to serve approximately 46,000 VPD. All other roadways within the plan area are projected to serve fewer than 5,500 VPD, which can be accommodated on a two-lane roadway. For those streets serving residential areas with projected volumes between 4,500 and 5,500 VPD, special landscape and access treatments may be required, and in some cases, front-on housing may be precluded. Vineyard Road, which forms the eastern boundary of the site, is planned to be a four-lane facility and would accommodate a projected daily volume of approximately 12,000 VPD.

The only substantial change in the internal roadway system between the preferred project and the alternatives is the elimination of a street between Waterman Road (south of the Central California Traction Railroad) and Gerber Road (approximately halfway between Waterman Road and Bradshaw Road). This connection included a crossing of Gerber Creek and was projected to serve a volume of 5,500 VPD. With the relocation of part or all of the commercial core to the Bradshaw Road /Florin Road corridor, this roadway is not necessary to adequately serve the replacement residential uses and can be eliminated.

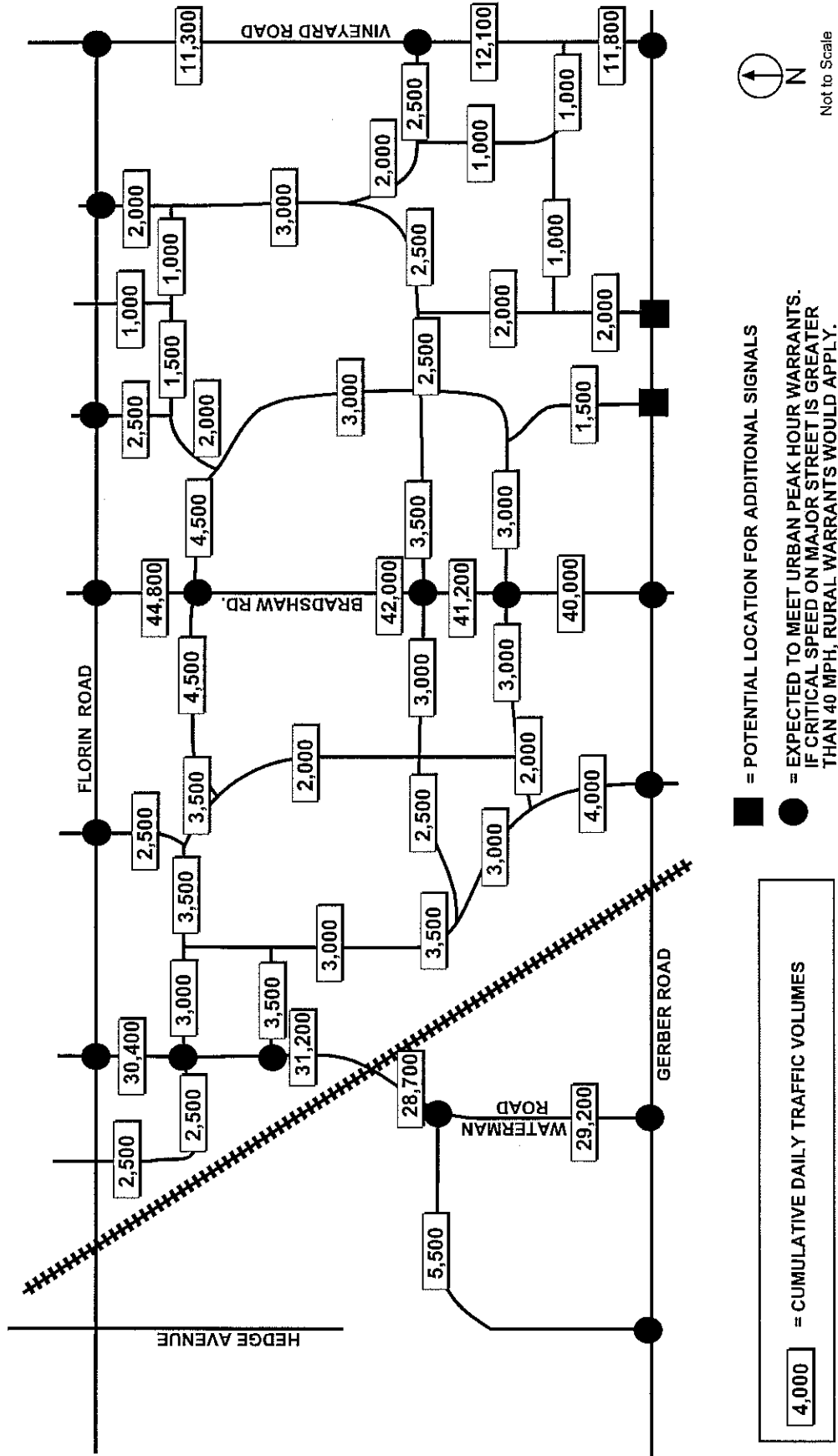


Figure 9
CUMULATIVE DAILY TRAFFIC VOLUMES ON INTERNAL ROADWAYS (ALTERNATE CORE)

921-11-03



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The only significant increases in traffic under either alternative will occur on the east-west collector street crossing Bradshaw road south of Florin Road. The development of retail commercial and/or business-professional uses will increase traffic on this street in the immediate vicinity of Bradshaw Road. However, most of the access points to these uses will be on Bradshaw and Florin Roads thereby limiting the amount of traffic on internal streets.

Figures 9 and 10 also show the location of signalized intersections that are projected to meet signal warrants under cumulative conditions. Implementation of either the Alternate Core or Dispersed Core alternative is not expected to change the need for traffic signals. When tentative maps are produced, the location of traffic signals will be finalized.

V. CONCLUSIONS

The two proposed plan area alternatives for the North Vineyard Station Specific Plan involve the relocation of part or all of the commercial core from near the Gerber Road/Waterman Road intersection to the southwest and southeast quadrants of the Bradshaw Road/Florin Road intersection. Specifically, the Alternate Core alternative includes the relocation of all of the commercial and business-professional uses, as well as the relocation of approximately 300 multi-family dwelling units to Florin Road west of Bradshaw Road. The Dispersed Core alternative only involves relocation of approximately 13 acres of retail commercial uses to the Bradshaw Road/Florin Road intersection. All of the relocated uses will be replaced by single-family residential dwelling units at varying densities, and the total number of residential units for all plan alternatives is 6,339.

The Alternate Core and Dispersed Core alternatives will generate total numbers of daily and peak hour external vehicle trips that are within three percent of the number of trips generated by the preferred project. Slight variations result from differences in the size of commercial and business-professional parcels and differences in dwelling unit intensity. In particular, the Alternate Core alternative includes only half as much acreage designated for business-professional uses as compared to the Dispersed Core and preferred project.

IMPACTS UNDER EXISTING CONDITIONS

Analysis of the proposed alternatives under existing conditions shows that each alternative will result in the same impacts and mitigation measures as the preferred project except for the following:

Alternate Core

- Impact EP-4 would not occur and Improvement EP-4 (widening of Gerber Road between Elk Grove-Florin Road and Bradshaw Road) would not be required;
- Improvement EP-15 would have to be modified to include three southbound lanes on Bradshaw Road instead of two (referred to as Improvement EP-15A); and
- A new impact at the Florin Road/Waterman Road intersection would occur (Impact EP-20A) and would require two through lanes on the eastbound and westbound approaches (referred to as Improvement EP-20A). This is consistent with Improvement EP-3 which widens Florin Road between Elk Grove-Florin Road and Bradshaw Road from two to four lanes.

Dispersed Core

- Improvement EP-15 would have to be modified to include three southbound lanes at the Bradshaw Road/Florin road intersection instead of two (referred to as Improvement EP-15D); and
- Improvement EP-16 would have to be modified to include dual right-turn lanes on the eastbound approach of the Elk Grove-Florin Road/Gerber Road intersection (referred to as Improvement EP-16D).

IMPACTS UNDER CUMULATIVE CONDITIONS

Implementation of either the Alternate Core or Dispersed Core alternatives under cumulative conditions will result in exactly the same roadway segment and intersection impacts as the preferred project. Therefore, no changes to the mitigation measures presented in the October 1996 transportation analysis would be required for either alternative.

The projected traffic volumes on roadways within the plan area will vary less than 3,000 vehicles per day in the vicinity of the relocated land uses. Except for major arterials, the east-west collector street crossing Bradshaw Road south of Florin Road would experience the highest increases in traffic since the relocated retail commercial and business-professional uses will likely include driveways on this roadway. No additional special street treatments (i.e., landscape and access restrictions) or additional collector streets will be required to adequately serve traffic within the plan area.

APPENDIX A

**Trip Generation Tables:
Preferred Project and Alternatives**

Table A-1
DAILY TRIP GENERATION - PREFERRED PROJECT

Land Use	Quantity	Units	Daily Trip Rate	Daily Vehicle Trips	Internal-ization	Internal Trips	External Trips	Pass-By Factor	Net External Trips	External Person Trips	Transit Trips
Low Density Residential	5,259	D.U.	9.55	50,223	14%	7,031	43,192	0%	43,192	49,671	2,484
Medium Density Residential	442	D.U.	9.45	4,177	14%	585	3,592	0%	3,592	4,131	207
High Density Residential	637	D.U.	6.47	4,121	14%	577	3,544	0%	3,544	4,076	204
Neighborhood Commercial	49	ksf GLA	varies	6,555	40%	2,622	3,933	20%	3,925	4,514	226
Community Commercial	262	ksf GLA	varies	16,603	30%	4,981	11,622	30%	11,587	13,325	666
Professional/Offices	103	ksf GFA	14.37	1,480	15%	222	1,258	0%	1,258	1,447	72
Elementary School	600	Students	1.09	654	90%	589	65	0%	65	75	4
Neighborhood Park	64	Acres	2.23	143	90%	128	14	0%	14	16	1
Community Park	27	Acres	2.99	81	50%	40	40	0%	40	46	2
Totals:				84,037	20%	16,775	67,262	N/A	67,219	77,302	3,865

Table A-2 A.M. PEAK HOUR TRIP GENERATION - PREFERRED PROJECT											
Land Use	Quantity	Units	A.M. Peak Trip Rate	Daily Vehicle Trips	Internal-ization	Internal Trips	External Trips	Pass-By Factor	Net External Trips	External Person Trips	Transit Trips
Low Density Residential	5,259	D.U.	0.74	3,892	14%	545	3,347	0%	3,347	3,849	192
Medium Density Residential	442	D.U.	0.74	327	14%	46	281	0%	281	323	16
High Density Residential	637	D.U.	0.56	357	14%	50	307	0%	307	353	18
Neighborhood Commercial	49	ksf GLA	varies	161	40%	64	97	20%	96	111	6
Community Commercial	262	ksf GLA	varies	378	30%	113	265	30%	264	303	15
Professional/Offices	103	ksf GFA	1.62	167	15%	25	142	0%	142	163	8
Elementary School	600	Students	0.28	168	90%	151	17	0%	17	19	1
Neighborhood Park	64	Acres	0.11	7	90%	6	1	0%	1	1	0
Community Park	27	Acres	0.15	4	50%	2	2	0%	2	2	0
Totals:				5,460	18%	1,003	4,457	N/A	4,456	5,125	256

Table A-3 P.M. PEAK HOUR TRIP GENERATION - PREFERRED PROJECT											
Land Use	Quantity	Units	P.M. Peak Trip Rate	Daily Vehicle Trips	Internal-ization	Internal Trips	External Trips	Pass-By Factor	Net External Trips	External Person Trips	Transit Trips
Low Density Residential	5,259	D.U.	1.01	5,312	14%	744	4,568	0%	4,568	5,253	263
Medium Density Residential	442	D.U.	1.01	446	14%	62	384	0%	384	442	22
High Density Residential	637	D.U.	0.69	440	14%	62	378	0%	378	435	22
Neighborhood Commercial	49	ksf GLA	varies	595	40%	238	357	20%	356	410	20
Community Commercial	262	ksf GLA	varies	1,546	30%	464	1,082	30%	1,079	1,241	62
Professional/Offices	103	ksf GFA	1.48	152	15%	23	130	0%	130	149	7
Elementary School	600	Students	0.25	150	90%	135	15	0%	15	17	1
Neighborhood Park	64	Acres	0.22	14	90%	13	1	0%	1	2	0
Community Park	27	Acres	0.30	8	50%	4	4	0%	4	5	0
Totals:				8,663	20%	1,744	6,919	N/A	6,915	7,952	398

**Table A-4
DAILY TRIP GENERATION -- ALTERNATE CORE ALTERNATIVE**

<i>Land Use</i>	<i>Quantity</i>	<i>Units</i>	<i>Daily Trip Rate</i>	<i>Daily Vehicle Trips</i>	<i>Internal-ization</i>	<i>Internal Trips</i>	<i>External Trips</i>	<i>Pass-By Factor</i>	<i>Net External Trips</i>	<i>External Person Trips</i>	<i>Transit Trips</i>
Low Density Residential	5,467	D.U.	9.55	52,210	14%	7,309	44,900	0%	44,900	51,636	2,582
Medium Density Residential	199	D.U.	9.45	1,881	14%	263	1,617	0%	1,617	1,860	93
High Density Residential	673	D.U.	6.47	4,354	14%	610	3,745	0%	3,745	4,306	215
Neighborhood Commercial	70	ksf GLA	varies	7,955	40%	3,182	4,773	20%	3,818	4,391	220
Community Commercial	258	ksf GLA	varies	16,539	30%	4,962	11,577	30%	8,104	9,320	466
Professional/Offices	78	ksf GFA	16.90	1,318	15%	198	1,120	0%	1,120	1,289	64
Elementary School	600	Students	1.09	654	90%	589	65	0%	65	75	4
Neighborhood Park	83	Acres	2.23	185	90%	167	19	0%	19	21	1
Community Park	0	Acres	2.99	0	50%	0	0	0%	0	0	0
Totals:				85,096	2031%	17,279	67,817	N/A	63,389	72,898	3,645

Table A-5
A.M. PEAK HOUR TRIP GENERATION -- ALTERNATE CORE ALTERNATIVE

Land Use	Quantity	Units	Daily Trip Rate	Daily Vehicle Trips	Internal-ization	Internal Trips	External Trips	Pass-By Factor	Net External Trips	External Person Trips	Transit Trips
Low Density Residential	5,467	D.U.	0.74	4,046	14%	566	3,479	0%	3,479	4,001	200
Medium Density Residential	199	D.U.	0.74	147	14%	21	127	0%	127	146	7
High Density Residential	673	D.U.	0.56	377	14%	53	324	0%	324	373	19
Neighborhood Commercial	70	ksf GLA	varies	192	40%	77	115	20%	92	106	5
Community Commercial	258	ksf GLA	varies	377	30%	113	264	30%	185	212	11
Professional/Offices	78	ksf GFA	2.75	215	15%	32	182	0%	182	210	10
Elementary School	600	Students	0.28	168	90%	151	17	0%	17	19	1
Neighborhood Park	83	Acres	0.11	9	90%	8	1	0%	1	1	0
Community Park	0	Acres	0.15	0	50%	0	0	0%	0	0	0
Totals:				5,530	18%	1,021	4,509	N/A	4,407	5,068	253

**Table A-6
P.M. PEAK HOUR TRIP GENERATION -- ALTERNATE CORE ALTERNATIVE**

<i>Land Use</i>	<i>Quantity</i>	<i>Units</i>	<i>Daily Trip Rate</i>	<i>Daily Vehicle Trips</i>	<i>Internal-ization</i>	<i>Internal Trips</i>	<i>External Trips</i>	<i>Pass-By Factor</i>	<i>Net External Trips</i>	<i>External Person Trips</i>	<i>Transit Trips</i>
Low Density Residential	5,467	D.U.	1.01	5,522	14%	773	4,749	0%	4,749	5,461	273
Medium Density Residential	199	D.U.	1.01	201	14%	28	173	0%	173	199	10
High Density Residential	673	D.U.	0.69	464	14%	65	399	0%	399	459	23
Neighborhood Commercial	70	ksf GLA	varies	726	40%	290	436	20%	348	401	20
Community Commercial	258	ksf GLA	varies	1,540	30%	462	1,078	30%	755	868	43
Professional/Offices	78	ksf GFA	2.16	168	15%	25	143	0%	143	165	8
Elementary School	600	Students	0.25	150	90%	135	15	0%	15	17	1
Neighborhood Park	83	Acres	0.22	18	90%	16	2	0%	2	2	0
Community Park	0	Acres	0.30	0	50%	0	0	0%	0	0	0
Totals:				8,790	20%	1,795	6,994	N/A	6,584	7,572	379

**Table A-7
DAILY TRIP GENERATION -- DISPERSED CORE ALTERNATIVE**

<i>Land Use</i>	<i>Quantity</i>	<i>Units</i>	<i>Daily Trip Rate</i>	<i>Daily Vehicle Trips</i>	<i>Internal-ization</i>	<i>Internal Trips</i>	<i>External Trips</i>	<i>Pass-By Factor</i>	<i>Net External Trips</i>	<i>External Person Trips</i>	<i>Transit Trips</i>
Low Density Residential	5,467	D.U.	9.55	52,210	14%	7,309	44,900	0%	44,900	51,636	2,582
Medium Density Residential	199	D.U.	9.45	1,881	14%	263	1,617	0%	1,617	1,860	93
High Density Residential	673	D.U.	6.47	4,354	14%	610	3,745	0%	3,745	4,306	215
Neighborhood Commercial	70	kfs GLA	varies	7,955	40%	3,182	4,773	20%	3,818	4,391	220
Community Commercial	258	kfs GLA	varies	16,539	30%	4,962	11,577	30%	8,104	9,320	466
Professional/Offices	158	kfs GFA	13.45	2,125	15%	319	1,806	0%	1,806	2,077	104
Elementary School	600	Students	1.09	654	90%	589	65	0%	65	75	4
Neighborhood Park	83	Acres	2.23	185	90%	167	19	0%	19	21	1
Community Park	0	Acres	2.99	0	50%	0	0	0%	0	0	0
Totals:				85,903	2026%	17,400	68,503	N/A	64,075	73,686	3,684

Table A-8 A.M. PEAK HOUR TRIP GENERATION -- DISPERSED CORE ALTERNATIVE											
Land Use	Quantity	Units	Daily Trip Rate	Daily Vehicle Trips	Internal-ization	Internal Trips	External Trips	Pass-By Factor	Net External Trips	External Person Trips	Transit Trips
Low Density Residential	5,467	D.U.	0.74	4,046	14%	566	3,479	0%	3,479	4,001	200
Medium Density Residential	199	D.U.	0.74	147	14%	21	127	0%	127	146	7
High Density Residential	673	D.U.	0.56	377	14%	53	324	0%	324	373	19
Neighborhood Commercial	70	ksf GLA	varies	192	40%	77	115	20%	92	106	5
Community Commercial	258	ksf GLA	varies	377	30%	113	264	30%	185	212	11
Professional/Offices	158	ksf GFA	2.13	337	15%	50	286	0%	286	329	16
Elementary School	600	Students	0.28	168	90%	151	17	0%	17	19	1
Neighborhood Park	83	Acres	0.11	9	90%	8	1	0%	1	1	0
Community Park	0	Acres	0.15	0	50%	0	0	0%	0	0	0
Totals:				5,652	18%	1,040	4,613	N/A	4,511	5,187	259

**Table A-9
P.M. PEAK HOUR TRIP GENERATION -- DISPERSED CORE ALTERNATIVE**

<i>Land Use</i>	<i>Quantity</i>	<i>Units</i>	<i>Daily Trip Rate</i>	<i>Daily Vehicle Trips</i>	<i>Internal-ization</i>	<i>Internal Trips</i>	<i>External Trips</i>	<i>Pass-By Factor</i>	<i>Net External Trips</i>	<i>External Person Trips</i>	<i>Transit Trips</i>
Low Density Residential	5,467	D.U.	1.01	5,522	14%	773	4,749	0%	4,749	5,461	273
Medium Density Residential	199	D.U.	1.01	201	14%	28	173	0%	173	199	10
High Density Residential	673	D.U.	0.69	464	14%	65	399	0%	399	459	23
Neighborhood Commercial	70	ksf GLA	varies	726	40%	290	436	20%	348	401	20
Community Commercial	258	ksf GLA	varies	1,540	30%	462	1,078	30%	755	868	43
Professional/Offices	158	ksf GFA	1.70	269	15%	40	228	0%	228	263	13
Elementary School	600	Students	0.25	150	90%	135	15	0%	15	17	1
Neighborhood Park	83	Acres	0.22	18	90%	16	2	0%	2	2	0
Community Park	0	Acres	0.30	0	50%	0	0	0%	0	0	0
Totals:				8,890	20%	1,810	7,080	N/A	6,669	7,669	383

APPENDIX B

**Existing Plus Alternate Core Alternative
Level of Service Calculations**

Level Of Service Computation Report
 1994 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #3 ELK GROVE FLORIN/FLORIN ROAD ALTA E P AM

Cycle (sec): 1 Critical Vol./Cap. (X): 4.737
 Loss Time (sec): 0 Average Delay (sec/veh): OVERFLOW
 Optimal Cycle: 0 Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound												
Movement:	L	T	R	L	T	R	L	T	R	L	T	R										
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign												
Rights:	Include			Include			Include			Include												
Lanes:	0	0	1	0	0	1	1	0	1	0	1	0	0	1	0	1	0	1	0	1	0	1

Volume Module:

Base Vol:	345	624	161	297	294	91	98	503	171	205	691	516
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Bse:	345	624	161	297	294	91	98	503	171	205	691	516
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	363	657	169	313	309	96	103	529	180	216	727	543
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	363	657	169	313	309	96	103	529	180	216	727	543
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Vol.:	363	657	169	313	309	96	103	529	180	216	727	543

Saturation Flow Module:

Sat/Lane:	251	251	251	286	286	286	253	253	253	277	277	277
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.31	0.55	0.14	1.00	1.00	1.00	0.25	1.31	0.44	1.00	1.00	1.00
Final Sat.:	77	139	36	286	286	286	64	330	112	277	277	277

Capacity Analysis Module:

Vol/Sat:	4.74	4.74	4.74	1.09	1.08	0.34	1.60	1.60	1.60	0.78	2.62	1.96
Crit Moves:	****			****			****			****		

Level Of Service Module:

Delay/Veh:	xxxxx	xxxx	xxxxx	64.0	60.7	3.6	445.0	445	445.0	19.4	xxxx	1718
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	xxxxx	xxxx	xxxxx	64.0	60.7	3.6	445.0	445	445.0	19.4	xxxx	1718
LOS by Move:	F	F	F	F	F	A	F	F	F	C	F	F

Level Of Service Computation Report
1994 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #3 ELK GROVE FLORIN/FLORIN ROAD

ALTA EPAM

Cycle (sec): 1 Critical Vol./Cap. (X): 4.732
Loss Time (sec): 0 Average Delay (sec/veh): OVERFLOW
Optimal Cycle: 0 Level Of Service: F

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module: Table with 12 columns representing different traffic movements and 10 rows of volume-related metrics.

Saturation Flow Module: Table with 12 columns and 4 rows showing saturation flow rates and adjustments.

Capacity Analysis Module: Table with 12 columns and 2 rows showing volume-to-saturation ratios and critical moves.

Level Of Service Module: Table with 12 columns and 4 rows showing delay per vehicle and level of service (LOS) by move.

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND WATERMAN RD
Problem Statement: ALTA EXIST PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: WATERMAN RD R L N R T T T L T H H H T Approach 1 << >> LT--^ v v v <^--RTH LTH--> <--TH 1 1 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 1 1 Approach 4: WATERMAN RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 1 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph A2B1 67(B1) 785- 67= 718(A2) 67 A1A2 718(A2) OR 408(A1) 718 A4B3 336(B3) OR 100(A4) 336			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 0 TH= 0 RT= 0 Approach 3 2: RT= 0 TH= 785 LT= 67 Approach 2 Approach 1-->		Step 5. ASSIGN LANE VOLUMES, in vph <- 785 v- 67 < > 4: RT= 143 TH= 0 LT= 336 Approach 4 408 --> 184 -v				Step 7. SUM OF CRITICAL VOLUMES 785(B1A2)+336(B3)+0()+0() = 1121 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) C			
Step 3. IDENTIFY PHASING <-- A2B1 v-- --> <-- A1A2 < ^ A4B3 		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .79			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

ALT A

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN AND BRADSHAW

Design Hour: AM

Problem Statement: ALTA -EXIST PLUS PROJECT

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW RD 1 1 1 ^ R L N R T T T L T H H H T Approach 1 << >> ^--RT 1 LT--^ v v v <^--RTH 1 LTH--> <--TH TH--> <v--LTH 1 RTH--v ^ ^ ^ v--LT 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN ROAD H H 1 1 Approach 4:BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B4B3 73(B4) 437- 73= 364(B3) 73 A4B3 364(B3) 1458- 364=1094(A4) 364 A3A4 1094(A4) OR 582(A3) 1094 B2B1 92(B1) 196- 92= 104(B2) 92 A1B2 104(B2) 484- 104= 380(A1) 104 A1A2 509(A2) OR 380(A1) 509			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3 3: LT= 73 2:RT= 147 TH= 582 TH= 362 RT= 167 v LT= 92 <--Approach 2		1 5 6 8 7 7 2 3 ^+ 147 <v > <+ 362 v- 92		437(B4B3)+1094(A4)+196(B1B2)+509(A2) = 2236 vph	
Approach 1--> 1:LT= 196 ^ 4: RT= 116 TH= 224 TH=1342 RT= 260 LT= 437 Approach 4		196 -^ < ^ > 224 +> + + 260 +v 1 4 3 1 3 4 1 7 2 6		Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F	
				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
< B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		Exclusive right turns reduced 30 % V/C Ratio = 1.63
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN AND BRADSHAW
Problem Statement: ALTA -EXIST PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW RD 1 1 1 ^ R L N R T T T L T H H H T Approach 1 < < > > ^--RT 1 LT--^ v v v <^--RTH 1 LTH--> <--TH TH--> <v-LTH 1 RTH--v ^ ^ ^ v--LT 1 RT--v < < > > Approach 2 L L T R R T T H T T FLORIN ROAD H H 1 1 Approach 4:BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Prob- Critical Carryover Critical able Volume to next Volume Phase in vph phase in vph ----- B4B3 175(B4) 481- 175= 306(B3) 175 A4B3 306(B3) 887- 306= 581(A4) 306 A3A4 1457(A3) OR 581(A4) 1457 B2B1 133(B1) 258- 133= 125(B2) 133 A1B2 125(B2) 995- 125= 870(A1) 125 A1A2 870(A1) OR 421(A2) 870			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 175 Approach 3 2:RT= 123 TH=1457 TH= 298 RT= 258 v LT= 133 ----- <--Approach 2 ----- Approach 1--> ----- 1:LT= 258 ^ 4: RT= 117 TH= 410 TH= 770 RT= 585 LT= 481 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 1 2 4 1 ----- 5 5 7 8 7 5 < v > ----- 258 -^ < ^ > 410 +> + + 585 +v ----- 4 7 1 ----- 8 7 1 1 0 7				Step 7. SUM OF CRITICAL VOLUMES 481(B4B3)+1457(A3)+258(B1B2)+870(A1) = 3066 vph ----- Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- F ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 ----- --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS ----- A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 > ----- Exclusive right turns reduced 30 % V/C Ratio = 2.23			

Level Of Service Computation Report

1985 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 FLORIN/VINEYARD

PLTAEPRM

Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound						
Movement:	L	T	R	L	T	R	L	T	R	L	T	R				
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled						
Rights:	Include			Include			Include			Include						
Lanes:	1	0	0	0	0	0	0	0	1	0	1	1	0	1	0	0

Volume Module:

Base Vol:	129	0	22	0	0	0	0	292	54	4	477	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Bse:	129	0	22	0	0	0	0	292	54	4	477	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	136	0	23	0	0	0	0	307	57	4	502	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Final Vol.:	136	0	23	0	0	0	0	307	57	4	502	0

Adjusted Volume Module:

Grade:	0%			0%			0%			0%		
% Cycle/Cars:	xxxx	xxxx		xxxx	xxxx		xxxx	xxxx		xxxx	xxxx	
% Truck/Comb:	xxxx	xxxx		xxxx	xxxx		xxxx	xxxx		xxxx	xxxx	
PCE Adj:	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.00	1.00	1.10	1.00	1.00
Cycl/Car PCE:	xxxx	xxxx		xxxx	xxxx		xxxx	xxxx		xxxx	xxxx	
Trck/Cmb PCE:	xxxx	xxxx		xxxx	xxxx		xxxx	xxxx		xxxx	xxxx	
Adj Vol.:	149	0	25	0	0	0	0	307	57	5	502	0

Critical Gap Module: >> Population: 0 << >> Run Speed(E/W): 30 MPH <<

RT Rad/Ang:	20.0 ft/90.0 deg	20.0 ft/90.0 deg	20.0 ft/90.0 deg	20.0 ft/90.0 deg
Critical Gp:	7.0 xxxx	5.5 xxxxx	xxxxx xxxxx	xxxxx xxxxx

Capacity Module:

Cnflct Vol:	814 xxxx	307 xxxx	xxxx xxxx xxxxx	xxxx xxxx xxxxx	364 xxxx xxxxx
Potent Cap.:	260 xxxx	778 xxxx	xxxx xxxx xxxxx	xxxx xxxx xxxxx	729 xxxx xxxxx
% Used Cap.:	57.6 xxxx	3.3 xxxxx	xxxxx xxxx xxxxx	xxxxx xxxx xxxxx	0.6 xxxx xxxxx
Impedance:	xxxx xxxx	0.98 xxxx	xxxx xxxx xxxxx	xxxx xxxx xxxxx	1.00 xxxx xxxxx
Actual Cap.:	260 xxxx	778 xxxx	xxxx xxxx xxxxx	xxxx xxxx xxxxx	729 xxxx xxxxx

Level Of Service Module:

Unused Cap.:	110 xxxx	753 xxxx	xxxx xxxx xxxxx	xxxx xxxx xxxxx	724 xxxx xxxxx
LOS by Move:	D * A	* * *	* * *	* * *	A * *
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	xxxx xxxx xxxxx	xxxx xxxx xxxxx	xxxx xxxx xxxxx	xxxx xxxx xxxxx	xxxx xxxx xxxxx
Unused Cap.:	xxxx xxxx xxxxx	xxxx xxxx xxxxx	xxxx xxxx xxxxx	xxxx xxxx xxxxx	xxxx xxxx xxxxx
Shared LOS:	* * *	* * *	* * *	* * *	* * *

Level Of Service Computation Report
1985 HCM Unsignalized Method (Base Volume Alternative)

Intersection #2 FLORIN/VINEYARD

ALTA EPPM

Level Of Service: E

Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L-T-R), Control (Stop Sign, Uncontrolled), Rights (Include), Lanes (1 0 0 0 1, etc.)

Volume Module: Base Vol, Growth Adj, PasserByVol, Initial Bse, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Vol.

Adjusted Volume Module: Grade, % Cycle/Cars, % Truck/Comb, PCE Adj, Cycl/Car PCE, Trck/Cmb PCE, Adj Vol.

Critical Gap Module: >> Population: 0 << >> Run Speed(E/W): 30 MPH <<
RT Rad/Ang: 20.0 ft/90.0 deg 20.0 ft/90.0 deg 20.0 ft/90.0 deg 20.0 ft/90.0 deg
Critical Gp: 7.0 xxxx 5.5 xxxxx xxxxx xxxxx xxxxx xxxxx 5.5 xxxxx xxxxx

Capacity Module: Cnflct Vol, Potent Cap., % Used Cap., Impedance, Actual Cap.

Level Of Service Module: Unused Cap., LOS by Move, Movement, Shared Cap., Unused Cap., Shared LOS

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELK GROVE FLORIN AND GERBER
Problem Statement: ALTA EXIST PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE FLO 1 1 ^ R L N R T T T L T H H H T Approach 1 << >> ^--RT 1 LT--^ v v v <^--RTH 1 LTH-^> <--TH 1 1 TH--> <v--LTH RTH-v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 1 1 Approach 4: ELK GROVE FLO		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- Possible Volume Adjusted able Critical Carryover Critical Volume to next Volume Phase in vph phase in vph ----- B4B3 152(B4) 752- 152= 600(B3) 152 A4B3 600(B3) 878- 600= 278(A4) 600 A3A4 545(A3) OR 278(A4) 545 B2B1 113(B2) 185- 113= 72(B1) 113 A2B1 72(B1) 410- 72= 338(A2) 72 A1A2 369(A1) OR 338(A2) 369			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
3: LT= 152 TH= 351 RT= 194 Approach 3 2: RT= 371 TH= 449 LT= 185 <--Approach 2 Approach 1-->		1 3 1 9 5 5 4 1 2 + + < v > 113 -^ 369 -> 269 -v 4: RT= 153 TH= 725 LT= 752 Approach 4		^+ 371 <+ 39 <- 410 v- 185 752(B4B3)+545(A3)+185(B2B1)+369(A1) = 1851 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
< B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4		Exclusive right turns reduced 30 % V/C Ratio = 1.35

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND WATERMAN RD
Problem Statement: ALTA EXIST PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: WATERMAN RD 1 1 ^ R L N R T T T L T H H H T Approach 1 << >> ^-RT 1 1 LT--^ v v v <^-RTH LTH-^> <--TH 1 1 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H Approach 4: WATERMAN RD		-----Approach----- : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 0 0 d.Opposing volume in vph : 0 269 0 244 e.LT capacity on green (vph) : 0 0 0 0 f.LT capacity in vph (b+e) : 0 0 0 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? : NO NO				Prob-able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph A1B2 153(B2) 269- 153= 116(A1) 153 A1A2 479(A2) OR 116(A1) 479 A3B4 171(A3) OR 89(B4) 171			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3 3: LT= 89 TH= 0 RT= 244 v <--Approach 2 Approach 1-->		2 4 8 4 9 < >		632(B2A2)+171(A3)+0()+0() = 803 vph	
Approach 4 1:LT= 153 TH= 269 RT= 0		4: RT= 0 TH= 0 LT= 0		Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) A	
				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ A1B2 --> --> <-- A1A2 A3B4 v >		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				Exclusive right turns reduced 30 % V/C Ratio = .56

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND WATERMAN RD
Problem Statement: ALTA EXIST PLUS PROJECT

Design Hour: PM

<p>Step 1. IDENTIFY LANE GEOMETRY</p> <p>Approach 3: WATERMAN RD</p> <pre> 1 1 ^ R L N R T T T L T H H H T ----- Approach 1 1 LT--^ ^--RT 1 LTH--> <^--RTH 1 TH--> <--TH 1 RTH-v> <v--LTH RT--v> ^ ^ ^ v--LT < < > > Approach 2 L L T R R T T H T T H H ----- Approach 4: WATERMAN RD </pre>	<p>Step 4. LEFT TURN CHECK</p> <pre> -----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? : </pre>	<p>Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Prob-Phase</th> <th>Possible Critical Volume in vph</th> <th>Volume Carryover to next phase</th> <th>Adjusted Critical Volume in vph</th> </tr> </thead> <tbody> <tr> <td>A1B2</td> <td>167(B2)</td> <td>720-167=553(A1)</td> <td>167</td> </tr> <tr> <td>A1A2</td> <td>553(A1) OR 467(A2)</td> <td></td> <td>553</td> </tr> <tr> <td>A3B4</td> <td>188(A3) OR 167(B4)</td> <td></td> <td>188</td> </tr> </tbody> </table>	Prob-Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph	A1B2	167(B2)	720-167=553(A1)	167	A1A2	553(A1) OR 467(A2)		553	A3B4	188(A3) OR 167(B4)		188
Prob-Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph															
A1B2	167(B2)	720-167=553(A1)	167															
A1A2	553(A1) OR 467(A2)		553															
A3B4	188(A3) OR 167(B4)		188															
<p>Step 2. IDENTIFY VOLUMES, in vph</p> <pre> Approach 3 3: LT= 167 2:RT= 163 TH= 0 TH= 467 RT= 269 v LT= 0 ----- <--Approach 2 Approach 1--> ----- 1:LT= 167 ^ 4: RT= 0 TH= 720 TH= 0 RT= 0 LT= 0 Approach 4 </pre>	<p>Step 5. ASSIGN LANE VOLUMES, in vph</p> <pre> 2 1 ----- 6 6 9 7 < > </pre>	<p>Step 7. SUM OF CRITICAL VOLUMES</p> <p>720(B2A1)+188(A3)+0()+0() = 908 vph</p> <p>Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)</p> <p style="text-align: center;"> 8 </p>																
<p>Step 3. IDENTIFY PHASING</p> <pre> --^ A1B2 --> --> <-- A1A2 A3B4 v > </pre>	<p>Step 6a. CRITICAL VOLUMES, in vph (two phase signal)</p> <pre> Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4 </pre>	<p>COMMENTS</p> <p>Exclusive right turns reduced 30 % V/C Ratio = .64</p>																

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER AND BRADSHAW

Design Hour: PM

Problem Statement: ALTA EXIST PLUS PROJECT

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP									
Approach 3: BRADSHAW RD 1 1 1 ^ R L N R T T T L T H H H T Approach 1 << >> ^--RT 1 LT--^ v v v <^--RTH 1 LTH--> <--TH TH--> <v-LTH 1 RTH-v> ^ ^ ^ v--LT 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 1 1 Approach 4: BRADSHAW RD		a.No. of change intervals/hour	: 0	: 0	: 0	: 0	b.LT capacity on change (vph)	: 0	: 0	: 0	: 0	Prob-able Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph
		c.G/C ratio	: 0	: 0	: 0	: 0	d.Opposing volume in vph	: 0	: 0	: 0	: 0	B4B3	73(B3)	205- 73= 132(B4)	73
		e.LT capacity on green (vph)	: 0	: 0	: 0	: 0	f.LT capacity in vph (b+e)	: 0	: 0	: 0	: 0	A3B4	132(B4)	750- 132= 618(A3)	132
		g.Left turn volume in vph	: 0	: 0	: 0	: 0	h.Is volume > cap. (g>f) ?	:	:	:	:	A3A4	618(A3) OR 339(A4)		618
												B2B1	37(B1)	532- 37= 495(B2)	37
												A1B2	495(B2)	355- 495= 0(A1)	495
												A1A2	441(A2) OR 0(A1)		441

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES		
Approach 3 3: LT= 205 2:RT= 191 TH= 750 TH= 250 RT= 610 v LT= 37 <--Approach 2		6 7 2	1 5 0	0 0 5	^+ 191 <+ 250 v- 37	205(B3B4)+618(A3)+532(B1B2)+441(A2) = 1796 vph
Approach 1--> 1:LT= 532 ^ 4: RT= 25 TH= 279 TH= 314 RT= 76 LT= 73 Approach 4		532 -^	279 +>	76 +v	< ^ > + + 3 7 1 2 3 4 5	Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F
				Step 9. RECALCULATE		
				Geometric Change: Signal Change: Volume Change:		

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
< B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				

Level Of Service Computation Report
1985 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 GERBER ROAD/VINEYARD ROAD

ALTAEPAM

Level Of Service: B

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Approach, Movement, Control, Rights, and Lanes.

Volume Module table with 12 columns for volume components and 4 columns for approach directions.

Adjusted Volume Module table with 4 columns for approach directions and rows for Grade, % Cycle/Cars, % Truck/Comb, PCE Adj, Cycl/Car PCE, Trck/Comb PCE, and Adj Vol.

Critical Gap Module table with 4 columns for approach directions and rows for Population, Run Speed, RT Rad/Ang, and Critical Gp.

Capacity Module table with 4 columns for approach directions and rows for Conflict Vol, Potent Cap., % Used Cap., Impedance, and Actual Cap.

Level Of Service Module table with 4 columns for approach directions and rows for Unused Cap., LOS by Move, Movement, Shared Cap., Unused Cap., and Shared LOS.

Level Of Service Computation Report
 1985 HCM Unsignalized Method (Base Volume Alternative)

 Intersection #1 GERBER ROAD/VINEYARD ROAD

ACTA EPPM

Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	1	0	1	1	0	0	1	0	1

Volume Module:

Base Vol:	76	27	3	24	36	29	52	140	73	2	183	30
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Bse:	76	27	3	24	36	29	52	140	73	2	183	30
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	80	28	3	25	38	31	55	147	77	2	193	32
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Final Vol.:	80	28	3	25	38	31	55	147	77	2	193	32

Adjusted Volume Module:

Grade:	0%			0%			0%			0%		
% Cycle/Cars:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
% Truck/Comb:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
PCE Adj:	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.00	1.00	1.10	1.00	1.00
Cycl/Car PCE:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Trck/Cmb PCE:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Adj Vol.:	88	31	3	28	42	34	60	147	77	2	193	32

Critical Gap Module: >> Population: 0 << >> Run Speed(E/W): 30 MPH <<

RT Rad/Ang:	20.0	ft/90.0	deg	20.0	ft/90.0	deg	20.0	ft/90.0	deg	20.0	ft/90.0	deg
Critical Gp:	7.0	6.5	5.5	7.0	6.5	5.5	5.5	xxxx	xxxxxx	5.5	xxxx	xxxxxx

Capacity Module:

Cnflict Vol:	535	467	186	521	489	208	224	xxxx	xxxxxx	224	xxxx	xxxxxx
Potent Cap.:	410	510	904	420	496	882	866	xxxx	xxxxxx	866	xxxx	xxxxxx
% Used Cap.:	21.4	6.1	0.4	6.6	8.4	3.8	7.0	xxxx	xxxxxx	0.3	xxxx	xxxxxx
Impedance:	xxxx	0.96	1.00	xxxx	0.95	0.98	0.96	xxxx	xxxxxx	1.00	xxxx	xxxxxx
Actual Cap.:	365	489	904	388	476	882	866	xxxx	xxxxxx	866	xxxx	xxxxxx

Level Of Service Module:

Unused Cap.:	277	457	901	360	434	848	806	xxxx	xxxxxx	863	xxxx	xxxxxx
LOS by Move:	C	*	*	B	A	A	A	*	*	A	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	512	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Unused Cap.:	xxxx	xxxx	477	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Shared LOS:	*	*	A	*	*	*	*	*	*	*	*	*

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN AND ELK GROVE FLORIN
Problem Statement: ALTA MIT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE FLO 1 2 2 ^ R L N R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Volume Critical in vph phase in vph B4B3 164(B4) 190- 164= 26(B3) 164 A4B3 26(B3) 312- 26= 286(A4) 26 A3A4 286(A4) OR 147(A3) 286 B2B1 98(B2) 205- 98= 107(B1) 98 A2B1 107(B1) 361- 107= 254(A2) 107 A1A2 254(A2) OR 252(A1) 254			
FLORIN Approach 1 << >> ^--RT 1 1 LT--^ v v v <^--RTH LTH--> <--TH 2 2 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN H H 2 2 1 Approach 4: ELK GROVE FLO									

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
3: LT= 297 Approach 3 TH= 294 2:RT= 516 RT= 91 v TH= 691 LT= 205 <--Approach 2		1 1 1 1 9 4 4 3 6 1 7 7 4 4 < v v v > 98 -^ 252 -> < < ^ ^ ^ > 252 -> 171 -v 1 1 3 3 1 9 5 1 1 6 0 6 2 2 1		190(B4B3)+286(A4)+205(B2B1)+254(A2) = 935 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) B	
Approach 1--> 1:LT= 98 ^ 4: RT= 161 TH= 503 TH= 624 RT= 171 LT= 345 Approach 4				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
< B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		Exclusive right turns reduced 30 % V/C Ratio = .68
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN AND ELK GROVE FLORIN
Problem Statement: ALTA MIT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE FLO 1 2 2 ^ R L N R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B4B3 142(B3) 403- 142= 261(B4) 142 A3B4 261(B4) 324- 261= 63(A3) 261 A3A4 206(A4) OR 63(A3) 206 B2B1 84(B2) 267- 84= 183(B1) 84 A2B1 183(B1) 398- 183= 215(A2) 183 A1A2 417(A1) OR 215(A2) 417			
FLORIN Approach 1 1 LT--^ LTH--> 2 TH--> RTH-v> 1 RT--v L L T R R T T H T T H H 2 2 1 Approach 4: ELK GROVE FLO									
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 732 TH= 647 RT= 130 Approach 3 2:RT= 569 TH= 737 LT= 267 <--Approach 2 Approach 1--> 1:LT= 84 TH= 834 RT= 376 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 1 3 3 3 4 3 2 2 2 0 0 4 4 9 3 < v v v > 84 -^ 417 -> 417 -> 376 -v 1 1 2 2 2 4 1 0 0 7 2 6 6 6 0				Step 7. SUM OF CRITICAL VOLUMES 403(B3B4)+206(A4)+267(B2B1)+417(A1) = 1293 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) E			
Step 3. IDENTIFY PHASING < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .94			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND WATERMAN RD
Problem Statement: ALTA E+P MIT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP				
Approach 3: WATERMAN RD R L N R T T T L T H H H T Approach 1 < < > > ^--RT LT--^ v v v <^--RTH LTH--^> 2 TH--> RTH--v> ^ ^ ^ v--LT 1 1 RT--v < < > > Approach 2 L L T R R T T H T T FLORIN RD H H 1 1 Approach 4: WATERMAN RD		a.No. of change intervals/hour	: 0	: 0	: 0	: 0	Prob-able Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph
		b.LT capacity on change (vph)	: 0	: 0	: 0	: 0	A2B1	67(B1) 393-	67= 326(A2) 67	
		c.G/C ratio	: 0	: 0	: 1	: 0	A1A2	326(A2) OR 204(A1)	326	
		d.Opposing volume in vph	: 785	: 0	: 143	: 0	A4B3	336(B3) OR 100(A4)	336	
		e.LT capacity on green (vph)	: 0	: 0	: 1057	: 0				
		f.LT capacity in vph (b+e)	: 0	: 0	: 1057	: 0				
		g.Left turn volume in vph	: 0	: 0	: 0	: 0				
		h.Is volume > cap. (g>f) ?	: NO	: NO	: NO	: NO				
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES				
Approach 3 3: LT= 0 2:RT= 0 TH= 0 TH= 785 RT= 0 v LT= 67 <--Approach 2 Approach 1--> 1:LT= 0 ^ 4: RT= 143 TH= 408 TH= 0 RT= 184 LT= 336 Approach 4		204 -> < > 204 -> 184 -v 3 1 3 4 6 3				393(B1A2)+336(B3)+0()+0() = 729 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) A				
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph				Step 9. RECALCULATE				
<-- A2B1 v-- --> <-- A1A2 < ^ A4B3 		(two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				Geometric Change: Signal Change: Volume Change:				
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >						Exclusive right turns reduced 30 % V/C Ratio = .51				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND WATERMAN RD
Problem Statement: ALTA E+P MIT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: WATERMAN RD		-----Approach-----				Possible Volume Adjusted			
R L N		: -1- -2- -3- -4-				Prob- Critical Volume Critical			
FLORIN RD R T T T L		a.No. of change : 0 0 0 0				able Volume to next Volume			
T H H H T		b.LT capacity on : 0 0 0 0				Phase in vph phase in vph			
-----		change (vph) :				-----			
Approach 1 < < > ^--RT		c.G/C ratio : 0 0 1 0				A2B1 104 (B1) 456- 104= 352 (A2) 104			
LT--^ v v v <^--RTH		d.Opposing volume : 912 0 78 0				A1A2 493 (A1) OR 352 (A2) 493			
LTH--^>		in vph :				A4B3 374 (B3) OR 55 (A4) 374			
2 TH-->		e.LT capacity on : 0 0 1122 0							
RTH--v>		green (vph) :							
1 RT--v < < > Approach 2		f.LT capacity in : 0 0 1122 0							
L L T R R		vph (b+e) :							
T T H T T		g.Left turn volume : 0 0 0 0							
H H		in vph :							
1 1		h.Is volume > cap. : NO NO :							
Approach 4: WATERMAN RD		(g>f) ? :							
-----		-----				-----			
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
Approach 3						597 (B1A1)+374 (B3)+0()+0()			
3: LT= 0 2:RT= 0						= 971 vph			
TH= 0 TH= 912						-----			
RT= 0 v LT= 104						Step 8. INTERSECTION LEVEL OF SERVICE			
-----						(compare step 7 with table 6)			
<--Approach 2						B			
-----						-----			
Approach 1-->						Step 9. RECALCULATE			
-----						Geometric Change:			
1:LT= 0 4: RT= 78		493 -> < >				Signal Change:			
TH= 986 TH= 0		493 ->				Volume Change:			
RT= 464 LT= 374		-----							
Approach 4									

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
<-- A2B1		Approach 3							
v--									
--> <-- A1A2									
< ^ A4B3									
		Approach 1							
-----		See Step 6b.							
		Approach 2							
-----		-----							
A1 --> A3 B1 v-- B3 <		Approach 4				Exclusive right turns reduced 30 %			
v ^						V/C Ratio = .68			
A2 <-- A4 B2 --^ B4 >									
-----		-----				-----			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN AND BRADSHAW
Problem Statement: ALTA E+P MIT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD 1 3 2 ^ R L N R T T T L T H H H T Approach 1 < < > ^--RT 1 1 LT--^ v v v <^--RTH LTH-^> <--TH 2 2 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 1 1 RT--v < < > Approach 2 L L T R R T T H T T FLORIN ROAD H H 2 2 1 Approach 4: BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B4B3 41(B4) 241- 41= 200(B3) 41 A4B3 200(B3) 671- 200= 471(A4) 200 A3A4 471(A4) OR 194(A3) 471 B2B1 92(B1) 196- 92= 104(B2) 92 A1B2 104(B2) 182- 104= 78(A1) 104 A1A2 181(A2) OR 78(A1) 181			
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
Approach 3 3: LT= 73 2:RT= 147 TH= 582 TH= 362 RT= 167 v LT= 92 <--Approach 2		1 1 1 1 6 9 9 9 3 4 7 4 4 4 3 1 < v v v > ^- 147 <- 181 v- 92				241(B4B3)+471(A4)+196(B1B2)+181(A2) = 1089 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) C			
Approach 1--> 1:LT= 196 ^ 4: RT= 116 TH= 224 TH=1342 RT= 260 LT= 437 Approach 4		196 -^ 112 -> 112 -> 260 -v < < ^ ^ > 2 1 6 6 1 4 9 7 7 1 1 7 1 1 6				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
< B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				Exclusive right turns reduced 30 % V/C Ratio = .79			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN AND BRADSHAW
Problem Statement: ALTA E+P MIT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD 1 3 2 ^ R L N R T T T L T H H H T Approach 1 < < > > ^--RT 1 1 LT--^ v v v <^--RTH LTH-^> <--TH 2 2 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 1 1 RT--v < < > > Approach 2 L L T R R T T H T T FLORIN ROAD H H 2 2 1 Approach 4: BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- Possible Volume Adjusted able Critical Carryover Critical Phase Volume to next Volume in vph phase in vph ----- B4B3 97(B4) 265- 97= 168(B3) 97 A4B3 168(B3) 385- 168= 217(A4) 168 A3A4 486(A3) OR 217(A4) 486 B2B1 133(B1) 258- 133= 125(B2) 133 A1B2 125(B2) 410- 125= 285(A1) 125 A1A2 285(A1) OR 149(A2) 285			
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
Approach 3 3: LT= 175 2:RT= 123 TH=1457 TH= 298 RT= 258 v LT= 133 <--Approach 2		2 4 4 4 5 8 8 8 7 9 8 6 6 6 9 7 < v v v > >				265(B4B3)+486(A3)+258(B1B2)+285(A1) = 1294 vph			
Approach 1--> 1:LT= 258 ^ 4: RT= 117 TH= 410 TH= 770 RT= 585 LT= 481 Approach 4		258 -^ 205 -> 205 -> 585 -v < < ^ ^ > 2 2 3 3 1 6 1 8 8 1 5 7 5 5 7				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- E ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
< B4B3 > { AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				Exclusive right turns reduced 30 % V/C Ratio = .94			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELK GROVE FLORIN AND GERBER
Problem Statement: ALTA MIT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
GERBER RD Approach 3: ELK GROVE FLO 1 2 2 ^ R L N R T T T L T H H H T Approach 1 << >> ^--RT 1 LT--^ v v v <^--RTH 1 LTH--^> <--TH 1 1 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 2 2 1 Approach 4: ELK GROVE FLO		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B4B3 84(B4) 414- 84= 330(B3) 84 A4B3 330(B3) 363- 330= 33(A4) 330 A3A4 176(A3) OR 33(A4) 176 B2B1 113(B2) 185- 113= 72(B1) 113 A2B1 72(B1) 410- 72= 338(A2) 72 A1A2 369(A1) OR 338(A2) 369			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 152 Approach 3 2:RT= 371 TH= 351 TH= 449 RT= 194 v LT= 185 <--Approach 2 Approach 1--> 1:LT= 113 ^ 4: RT= 153 TH= 369 TH= 725 RT= 269 LT= 752 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 1 1 1 ----- 9 7 7 6 8 4 6 6 8 4 < v v v > ^+ 371 <+ 39 <- 410 v- 185				Step 7. SUM OF CRITICAL VOLUMES 414(B4B3)+176(A3)+185(B2B1)+369(A1) = 1144 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) D Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. Approach 2 ----- Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .83			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELK GROVE FLORIN AND GERBER
Problem Statement: ALTA MIT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE FLO 1 2 2 ^ R L N R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph			
GERBER RD Approach 1 << >> ^-RT 1 LT--^ v v v <^--RTH 1 LTH--> <--TH 1 1 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 2 2 1 Approach 4: ELK GROVE FLO						B4B3 185(B3) 227- 185= 42(B4) 185 A3B4 42(B4) 367- 42= 325(A3) 42 A3A4 325(A3) OR 239(A4) 325 B2B1 222(B1) 238- 222= 16(B2) 222 A1B2 16(B2) 585- 16= 569(A1) 16 A1A2 569(A1) OR 383(A2) 569			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
3: LT= 411 TH= 733 RT= 232 Approach 3 2:RT= 298 TH= 467 LT= 222 <--Approach 2		2 3 3 1 2 3 6 6 8 2 2 7 7 5 7 < v v v >		227(B3B4)+325(A3)+238(B1B2)+569(A1) = 1359 vph	
Approach 1--> 1:LT= 238 TH= 502 RT= 835 Approach 4		238 -^ << ^ ^ ^ > 502 -> 835 -v 1 1 2 2 2 8 5 3 3 1 5 1 9 9 8		Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) E	
				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
< B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4		Exclusive right turns reduced 30 % V/C Ratio = .99
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER AND BRADSHAW
Problem Statement: ALTA E+P MIT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD 1 2 1 R L N		Approach -1- -2- -3- -4- a.No. of change : 0 0 0 0				Possible Volume Adjusted Critical Volume Critical Volume Carryover to next phase in vph			
GERBER RD R T T T L T H H H T		b.LT capacity on change (vph) : 0 0 0 0				Phase in vph			
Approach 1 < < > ^--RT 1 2 LT--^ v v v <^--RTH LTH--^ <--TH 1 1 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 2		c.G/C ratio : 0 0 0 0				B4B3 56(B3) 95- 56= 39(B4) 56			
Approach 2 < < > Approach 2 L L T R R T T H T T GERBER RD H H 1 2 1		d.Opposing volume in vph : 0 0 0 0				A3B4 39(B4) 240- 39= 201(A3) 39			
Approach 4: BRADSHAW RD		e.LT capacity on green (vph) : 0 0 0 0				A3A4 401(A4) OR 201(A3) 401			
		f.LT capacity in vph (b+e) : 0 0 0 0				B2B1 17(B1) 229- 17= 212(B2) 17			
		g.Left turn volume in vph : 0 0 0 0				A1B2 212(B2) 170- 212= 0(A1) 212			
		h.Is volume > cap. (g>f) ?				A1A2 231(A2) OR 0(A1) 231			
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
Approach 3 3: LT= 95 TH= 319 RT= 343		3 1 1 4 6 6 9 3 0 0 5 < v v v >				95(B3B4)+401(A4)+229(B1B2)+231(A2) = 956 vph			
Approach 2 2: RT= 151 TH= 231 LT= 29		^ - 151 < - 231 v - 14 v - 17				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) B			
Approach 1--> 1: LT= 416 TH= 170 RT= 54		229 -^ 187 -^ 170 -> 54 -v 4 4 5 0 0 3 6 1 1 1				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Approach 4									
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
< B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		Approach 3 Approach 1 See Step 6b. Approach 2							
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Approach 4				Exclusive right turns reduced 30 % V/C Ratio = .7			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER AND BRADSHAW
Problem Statement: ALTA E+P MIT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD		-----Approach-----				Possible Volume Adjusted			
1 2 1 ^		: -1 -2 -3 -4				Critical Volume Carryover Critical			
R L N		a.No. of change : 0 0 0 0				Prob- Critical Volume Carryover Critical			
GERBER RD	R T T T L	intervals/hour :				able Volume to next Volume			
T H H H T		b.LT capacity on : 0 0 0 0				Phase in vph phase in vph			
Approach 1 < < > > ^--RT 1		change (vph) :				-----			
2 LT--^ v v v <^--RTH		c.G/C ratio : 0 0 0 0				B4B3 73 (B3) 205- 73= 132 (B4) 73			
LTH-^>		d.Opposing volume : 0 0 0 0				A3B4 132 (B4) 427- 132= 295 (A3) 132			
1 TH-->		in vph :				A3A4 295 (A3) OR 157 (A4) 295			
RTH-v>		e.LT capacity on : 0 0 0 0				B2B1 21 (B1) 293- 21= 272 (B2) 21			
1 RT--v < < > > Approach 2		green (vph) :				A1B2 272 (B2) 279- 272= 7 (A1) 272			
L L T R R		f.LT capacity in : 0 0 0 0				A1A2 250 (A2) OR 7 (A1) 250			
T T H T T GERBER RD		vph (b+e) :							
H H		g.Left turn volume : 0 0 0 0							
1 2 1		in vph :							
Approach 4: BRADSHAW RD		h.Is volume > cap. :							
		(g>f) ? :							
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
Approach 3		6 3 3 2				205 (B3B4) + 295 (A3) + 293 (B1B2) + 250 (A2)			
3: LT= 205		1 7 7 0				-----			
TH= 750		0 5 5 5				^ - 191			
RT= 610						< - 250			
v		< v v >				v - 17			
-----		v - 21				Step 8. INTERSECTION LEVEL OF SERVICE			
<--Approach 2						(compare step 7 with table 6)			
Approach 1-->		293 ^				-----			
-----		239 ^				-----			
1: LT= 532		< ^ ^ >				-----			
TH= 279						Step 9. RECALCULATE			
RT= 76		279 ->				Geometric Change:			
Approach 4		76 -v				Signal Change:			
		1 1				Volume Change:			
		7 5 5 2							
		3 7 7 5							
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph				COMMENTS			
< B4B3		(two phase signal)							
AND < ^ A3B4 AND		Approach 3							
v > OR /OR A4B3		-----							
^ A3A4		Approach 1							
v		See Step 6b.							
--^ v-- B2B1		Approach 2							
--^ AND <-- A1B2 AND		-----							
--> OR v-- /OR A2B1									
--> <-- A1A2									
A1 --> A3 B1 v-- B3 <		Approach 4				Exclusive right turns reduced 30 %			
v ^						V/C Ratio = .76			
A2 <-- A4 B2 --^ B4 >									

APPENDIX C

**Existing Plus Dispersed Core Alternative
Level of Service Calculations**

Level Of Service Computation Report

1994 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #3 ELK GROVE FLORIN/FLORIN ROAD

Cycle (sec): 1 Critical Vol./Cap. (X): 4.319
 Loss Time (sec): 0 Average Delay (sec/veh): OVERFLOW
 Optimal Cycle: 0 Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound												
Movement:	L	T	R	L	T	R	L	T	R	L	T	R										
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign												
Rights:	Include			Include			Include			Include												
Lanes:	0	0	1	0	0	1	1	0	1	0	1	0	0	1	0	1	0	1	0	1	0	1

Volume Module:

Base Vol:	366	650	142	255	323	91	98	469	194	187	673	482
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Bse:	366	650	142	255	323	91	98	469	194	187	673	482
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	385	684	149	268	340	96	103	494	204	197	708	507
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	385	684	149	268	340	96	103	494	204	197	708	507
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Vol.:	385	684	149	268	340	96	103	494	204	197	708	507

Saturation Flow Module:

Sat/Lane:	282	282	282	287	287	287	249	249	249	275	275	275
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.32	0.56	0.12	1.00	1.00	1.00	0.26	1.23	0.51	1.00	1.00	1.00
Final Sat.:	89	158	34	287	287	287	64	307	127	275	275	275

Capacity Analysis Module:

Vol/Sat:	4.32	4.32	4.32	0.93	1.18	0.33	1.61	1.61	1.61	0.72	2.57	1.84
Crit Moves:	****			****			****			****		

Level Of Service Module:

Delay/Veh:	xxxxx	xxxx	xxxxx	34.8	90.2	3.6	451.3	451	451.3	15.2	xxxx	1103
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	xxxxx	xxxx	xxxxx	34.8	90.2	3.6	451.3	451	451.3	15.2	xxxx	1103
LOS by Move:	F	F	F	E	F	A	F	F	F	C	F	F

Level Of Service Computation Report
 1994 HCM 4-Way Stop Method (Base Volume Alternative)

 Intersection #3 ELK GROVE FLORIN/FLORIN ROAD

Cycle (sec): 1 Critical Vol./Cap. (X): 4.343
 Loss Time (sec): 0 Average Delay (sec/veh): OVERFLOW
 Optimal Cycle: 0 Level Of Service: F

Approach:	North Bound			South Bound			East Bound			West Bound												
Movement:	L	T	R	L	T	R	L	T	R	L	T	R										
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign												
Rights:	Include			Include			Include			Include												
Lanes:	0	0	1	0	0	1	1	0	1	0	1	0	0	1	0	1	0	1	0	1	0	1

Volume Module:

Base Vol:	288	469	241	655	707	130	84	778	425	250	690	491
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Bse:	288	469	241	655	707	130	84	778	425	250	690	491
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	303	494	254	689	744	137	88	819	447	263	726	517
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	303	494	254	689	744	137	88	819	447	263	726	517
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Vol.:	303	494	254	689	744	137	88	819	447	263	726	517

Saturation Flow Module:

Sat/Lane:	242	242	242	316	316	316	243	243	243	275	275	275
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.29	0.47	0.24	1.00	1.00	1.00	0.13	1.21	0.66	1.00	1.00	1.00
Final Sat.:	70	114	58	316	316	316	32	294	160	275	275	275

Capacity Analysis Module:

Vol/Sat:	4.34	4.34	4.34	2.18	2.35	0.43	2.79	2.79	2.79	0.96	2.64	1.88
Crit Moves:	****			****			****			****		

Level Of Service Module:

Delay/Veh:	xxxxx	xxxx	xxxxx	3966	7684	5.2	39610	xxxx	39610	37.9	xxxx	1266
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	xxxxx	xxxx	xxxxx	3966	7684	5.2	39610	xxxx	39610	37.9	xxxx	1266
LOS by Move:	F	F	F	F	F	B	F	F	F	E	F	F

ALTD

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND WATERMAN RD
Problem Statement: ALTD

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: WATERMAN RD R L N R T T T L T H H H T Approach 1 << >> ^--RT LT--^ v v v <^--RTH LTH--> <--TH 1 1 TH--> <v--LTH RTH--v ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 1 1 Approach 4: WATERMAN RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 1 0 d.Opposing volume : 697 0 153 0 in vph : e.LT capacity on : 0 0 1047 0 green (vph) : f.LT capacity in : 0 0 1047 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : NO NO (g>f) ? :				Prob- Possible Volume Adjusted able Critical Carryover Critical Phase Volume to next Volume in vph phase in vph ----- A2B1 80(B1) 697- 80= 617(A2) 80 A1A2 617(A2) OR 298(A1) 617 A4B3 346(B3) OR 107(A4) 346			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 0 TH= 0 RT= 0 Approach 3 2:RT= 0 TH= 697 LT= 80 <--Approach 2 Approach 1--> 1:LT= 0 TH= 298 RT= 199 Approach 4 4: RT= 153 TH= 0 LT= 346		Step 5. ASSIGN LANE VOLUMES, in vph <- 697 v- 80 < > 3 1 4 5 6 3				Step 7. SUM OF CRITICAL VOLUMES 697(B1A2)+346(B3)+0()+0() = 1043 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) C			
Step 3. IDENTIFY PHASING <-- A2B1 v-- --> <-- A1A2 < ^ A4B3 		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .73			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND WATERMAN RD
Problem Statement: ALTD

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP				
Approach 3: WATERMAN RD R L N R T T T L T H H H T Approach 1 << >> ^--RT LT--^ v v v <^--RTH LTH--> <--TH 1 1 TH--> <v--LTH RTH-v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 1 1 Approach 4: WATERMAN RD		a.No. of change intervals/hour	: -1- 0	-2- 0	-3- 0	-4- 0	Prob-able Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph
		b.LT capacity on change (vph)	: 0	0	0	0	A2B1	126(B1) 749-	126= 623(A2)	126
		c.G/C ratio	: 0	0	1	0	A1A2	817(A1) OR	623(A2)	817
		d.Opposing volume in vph	: 749	0	99	0	A4B3	382(B3) OR	69(A4)	382
		e.LT capacity on green (vph)	: 0	0	1101	0				
		f.LT capacity in vph (b+e)	: 0	0	1101	0				
		g.Left turn volume in vph	: 0	0	0	0				
		h.Is volume > cap. (g>f) ?	: NO		NO					

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3 3: LT= 0 2:RT= 0 TH= 0 TH= 749 RT= 0 v LT= 126 <--Approach 2 Approach 1--> 1:LT= 0 ^ 4: RT= 99 TH= 817 TH= 0 RT= 471 LT= 382 Approach 4		<- 749 v- 126		943(B1A1)+382(B3)+0()+0() = 1325 vph	
				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) E	
		<> 3 8 9 2 9		Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
<-- A2B1 v-- --> <-- A1A2 < ^ A4B3 		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				Exclusive right turns reduced 30 % V/C Ratio = .93

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN AND BRADSHAW
Problem Statement: ALTD

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP					
Approach 3: BRADSHAW RD 1 1 1 R L N R T T T L T H H H T Approach 1 1 LT--^ v v v <^--RTH 1 LTH--^ <--TH TH--> <v-LTH 1 RTH-v> ^ ^ ^ v--LT 1 RT--v < < > > Approach 2 L L T R R T H T T T FLORIN ROAD H H 1 1 Approach 4: BRADSHAW RD		a.No. of change intervals/hour	: 0	0	0	0	Prob-able Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph	
		b.LT capacity on change (vph)	: 0	0	0	0	B4B3	55 (B4) 402-	55= 347 (B3)	55	
		c.G/C ratio	: 0	0	0	0	A4B3	347 (B3) 1445-	347=1098 (A4)	347	
		d.Opposing volume in vph	: 0	0	0	0	A3A4	1098 (A4) OR 578 (A3)		1098	
		e.LT capacity on green (vph)	: 0	0	0	0	B2B1	84 (B1) 194-	84= 110 (B2)	84	
		f.LT capacity in vph (b+e)	: 0	0	0	0	A1B2	110 (B2) 397-	110= 287 (A1)	110	
		g.Left turn volume in vph	: 0	0	0	0	A1A2	509 (A2) OR 287 (A1)		509	
		h.Is volume > cap. (g>f) ?	:								
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES					
3: LT= 55 TH= 578 RT= 170 v Approach 3 2: RT= 147 TH= 362 LT= 84 <--Approach 2		1 5 7 7 5 0 8 5 < v >					402 (B4B3) + 1098 (A4) + 194 (B1B2) + 509 (A2) = 2203 vph				
Approach 1--> 1: LT= 194 TH= 175 RT= 222 Approach 4		194 -^ 175 +> 222 +v ----- 4 3 1 0 3 0 2 9 6					Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F				
		Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:									
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS					
< B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				Exclusive right turns reduced 30 % v/c = 1.60					
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >											

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN AND BRADSHAW
Problem Statement: ALTD

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD 1 1 1 ^ R L N R T T T L T H H H T Approach 1 << >> ^--RT 1 LT--^ v v v <^--RTH 1 LTH--> <--TH TH--> <v--LTH 1 RTH--v> ^ ^ ^ v--LT 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN ROAD H H 1 1 Approach 4: BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- Possible Volume Adjusted able Critical Carryover Critical Phase Volume to next Volume in vph phase in vph ----- B4B3 157(B4) 371- 157= 214(B3) 157 A4B3 214(B3) 861- 214= 647(A4) 214 A3A4 1454(A3) OR 647(A4) 1454 B2B1 123(B1) 224- 123= 101(B2) 123 A1B2 101(B2) 886- 101= 785(A1) 101 A1A2 785(A1) OR 371(A2) 785			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 157 Approach 3 2:RT= 100 TH=1454 TH= 271 RT= 267 v LT= 123 ----- <--Approach 2 ----- Approach 1--> ----- 1:LT= 224 ^ 4: RT= 109 TH= 362 TH= 752 RT= 524 LT= 371 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 1 2 4 1 ----- 6 5 5 7 4 7 ^+ 100 <+ 271 < v > v- 123 ----- 224 -^ < ^ > 362 +> + + 524 +v ----- 3 7 1 ----- 7 5 0 1 2 9				Step 7. SUM OF CRITICAL VOLUMES 371(B4B3)+1454(A3)+224(B1B2)+785(A1) = 2834 vph ----- Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- F ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 ----- --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS ----- Exclusive right turns reduced 30 % V/C Ratio = 2.06			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Level Of Service Computation Report

1985 HCM Unsignalized Method (Base Volume Alternative)

 Intersection #2 FLORIN/VINEYARD

Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound							
Movement:	L	T	R	L	T	R	L	T	R	L	T	R					
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled							
Rights:	Include			Include			Include			Include							
Lanes:	1	0	0	0	1	0	0	0	0	1	0	1	1	0	1	0	0

Volume Module:

Base Vol:	125	0	22	0	0	0	0	287	49	8	468	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Bse:	125	0	22	0	0	0	0	287	49	8	468	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	132	0	23	0	0	0	0	302	52	8	493	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Final Vol.:	132	0	23	0	0	0	0	302	52	8	493	0

Adjusted Volume Module:

Grade:	0%			0%			0%			0%		
% Cycle/Cars:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
% Truck/Comb:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
PCE Adj:	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.00	1.00	1.10	1.00	1.00
Cycl/Car PCE:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Trck/Cmb PCE:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx
Adj Vol.:	145	0	25	0	0	0	0	302	52	9	493	0

Critical Gap Module: >> Population: 0 << >> Run Speed(E/W): 30 MPH <<
 RT Rad/Ang: 20.0 ft/90.0 deg 20.0 ft/90.0 deg 20.0 ft/90.0 deg 20.0 ft/90.0 deg
 Critical Gp: 7.0 xxxx 5.5 xxxxxx xxxx xxxxxx xxxxxx xxxx xxxxxx 5.5 xxxx xxxxxx

Capacity Module:

Cnflct Vol:	803	xxxx	302	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	354	xxxx	xxxxxx
Potent Cap.:	264	xxxx	783	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	737	xxxx	xxxxxx
% Used Cap.:	54.9	xxxx	3.3	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	1.3	xxxx	xxxxxx
Impedance:	xxxx	xxxx	0.98	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	0.99	xxxx	xxxxxx
Actual Cap.:	262	xxxx	783	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	737	xxxx	xxxxxx

Level Of Service Module:

Unused Cap.:	117	xxxx	758	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	728	xxxx	xxxxxx
LOS by Move:	D	*	A	*	*	*	*	*	*	A	*	*
Movement:	LT - LTR - RT			LT - LTR - RT			LT - LTR - RT			LT - LTR - RT		
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Unused Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*

Level Of Service Computation Report
 1985 HCM Unsignalized Method (Base Volume Alternative)

 Intersection #2 FLORIN/VINEYARD

Level Of Service: E

Approach:	North Bound			South Bound			East Bound			West Bound						
Movement:	L	T	R	L	T	R	L	T	R	L	T	R				
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled						
Rights:	Include			Include			Include			Include						
Lanes:	1	0	0	0	1	0	0	0	0	0	0	1	0	1	0	0

Volume Module:

Base Vol:	88	0	14	0	0	0	0	484	144	23	406	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Bse:	88	0	14	0	0	0	0	484	144	23	406	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	93	0	15	0	0	0	0	509	152	24	427	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Final Vol.:	93	0	15	0	0	0	0	509	152	24	427	0

Adjusted Volume Module:

Grade:	0%			0%			0%			0%		
% Cycle/Cars:	xxxx	xxxx		xxxx	xxxx		xxxx	xxxx		xxxx	xxxx	
% Truck/Comb:	xxxx	xxxx		xxxx	xxxx		xxxx	xxxx		xxxx	xxxx	
PCE Adj:	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.00	1.00	1.10	1.00	1.00
Cycl/Car PCE:	xxxx	xxxx		xxxx	xxxx		xxxx	xxxx		xxxx	xxxx	
Trck/Cmb PCE:	xxxx	xxxx		xxxx	xxxx		xxxx	xxxx		xxxx	xxxx	
Adj Vol.:	102	0	16	0	0	0	0	509	152	27	427	0

Critical Gap Module: >> Population: 0 << >> Run Speed(E/W): 30 MPH <<
 RT Rad/Ang: 20.0 ft/90.0 deg 20.0 ft/90.0 deg 20.0 ft/90.0 deg 20.0 ft/90.0 deg
 Critical Gp: 7.0 xxxx 5.5 xxxxxx xxxx xxxxxx xxxxxx xxxx xxxxxx 5.5 xxxx xxxxxx

Capacity Module:

Cnflct Vol:	961	xxxx	509	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	661	xxxx	xxxxxx
Potent Cap.:	202	xxxx	621	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	512	xxxx	xxxxxx
% Used Cap.:	50.5	xxxx	2.6	xxxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxxx	5.2	xxxx	xxxxxx
Impedance:	xxxx	xxxx	0.98	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	0.97	xxxx	xxxxxx
Actual Cap.:	196	xxxx	621	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	512	xxxx	xxxxxx

Level Of Service Module:

Unused Cap.:	94	xxxx	605	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	486	xxxx	xxxxxx			
LOS by Move:	E	*	A	*	*	*	*	*	*	A	*	*			
Movement:	LT	-	LTR	-	RT	LT	-	LTR	-	RT	LT	-	LTR	-	RT
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx			
Unused Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx			
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELK GROVE FLORIN AND GERBER
Problem Statement: ALTD

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE FLO 1 1 ^ R L N R T T T L T H H H T Approach 1 << >> ^--RT 1 LT--^ v v v <^--RTH 1 LTH--> <--TH 1 1 TH--> <v--LTH RTH--v ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 1 1 Approach 4: ELK GROVE FLO		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B4B3 204(B4) 752- 204= 548(B3) 204 A4B3 548(B3) 875- 548= 327(A4) 548 A3A4 531(A3) OR 327(A4) 531 B2B1 102(B2) 187- 102= 85(B1) 102 A2B1 85(B1) 436- 85= 351(A2) 85 A1A2 374(A1) OR 351(A2) 374			
Step 2. IDENTIFY VOLUMES, in vph Approach 3 3: LT= 204 2:RT= 419 TH= 345 TH= 453 RT= 186 v LT= 187 <--Approach 2 Approach 1--> 1:LT= 102 ^ 4: RT= 158 TH= 374 TH= 717 RT= 269 LT= 752 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 1 3 2 ----- 8 4 0 ----- 6 5 4 ^+ 419 + + <+ 17 < v > <- 436 v- 187				Step 7. SUM OF CRITICAL VOLUMES 752(B4B3)+531(A3)+187(B2B1)+374(A1) = 1844 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F			
Step 3. IDENTIFY PHASING < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.34			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELK GROVE FLORIN AND GERBER
Problem Statement: ALTD

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE FLO 1 1 ^ R L N R T T T L T H H H T Approach 1 << >> ^--RT 1 LT--^ v v v <^--RTH 1 LTH-^> <--TH 1 1 TH--> <v--LTH RTH-v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 1 1 Approach 4: ELK GROVE FLO		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B4B3 336(B3) 520- 336= 184(B4) 336 A3B4 184(B4) 909- 184= 725(A3) 184 A3A4 725(A3) OR 697(A4) 725 B2B1 220(B2) 258- 220= 38(B1) 220 A2B1 38(B1) 448- 38= 410(A2) 38 A1A2 585(A1) OR 410(A2) 585			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 520 Approach 3 TH= 707 2:RT= 395 RT= 202 v TH= 500 LT= 258 <--Approach 2 Approach 1--> 1:LT= 220 ^ 4: RT= 231 TH= 510 TH= 466 RT= 835 LT= 336 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 2 7 5 ----- 0 0 2 2 7 0 ^+ 395 + + <+ 53 < v > <- 448 v- 258 220 -^ < ^ > 510 -> + + 835 -v 3 4 2 3 6 3 6 6 1				Step 7. SUM OF CRITICAL VOLUMES 520(B3B4)+725(A3)+258(B2B1)+585(A1) = 2088 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. Approach 2 ----- Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.52			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND WATERMAN RD
Problem Statement: ALTD

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: WATERMAN RD 1 1 ^ R L N R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 325 0 270 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : NO NO (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph A1B2 158(B2) 325- 158= 167(A1) 158 A1A2 492(A2) OR 167(A1) 492 A3B4 189(A3) OR 105(B4) 189			
Approach 1 << >> ^--RT 1 1 LT--^ v v v <^--RTH LTH--^ <--TH 1 1 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H Approach 4: WATERMAN RD									
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 105 Approach 3 TH= 0 2:RT= 89 RT= 270 v TH= 492 LT= 0 <--Approach 2 Approach 1--> 1:LT= 158 ^ 4: RT= 0 TH= 325 TH= 0 RT= 0 LT= 0 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 2 1 ----- 7 0 ----- 0 5 ^- 89 <- 492 <>				Step 7. SUM OF CRITICAL VOLUMES 650(B2A2)+189(A3)+0()+0() = 839 vph			
						Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) A			
						Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ A1B2 --> --> <-- A1A2 A3B4 v >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. Approach 2 ----- Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .59			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER AND BRADSHAW
Problem Statement: ALTD

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
GERBER RD Approach 3:BRADSHAW RD 1 1 1 ^ R L N R T T T L T H H H T Approach 1 << >> ^--RT 1 LT--^ v v v <^--RTH 1 LTH-^> <--TH TH--> <v-LTH 1 RTH-v> ^ ^ ^ v--LT 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 1 1 Approach 4:BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B4B3 65(B3) 92- 65= 27(B4) 65 A3B4 27(B4) 310- 27= 283(A3) 27 A3A4 821(A4) OR 283(A3) 821 B2B1 29(B1) 426- 29= 397(B2) 29 A1B2 397(B2) 237- 397= 0(A1) 397 A1A2 387(A2) OR 0(A1) 387			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 92 TH= 310 RT= 357 Approach 3 v 2:RT= 146 TH= 241 LT= 29 <--Approach 2 Approach 1-->		Step 5. ASSIGN LANE VOLUMES, in vph 3 3 5 1 9 7 0 2 < v > 426 -^ < ^ > 178 +> + + 59 +v 4: RT= 31 TH= 790 LT= 65 Approach 4 7 6 9 3 5 0 1				Step 7. SUM OF CRITICAL VOLUMES 92(B3B4)+821(A4)+426(B1B2)+387(A2) = 1726 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.26			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER AND BRADSHAW
Problem Statement: ALTD

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW RD 1 1 1 ^ R L N R T T T L T H H H T ----- Approach 1 << >> ^--RT 1 LT--^ v v v <^--RTH 1 LTH--> <--TH TH--> <v--LTH 1 RTH--v ^ ^ ^ v--LT 1 RT--v << >> Approach 2 L L T R R ----- T T H T T GERBER RD H H 1 1 Approach 4:BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Prob- Critical Carryover Critical able Volume to next Volume Phase in vph phase in vph ----- B4B3 0(B3) 198- 0= 198(B4) 0 A3B4 198(B4) 735- 198= 537(A3) 198 A3A4 537(A3) OR 321(A4) 537 B2B1 37(B1) 548- 37= 511(B2) 37 A1B2 511(B2) 381- 511= 0(A1) 511 A1A2 448(A2) OR 0(A1) 448			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 198 TH= 735 RT= 632 Approach 3 2:RT= 183 TH= 265 LT= 37 v <--Approach 2 Approach 1--> 1:LT= 548 TH= 294 RT= 87 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 6 7 1 ----- 3 3 9 2 5 8 < v > 548 -^ < ^ > 294 +> + + 87 +v ----- 2 9 2 0 6 5				Step 7. SUM OF CRITICAL VOLUMES 198(B3B4)+537(A3)+548(B1B2)+448(A2) = 1731 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- F ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.26			

Level Of Service Computation Report
 1985 HCM Unsignalized Method (Base Volume Alternative)

 Intersection #1 GERBER ROAD/VINEYARD ROAD

Level Of Service: B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	1	0	1	1	0	0	1	0	0

Volume Module:

Base Vol:	63	33	3	31	17	39	20	175	50	2	104	10
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Bse:	63	33	3	31	17	39	20	175	50	2	104	10
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	66	35	3	33	18	41	21	184	53	2	109	11
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Final Vol.:	66	35	3	33	18	41	21	184	53	2	109	11

Adjusted Volume Module:

Grade:	0%			0%			0%			0%		
% Cycle/Cars:	xxxx	xxxx		xxxx	xxxx		xxxx	xxxx		xxxx	xxxx	
% Truck/Comb:	xxxx	xxxx		xxxx	xxxx		xxxx	xxxx		xxxx	xxxx	
PCE Adj:	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.00	1.00	1.10	1.00	1.00
Cycl/Car PCE:	xxxx	xxxx		xxxx	xxxx		xxxx	xxxx		xxxx	xxxx	
Trck/Comb PCE:	xxxx	xxxx		xxxx	xxxx		xxxx	xxxx		xxxx	xxxx	
Adj Vol.:	73	38	3	36	20	45	23	184	53	2	109	11

Critical Gap Module: >> Population: 0 << >> Run Speed(E/W): 30 MPH <<

RT Rad/Ang:	20.0 ft/90.0 deg	20.0 ft/90.0 deg	20.0 ft/90.0 deg	20.0 ft/90.0 deg								
Critical Gp:	7.0	6.5	5.5	7.0	6.5	5.5	5.5	xxxx	xxxxxx	5.5	xxxx	xxxxxx

Capacity Module:

Cnflct Vol:	413	354	211	413	375	115	120	xxxx	xxxxxx	237	xxxx	xxxxxx
Potent Cap.:	491	587	879	491	573	982	976	xxxx	xxxxxx	853	xxxx	xxxxxx
% Used Cap.:	14.9	6.5	0.4	7.3	3.4	4.6	2.4	xxxx	xxxxxx	0.3	xxxx	xxxxxx
Impedance:	xxxx	0.96	1.00	xxxx	0.98	0.97	0.99	xxxx	xxxxxx	1.00	xxxx	xxxxxx
Actual Cap.:	461	579	879	465	565	982	976	xxxx	xxxxxx	853	xxxx	xxxxxx

Level Of Service Module:

Unused Cap.:	388	541	876	429	545	937	953	xxxx	xxxxxx	851	xxxx	xxxxxx
LOS by Move:	B	*	*	A	A	A	A	*	*	A	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	596	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Unused Cap.:	xxxx	xxxx	554	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx
Shared LOS:	*	*	A	*	*	*	*	*	*	*	*	*

Level Of Service Computation Report
 1985 HCM Unsignalized Method (Base Volume Alternative)

 Intersection #1 GERBER ROAD/VINEYARD ROAD

Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Uncontrolled			Uncontrolled		
Rights:	Include			Include			Include			Include		
Lanes:	1	0	0	1	0	1	1	0	0	1	0	0

Volume Module:

Base Vol:	79	24	3	18	32	30	48	142	77	2	185	25
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Bse:	79	24	3	18	32	30	48	142	77	2	185	25
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	83	25	3	19	34	32	51	149	81	2	195	26
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Final Vol.:	83	25	3	19	34	32	51	149	81	2	195	26

Adjusted Volume Module:

Grade:	0%			0%			0%			0%		
% Cycle/Cars:	xxxx	xxxx		xxxx	xxxx		xxxx	xxxx		xxxx	xxxx	
% Truck/Comb:	xxxx	xxxx		xxxx	xxxx		xxxx	xxxx		xxxx	xxxx	
PCE Adj:	1.10	1.10	1.10	1.10	1.10	1.10	1.10	1.00	1.00	1.10	1.00	1.00
Cycl/Car PCE:	xxxx	xxxx		xxxx	xxxx		xxxx	xxxx		xxxx	xxxx	
Trck/Cmb PCE:	xxxx	xxxx		xxxx	xxxx		xxxx	xxxx		xxxx	xxxx	
Adj Vol.:	91	28	3	21	37	35	56	149	81	2	195	26

Critical Gap Module: >> Population: 0 << >> Run Speed(E/W): 30 MPH <<
 RT Rad/Ang: 20.0 ft/90.0 deg 20.0 ft/90.0 deg 20.0 ft/90.0 deg 20.0 ft/90.0 deg
 Critical Gp: 7.0 6.5 5.5 7.0 6.5 5.5 5.5 xxxx xxxxx 5.5 xxxx xxxxx

Capacity Module:

Cnflct Vol:	529	464	190	519	491	208	221	xxxx	xxxxx	231	xxxx	xxxxx
Potent Cap.:	415	512	900	421	495	882	869	xxxx	xxxxx	859	xxxx	xxxxx
% Used Cap.:	22.1	5.4	0.4	4.9	7.5	3.9	6.4	xxxx	xxxxx	0.3	xxxx	xxxxx
Impedance:	xxxx	0.97	1.00	xxxx	0.96	0.98	0.96	xxxx	xxxxx	1.00	xxxx	xxxxx
Actual Cap.:	372	492	900	393	476	882	869	xxxx	xxxxx	859	xxxx	xxxxx

Level Of Service Module:

Unused Cap.:	280	464	897	372	439	847	813	xxxx	xxxxx	857	xxxx	xxxxx
LOS by Move:	C	*	*	B	A	A	A	*	*	A	*	*
Movement:	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT	LT	LTR	RT
Shared Cap.:	xxxx	xxxx	518	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
Unused Cap.:	xxxx	xxxx	487	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
Shared LOS:	*	*	A	*	*	*	*	*	*	*	*	*

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN AND ELK GROVE FLORIN
Problem Statement: ALTD MIT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE FLO 1 2 2 ^ R L N R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph ----- B4B3 141(B4) 201- 141= 60(B3) 141 A4B3 60(B3) 325- 60= 265(A4) 60 A3A4 265(A4) OR 162(A3) 265 B2B1 98(B2) 191- 98= 93(B1) 98 A2B1 93(B1) 337- 93= 244(A2) 93 A1A2 244(A2) OR 235(A1) 244			
Approach 1 << >> ^-RT 1 1 LT--^ v v v <^RTH LTH-^> 2 TH--> <--TH 2 RTH-v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R									
T T H T T FLORIN H H 2 2 1 Approach 4: ELK GROVE FLO									
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
Approach 3 3: LT= 255 TH= 323 RT= 91 v <--Approach 2		1 1 1 1 9 6 6 1 4 1 2 2 5 1 < v v >>				201(B4B3)+265(A4)+191(B2B1)+244(A2) = 901 vph			
Approach 1--> 1: LT= 98 ^ TH= 469 RT= 194 Approach 4		98 -^ 235 -> 235 -> 194 -v << ^ ^ ^> 2 1 3 3 1 0 6 2 2 4 1 5 5 5 2				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- B ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
< B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				Exclusive right turns reduced 30 % V/C Ratio = .66			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN AND ELK GROVE FLORIN
Problem Statement: ALTD MIT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP				
Approach 3: ELK GROVE FLO		Approach				Possible	Volume	Adjusted		
1 2 2 ^		-1-	-2-	-3-	-4-	Critical	Carryover	Critical		
R L N		a.No. of change	: 0	0	0	0	Phase	in vph	to next	Volume
R T T T L		intervals/hour	: 0	0	0	0				
T H H H T		b.LT capacity on	: 0	0	0	0				
Approach 1 < < > > ^--RT 1		change (vph)	: 0	0	0	0	B4B3	158(B3)	361- 158=	203(B4)
1 LT--^ v v v <^RTH		c.G/C ratio	: 0	0	0	0	A3B4	203(B4)	354- 203=	151(A3)
LTH-^>		d.Opposing volume	: 0	0	0	0	A3A4	235(A4)	OR	151(A3)
2 TH-->		in vph	: 0	0	0	0	B2B1	84(B2)	240- 84=	156(B1)
RTH-v>		e.LT capacity on	: 0	0	0	0	A2B1	156(B1)	345- 156=	189(A2)
1 RT--v < < > > v--LT 1		green (vph)	: 0	0	0	0	A1A2	389(A1)	OR	189(A2)
Approach 2		f.LT capacity in	: 0	0	0	0				
L L T R R		vph (b+e)	: 0	0	0	0				
T T H T T FLORIN		g.Left turn volume	: 0	0	0	0				
H H		in vph	: 0	0	0	0				
2 2 1		h.Is volume > cap.	: 0	0	0	0				
Approach 4: ELK GROVE FLO		(g>f) ?	:	:	:	:				

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
3: LT= 655		1 3 3 2 3		361(B3B4)+235(A4)+240(B2B1)+389(A1)	
TH= 707		3 5 5 9 6		= 1225 vph	
RT= 130		0 4 4 5 1			
v					
		< v v >			
				Step 8. INTERSECTION LEVEL OF SERVICE	
				(compare step 7 with table 6)	
				D	
Approach 1-->		84 ^			
		389 ->			
		< < ^ ^ >			
		425 -v			
				Step 9. RECALCULATE	
				Geometric Change:	
				Signal Change:	
				Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
< B4B3		Approach 3		
>				
AND < ^ A3B4 AND				
v > OR /OR A4B3				
^ A3A4				
v				
--^ v-- B2B1		Approach 1		
		See Step 6b.		
--^ AND <-- A1B2 AND		Approach 2		
--> OR v-- /OR A2B1				
--> <-- A1A2				
		Approach 4		
A1 --> A3 B1 v-- B3 <				
v ^				
A2 <-- A4 B2 --^ B4 >				Exclusive right turns reduced 30 %
				V/C Ratio = .89

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN AND BRADSHAW
Problem Statement: ALTD MIT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD		-----Approach-----				Possible Volume Adjusted			
1 3 2 ^		: -1- -2- -3- -4-				Critical Volume Carryover Critical			
R L N		a.No. of change : 0 0 0 0				able Volume to next Volume			
FLORIN ROAD R T T T L		b.LT capacity on : 0 0 0 0				Phase in vph phase in vph			
----- T H H H T -----		c.G/C ratio : 0 0 0 0				B4B3 31(B4) 221- 31= 190(B3) 31			
Approach 1 < < > > ^--RT 1		d.Opposing volume : 0 0 0 0				A4B3 190(B3) 670- 190= 480(A4) 190			
1 LT--^ v v v <^--RTH		in vph : 0 0 0 0				A3A4 480(A4) OR 193(A3) 480			
LTH-^>		e.LT capacity on : 0 0 0 0				B2B1 84(B1) 194- 84= 110(B2) 84			
2 TH-->		green (vph) : 0 0 0 0				A1B2 110(B2) 155- 110= 45(A1) 110			
RTH-v> ^ ^ ^ v--LT 1		f.LT capacity in : 0 0 0 0				A1A2 181(A2) OR 45(A1) 181			
1 RT--v < < > > Approach 2		g.Left turn volume : 0 0 0 0							
----- L L T R R -----		in vph : 0 0 0 0							
T T H T T FLORIN ROAD		h.Is volume > cap. : 0 0 0 0							
H H		(g>f) ? : 0 0 0 0							
2 2 1									
Approach 4: BRADSHAW RD									

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		1 1 1 1		221(B4B3)+480(A4)+194(B1B2)+181(A2)	
3: LT= 55 2:RT= 147		7 9 9 9 2 3		= 1076 vph	
TH= 578 TH= 362		0 3 3 3 5 1		-----	
RT= 170 v LT= 84				Step 8. INTERSECTION LEVEL OF SERVICE	
-----		< v v v > >		(compare step 7 with table 6)	
<--Approach 2				-----	
Approach 1-->		194 -^		C	
-----		88 ->		-----	
1:LT= 194 ^ 4: RT= 106		< < ^ ^ >		Step 9. RECALCULATE	
TH= 175 TH=1339				Geometric Change:	
RT= 222 LT= 402		2 2 2 2 -v		Signal Change:	
Approach 4		-----		Volume Change:	
		2 1 6 6 1			
		2 8 7 7 0			
		1 1 0 0 6			

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph		COMMENTS	
< B4B3		(two phase signal)			
AND < ^ A3B4 AND		Approach 3			
v > OR /OR A4B3					
^ A3A4		-----			
v		Approach 1			
--^ v-- B2B1		See Step 6b.			
--^ AND <-- A1B2 AND		Approach 2			
--> OR v-- /OR A2B1		-----			
--> <-- A1A2					

A1 --> A3 B1 v-- B3 <		Approach 4		Exclusive right turns reduced 30 %	
v ^				V/C Ratio = .78	
A2 <-- A4 B2 --^ B4 >					

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN AND BRADSHAW
Problem Statement: ALTD E+P MIT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD		Approach-----				Possible Volume Adjusted			
1 3 2 ^		: -1- -2- -3- -4-				Critical Volume Critical			
R L N		a.No. of change				Prob- Critical Volume Critical			
FLORIN ROAD R T T T L		intervals/hour				able Volume to next Volume			
T H H H T		b.LT capacity on				Phase in vph phase in vph			
Approach 1 < < > > ^--RT 1		change (vph)				-----			
1 LT--^ v v v <^--RTH		c.G/C ratio				B4B3 87(B4) 205- 87= 118(B3) 87			
LTH-^>		d.Opposing volume				A4B3 118(B3) 376- 118= 258(A4) 118			
2 TH-->		in vph				A3A4 485(A3) OR 258(A4) 485			
RTH-v> ^ ^ ^ v--LT 1		e.LT capacity on				B2B1 123(B1) 224- 123= 101(B2) 123			
1 RT--v < < > > Approach 2		green (vph)				A1B2 101(B2) 367- 101= 266(A1) 101			
L L T R R		f.LT capacity in				A1A2 266(A1) OR 136(A2) 266			
T T H T T FLORIN ROAD		vph (b+e)							
H H		g.Left turn volume				: 0 0 0 0			
2 2 1		in vph							
Approach 4: BRADSHAW RD		h.Is volume > cap.							
		(g>f) ?							
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
Approach 3		2 4 4 4				205(B4B3)+485(A3)+224(B1B2)+266(A1)			
3: LT= 157		6 8 8 8 7 8				-----			
TH=1454		7 5 5 5 1 7				^ - 100			
RT= 267						< - 136			
v		< v v v > >				< - 136			
						v - 123			
						Step 8. INTERSECTION LEVEL OF SERVICE			
						(compare step 7 with table 6)			
						D			
Approach 1-->		224 -^				-----			
		181 ->				< < ^ ^ >			
1:LT= 224		181 ->							
TH= 362		524 -v				-----			
RT= 524						2 1 3 3 1			
Approach 4						0 6 7 7 0			
						5 7 6 6 9			
						Step 9. RECALCULATE			
						Geometric Change:			
						Signal Change:			
						Volume Change:			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph				COMMENTS			
< B4B3		(two phase signal)							
AND < ^ A3B4 AND		Approach 3							
v > OR /OR A4B3									
^ A3A4									
v		Approach 1							
--^ v-- B2B1		See Step 6b.							
--^ AND <-- A1B2 AND		Approach 2							
--> OR v-- /OR A2B1									
--> <-- A1A2									
A1 --> A3 B1 v-- B3 <									
v ^		Approach 4				Exclusive right turns reduced 30 %			
A2 <-- A4 B2 --^ B4 >						V/C Ratio = .86			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELK GROVE FLORIN AND GERBER
Problem Statement: ALT E+P MIT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE FLO		-----Approach-----				Possible Volume Adjusted			
1 2 2 ^		: -1- -2- -3- -4-				Critical Volume to next phase			
R L N		a.No. of change intervals/hour				Prob- able Volume in vph			
GERBER RD R T T T L		: 0 0 0 0				Critical Volume to next phase			
----- T H H H T		b.LT capacity on change (vph)				Phase in vph			
Approach 1 < < > > ^--RT		: 0 0 0 0				B4B3 112(B4) 414- 112= 302(B3) 112			
1 LT--^ v v v <^--RTH 1		: 0 0 0 0				A4B3 302(B3) 359- 302= 57(A4) 302			
LTH-^> <--TH 1		: 0 0 0 0				A3A4 173(A3) OR 57(A4) 173			
1 TH--> <v-LTH		: 0 0 0 0				B2B1 102(B2) 187- 102= 85(B1) 102			
RTH-v> ^ ^ ^ v--LT 1		: 0 0 0 0				A2B1 85(B1) 436- 85= 351(A2) 85			
2 RT--v < < > > Approach 2		f.LT capacity in vph (b+e)				A1A2 374(A1) OR 351(A2) 374			
----- L L T R R		: 0 0 0 0							
T T H T T GERBER RD		: 0 0 0 0							
H H		g.Left turn volume in vph							
2 2 1		: 0 0 0 0							
Approach 4: ELK GROVE FLO		h.Is volume > cap. (g>f) ?							
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
Approach 3		1 1 1 1				414(B4B3)+173(A3)+187(B2B1)+374(A1)			
3: LT= 204		8 7 7 9 1				= 1148 vph			
TH= 345		6 3 3 2 2							
RT= 186		< v v > >				Step 8. INTERSECTION LEVEL OF SERVICE			
----- <--Approach 2						(compare step 7 with table 6)			

						D			
Approach 1-->		102 -^				-----			
		374 ->				Step 9. RECALCULATE			
1:LT= 102		< < ^ ^ >				Geometric Change:			
TH= 374						Signal Change:			
RT= 269		4 3 3 3 1				Volume Change:			
Approach 4		1 3 5 5 5							
		4 8 9 9 8							
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
< B4B3		Approach 3							
>									
AND < ^ A3B4 AND									
v > OR /OR A4B3									
^ A3A4									
v		Approach 1							
--^ v-- B2B1		See Step 6b.							
--^ AND <-- A1B2 AND		Approach 2							
--> OR v-- /OR A2B1									
--> <-- A1A2									

A1 --> A3 B1 v-- B3 <		Approach 4				Exclusive right turns reduced 30 %			
v ^						V/C Ratio = .83			
A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELK GROVE FLORIN AND GERBER
Problem Statement: ALTD E+P MIT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP					
Approach 3: ELK GROVE FLO 1 2 2 ^ R L N R T T L T H H H T Approach 1 < < > ^--RT 1 LT--^ v v v <^--RTH 1 LTH--^ <--TH 1 1 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 1 2 RT--v < < > Approach 2 L L T R R T T H T T GERBER RD H H 2 2 1 Approach 4: ELK GROVE FLO		a.No. of change intervals/hour	:	0	0	0	0	Prob-able Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph
		b.LT capacity on change (vph)	:	0	0	0	0	B4B3	185 (B3)	286 - 185 = 101 (B4)	185
		c.G/C ratio	:	0	0	0	0	A3B4	101 (B4)	354 - 101 = 253 (A3)	101
		d.Opposing volume in vph	:	0	0	0	0	A3A4	253 (A3) OR 233 (A4)		253
		e.LT capacity on green (vph)	:	0	0	0	0	B2B1	220 (B2)	258 - 220 = 38 (B1)	220
		f.LT capacity in vph (b+e)	:	0	0	0	0	A2B1	38 (B1)	448 - 38 = 410 (A2)	38
		g.Left turn volume in vph	:	0	0	0	0	A1A2	510 (A1) OR 410 (A2)		510
		h.Is volume > cap. (g>f) ?	:								
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES					
Approach 3 3: LT= 520 2:RT= 395 TH= 707 TH= 500 RT= 202 v LT= 258 <--Approach 2		2 3 3 2 2 0 5 5 3 8 2 4 4 4 6 < v v > >	286 (B3B4) + 253 (A3) + 258 (B2B1) + 510 (A1) = 1307 vph								
Approach 1--> 1:LT= 220 ^ 4: RT= 231 TH= 510 TH= 466 RT= 835 LT= 336 Approach 4		220 -^ 510 -> 418 -v 418 -v 1 1 2 2 2 8 5 3 3 3 5 1 3 3 1	Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) E								
		Step 9. RECALCULATE									
		Geometric Change: Signal Change: Volume Change:									
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS					
< B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		Approach 3 Approach 1 See Step 6b. Approach 2				Approach 4 Exclusive right turns reduced 30 % V/C Ratio = .95					
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >											

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER AND BRADSHAW
Problem Statement: ALTD E+P MIT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY			Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD			-----Approach-----				Possible Volume Adjusted			
1 2 1 ^			: -1- -2- -3- -4-				Critical Volume Critical			
R L N			a.No. of change : 0 0 0 0				Prob- Critical Volume to next phase in vph			
GERBER RD R T T T L			intervals/hour : 0 0 0 0				Phase in vph			
T H H H T			change (vph) : 0 0 0 0				B4B3 65(B3) 92- 65= 27(B4) 65			
Approach 1 < < > > ^--RT 1			c.G/C ratio : 0 0 0 0				A3B4 27(B4) 250- 27= 223(A3) 27			
2 LT--^ v v v <^--RTH			d.Opposing volume : 0 0 0 0				A3A4 395(A4) OR 223(A3) 395			
LTH-^>			e.LT capacity on : 0 0 0 0				B2B1 17(B1) 234- 17= 217(B2) 17			
1 TH-->			green (vph) : 0 0 0 0				A1B2 217(B2) 178- 217= 0(A1) 217			
RTH-v> ^ ^ ^ v--LT 2			f.LT capacity in : 0 0 0 0				A1A2 241(A2) OR 0(A1) 241			
1 RT--v < < > > Approach 2			vph (b+e) : 0 0 0 0							
L L T R R			g.Left turn volume : 0 0 0 0							
T T H T T GERBER RD			in vph :							
H H			h.Is volume > cap. :							
1 2 1			(g>f) ? :							
Approach 4: BRADSHAW RD										
Step 2. IDENTIFY VOLUMES, in vph			Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
Approach 3			3 1 1				92(B3B4)+395(A4)+234(B1B2)+241(A2)			
3: LT= 92			5 5 5 9				= 962 vph			
TH= 310			7 5 5 2							
RT= 357										
v			< v v >				Step 8. INTERSECTION LEVEL OF SERVICE			
			v- 14				(compare step 7 with table 6)			
			v- 17				B			
Approach 1-->			234 -^							
			192 -^							
1:LT= 426			< ^ ^ >				Step 9. RECALCULATE			
TH= 178							Geometric Change:			
RT= 59			178 ->				Signal Change:			
v			59 -v				Volume Change:			
Approach 4			3 3							
			6 9 9 3							
			5 5 5 1							
Step 3. IDENTIFY PHASING			Step 6a. CRITICAL VOLUMES, in vph				COMMENTS			
< B4B3			(two phase signal)							
>			Approach 3							
AND < ^ A3B4 AND										
v > OR /OR A4B3										
^ A3A4										
v			Approach 1							
--^ v-- B2B1			See Step 6b.							
--^ AND <-- A1B2 AND			Approach 2							
--> OR v-- /OR A2B1										
--> <-- A1A2										
A1 --> A3 B1 v-- B3 <										
v ^			Approach 4				Exclusive right turns reduced 30 %			
A2 <-- A4 B2 --^ B4 >							V/C Ratio = .7			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER AND BRADSHAW
Problem Statement: ALTD E+P MIT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP				
Approach 3: BRADSHAW RD 1 2 1 ^ R L N R T T T L T H H H T Approach 1 < < > ^--RT 1 2 LT--^ v v v <^--RTH LTH-^> <--TH 1 1 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v < < > Approach 2 L L T R R T T H T T GERBER RD H H 1 2 1 Approach 4: BRADSHAW RD		a.No. of change intervals/hour	: 0	: 0	: 0	: 0	Prob-able Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph
		b.LT capacity on change (vph)	: 0	: 0	: 0	: 0	B4B3	0(B3) 198-	0= 198(B4) 0	
		c.G/C ratio	: 0	: 0	: 0	: 0	A3B4	198(B4) 442-	198= 244(A3) 198	
		d.Opposing volume in vph	: 0	: 0	: 0	: 0	A3A4	244(A3) OR 148(A4)	244	
		e.LT capacity on green (vph)	: 0	: 0	: 0	: 0	B2B1	21(B1) 301-	21= 280(B2) 21	
		f.LT capacity in vph (b+e)	: 0	: 0	: 0	: 0	A1A2	280(B2) 294-	280= 14(A1) 280	
		g.Left turn volume in vph	: 0	: 0	: 0	: 0	A1A2	265(A2) OR 14(A1)	265	
		h.Is volume > cap. (g>f) ?	:	:	:	:	:	:	:	
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES				
Approach 3 3: LT= 198 2:RT= 183 TH= 735 TH= 265 RT= 632 v LT= 37 <--Approach 2 Approach 1--> 1:LT= 548 ^ 4: RT= 25 TH= 294 TH= 296 RT= 87 LT= 0 Approach 4		6 3 3 1	3 6 6 9	2 8 8 8	< v v v >	198(B3B4)+244(A3)+301(B1B2)+265(A2)	= 1008 vph			
		301 -^	247 -^	294 ->	87 -v	Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)				
		C				Step 9. RECALCULATE				
		Geometric Change:								
		Signal Change:								
		Volume Change:								
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph				COMMENTS				
< B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		(two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				Exclusive right turns reduced 30 % V/C Ratio = .73				

APPENDIX D

**Cumulative Plus Alternate Core Alternative
Level of Service Calculations**

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND ELK GROVE-FLORIN RD
Problem Statement: ALTA

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:ELK GROVE-FLO 1 3 2 ^ R L M R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph ----- B2B1 127(B1) 259- 127= 132(B2) 127 A1B2 132(B2) 190- 132= 58(A1) 132 A1A2 543(A2) OR 58(A1) 543 B4B3 44(B4) 308- 44= 264(B3) 44 A4B3 264(B3) 1100- 264= 836(A4) 264 A3A4 836(A4) OR 313(A3) 836			
FLORIN RD Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--^> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 2 3 1 Approach 4:ELK GROVE-FLO									

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES			
Approach 3 3: LT= 80 2:RT= 570 TH= 940 TH=1630 RT= 220 v LT= 230		2 3 3 3 ----- 2 1 1 1 3 4 0 3 3 3 6 4 < v v v v >		^- 570 <- 543 <- 543 <- 543 v- 104 v- 127 ----- 259 -^ 212 -^ 190 -> ----- 190 -> << ^ ^ ^ ^ > 190 -> 210 -v 1 1 1 ----- 3 2 1 1 1 0 5 0 0 0 9 8 2 0 0 0 0		259(B1B2)+543(A2)+308(B4B3)+836(A4) ----- = 1946 vph ----- Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- F ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	
Approach 1--> 1:LT= 470 ^ 4: RT= 90 TH= 570 TH=3300 RT= 210 LT= 560 Approach 4							

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND ELK GROVE-FLORIN RD
Problem Statement: ALTA

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE-FLO 1 3 2 ^ R L N R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph			
Approach 1 < < > > ^--RT 1 2 LT--^ v v v <^--RTH LTH--^ <--TH 3 3 TH--> <v--LTH RTH--v ^ ^ ^ v--LT 2 1 RT--v < < > > Approach 2 L L T R R FLORIN RD T T H T T H H 2 3 1 Approach 4: ELK GROVE-FLO						Prob- able Phase B2B1 121(B1) 138- 121= 17(B2) 121 A1B2 17(B2) 570- 17= 553(A1) 17 A1A2 553(A1) OR 310(A2) 553 B4B3 176(B3) 264- 176= 88(B4) 176 A3B4 88(B4) 817- 88= 729(A3) 88 A3A4 729(A3) OR 497(A4) 729			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3 3: LT= 480 TH=2450 RT= 570 v		2:RT= 220 TH= 930 LT= 220 <--Approach 2		5 8 8 8 2 2 7 1 1 1 1 6 0 7 7 7 6 4 < v v v >	
Approach 1--> 1:LT= 250 ^ TH=1710 RT= 400 Approach 4		4: RT= 90 TH=1490 LT= 320		^- 220 <- 310 <- 310 v- 99 v- 121 < < ^ ^ ^ ^ > 1 1 4 4 4 7 4 9 9 9 9 6 4 7 7 7 0	
				138(B1B2)+553(A1)+264(B3B4)+729(A3) = 1684 vph	
				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- F -----	
				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				Exclusive right turns reduced 30 % V/C Ratio = 1.22

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND WATERMAN RD
Problem Statement: ALTA

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP				
Approach 3: WATERMAN RD R L N R T T T L T H H H T Approach 1 << >> ^--RT LT--^ v v v <^--RTH LTH--> <--TH 3 3 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 2 1 Approach 4: WATERMAN RD		a.No. of change intervals/hour	: 0	: 0	: 0	: 0	Prob-able Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph
		b.LT capacity on change (vph)	: 0	: 0	: 0	: 0	A2B1	149(B1) 550- 149=	401(A2) 149	
		c.G/C ratio	: 0	: 0	: 1	: 0	A1A2	401(A2) OR 140(A1)	401	
		d.Opposing volume in vph	: 0	: 0	: 0	: 0	A4B3	567(B3) OR 406(A4)	567	
		e.LT capacity on green (vph)	: 0	: 0	: 0	: 0				
		f.LT capacity in vph (b+e)	: 0	: 0	: 0	: 0				
		g.Left turn volume in vph	: 0	: 0	: 0	: 0				
		h.Is volume > cap. (g>f) ?	:	:	:	:				
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES				
Approach 3 3: LT= 0 TH= 0 RT= 0 v <--Approach 2		<- 550 <- 550 <- 550 v- 122 v- 149 67 -> 67 -> 67 -> 200 -v << >> 5 4 5 6 6 8 7 4 0				550(B1A2)+567(B3)+0()+0() = 1117 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) C Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:				
Approach 1--> 1:LT= 0 TH= 200 RT= 200 ^ Approach 4		4: RT= 580 TH= 0 LT=1030				Step 3. IDENTIFY PHASING <-- A2B1 v-- --> <-- A1A2 < ^ A4B3 				
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .78				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND WATERMAN RD
Problem Statement: ALTA

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP					
Approach 3: WATERMAN RD		-----Approach-----				Possible Volume Adjusted					
R L N		: -1- -2- -3- -4-				Critical Volume					
FLORIN RD	R T T T L	a.No. of change intervals/hour	: 0	0	0	0	Prob-able Phase	Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph	
T H H H T		b.LT capacity on change (vph)	: 0	0	0	0					
Approach 1	<< >> ^--RT	c.G/C ratio	: 0	0	1	0	A2B1	220(B1)	173- 220=	0(A2)	220
	LT--^ v v v <^--RTH	d.Opposing volume in vph	: 520	0	300	0	A1A2	644(A1) OR	0(A2)	644	
3	TH--> <--TH 3	e.LT capacity on green (vph)	: 0	0	900	0	A4B3	336(B3) OR	210(A4)	336	
	LTH--> <v--LTH	f.LT capacity in vph (b+e)	: 0	0	900	0					
1	RT--v << >> Approach 2	g.Left turn volume in vph	: 0	0	0	0					
L L T R R		h.Is volume > cap. (g>f) ?	: NO		NO						
T T H T T		FLORIN RD									
H H											
2 1											
Approach 4: WATERMAN RD											

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3				864(B1A1)+336(B3)+0()+0()	
3: LT= 0	2:RT= 0	<- 173		= 1200 vph	
TH= 0	TH= 520	<- 173			
RT= 0	LT= 400	<- 173		Step 8. INTERSECTION LEVEL OF SERVICE	
v		v- 180		(compare step 7 with table 6)	
<--Approach 2		v- 220		D	
Approach 1-->	530 ->	<<>		Step 9. RECALCULATE	
1:LT= 0	4: RT= 300			Geometric Change:	
TH=1590	TH= 0	3 2 3		Signal Change:	
RT= 920	LT= 610	3 7 0		Volume Change:	
Approach 4		6 5 0			

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS	
<-- A2B1		Approach 3			
v--					
--> <-- A1A2		Approach 1			
< ^ A4B3		See Step 6b.			
		Approach 2			
A1 --> A3	B1 v-- B3 <	Approach 4		Exclusive right turns reduced 30 %	
A2 <-- A4	B2 --^ B4 >			V/C Ratio = .84	

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND BRADSHAW RD
Problem Statement: ALTA

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW RD 1 3 2 ^ R L N R T T T L T H H H T Approach 1 << >> ^-RT 1 2 LT--^ v v v <^-RTH LTH-^> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 2 3 1 Approach 4:BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph ----- B2B1 22(B1) 396- 22= 374(B2) 22 A1B2 374(B2) 60- 374= 0(A1) 374 A1A2 609(A2) OR 0(A1) 609 B4B3 6(B4) 457- 6= 451(B3) 6 A4B3 451(B3) 817- 451= 366(A4) 451 A3A4 366(A4) OR 189(A3) 366			
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
Approach 3 3: LT= 10 2:RT= 870 TH= 150 TH=1080 RT= 270 v LT= 40 ----- <--Approach 2 Approach 1-->		2 ----- 7 5 5 5 0 0 0 0 5 6 < v v v v > ----- 396 -^ 324 -^ 60 -> 60 -> 60 -> 60 -> 70 -v ----- 4 3 8 8 8 3 5 7 1 1 1 8 7 4 7 7 7 0				396(B1B2)+609(A2)+457(B4B3)+366(A4) ----- = 1828 vph ----- Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- F ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
--^ v-- B2B1 ----- --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 ----- < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v ----- A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				----- Exclusive right turns reduced 30 % V/C Ratio = 1.33			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND BRADSHAW RD
Problem Statement: ALTA

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP				
Approach 3:BRADSHAW RD 1 3 2 ^ R L N FLORIN RD R T T T L T H H H T ----- Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH-> <--TH 3 3 TH--> <v--LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R ----- T T H T T FLORIN RD H H 2 3 1 Approach 4:BRADSHAW RD		a.No. of change intervals/hour	: 0	: 0	: 0	: 0	Prob-able Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph
		b.LT capacity on change (vph)	: 0	: 0	: 0					
		c.G/C ratio	: 0	: 0	: 0	B2B1	154(B2)	187- 154= 33(B1)	154	
		d.Opposing volume in vph	: 0	: 0	: 0	A2B1	33(B1)	203- 33= 170(A2)	33	
		e.LT capacity on green (vph)	: 0	: 0	: 0	A1A2	497(A1) OR 170(A2)		497	
		f.LT capacity in vph (b+e)	: 0	: 0	: 0	B4B3	83(B3)	484- 83= 401(B4)	83	
		g.Left turn volume in vph	: 0	: 0	: 0	A3B4	401(B4)	827- 401= 426(A3)	401	
		h.Is volume > cap. (g>f) ?	:	:	:	A3A4	426(A3) OR 397(A4)		426	

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
3: LT= 880 Approach 3 TH=2480 2:RT= 290 RT= 600 v TH= 360 <--Approach 2 LT= 340 Approach 1--> 1:LT= 280 ^ 4: RT= 60 TH=1070 TH=1190 RT= 710 LT= 150 Approach 4		6 8 8 8 3 4		187(B2B1)+497(A1)+484(B3B4)+426(A3)	
		0 2 2 2 9 8		= 1594 vph	
		0 7 7 7 6 4	^ - 290		
			< - 120		
		< v v v >	< - 120	Step 8. INTERSECTION LEVEL OF SERVICE	(compare step 7 with table 6)
		154 -^	v - 153		
		126 -^	v - 187		
		357 ->			
		357 ->	< < ^ ^ ^ >		
		357 ->			
		710 -v		Step 9. RECALCULATE	
			3 3 3	Geometric Change:	
			8 6 9 9 9 6	Signal Change:	
			3 8 7 7 7 0	Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3		
		Approach 1	See Step 6b.	
		Approach 2		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Approach 4		
			Exclusive right turns reduced 30 % V/C Ratio = 1.16	

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND VINEYARD RD
Problem Statement: ALTA

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: VINEYARD RD R L N R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 1 0 d.Opposing volume in vph : 620 0 280 0 e.LT capacity on green (vph) : 0 0 920 0 f.LT capacity in vph (b+e) : 0 0 920 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? : NO NO				Possible Volume Adjusted Prob- Critical Volume Critical able Volume to next Volume Phase in vph phase in vph ----- A2B1 341(B1) 207- 341= 0(A2) 341 A1A2 476(A1) OR 0(A2) 476 A4B3 196(A4) OR 55(B3) 196			
Approach 1 << >> ^--RT LT--^ v v v <^--RTH LTH-^> <--TH 3 3 TH--> <v--LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 2 1 Approach 4: VINEYARD RD									

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3 3: LT= 0 2:RT= 0 TH= 0 TH= 620 RT= 0 v LT= 620 <--Approach 2		<- 207 <- 207 <- 207 v- 279 v- 341		817(B1A1)+196(A4)+0()+0() = 1013 vph	
Approach 1--> 1:LT= 0 ^ 4: RT= 280 TH= 460 TH= 0 RT= 680 LT= 100 Approach 4		153 -> 153 -> << > 153 -> 680 -v 2 5 4 8 5 5 0		Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- C ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
<-- A2B1 v-- --> <-- A1A2 < ^ A4B3 		Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4		Exclusive right turns reduced 30 % V/C Ratio = .71
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND ELK GROVE-FLORIN RD
Problem Statement: ALTA

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:ELK GROVE-FLO 1 3 2 ^ R L N R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph ----- B2B1 88(B1) 215- 88= 127(B2) 88 A1B2 127(B2) 210- 127= 83(A1) 127 A1A2 580(A2) OR 83(A1) 580 B4B3 143(B4) 440- 143= 297(B3) 143 A4B3 297(B3) 1110- 297= 813(A4) 297 A3A4 813(A4) OR 307(A3) 813			
GERBER RD Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--> <--TH 2 2 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v < < > > Approach 2 L L T R R T T H T T GERBER RD H H 2 3 1 Approach 4:ELK GROVE-FLO									
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 260 Approach 3 2:RT= 500 TH= 920 TH=1160 RT= 130 v LT= 160 <--Approach 2 Approach 1--> 1:LT= 390 ^ 4: RT= 70 TH= 400 TH=3330 RT= 300 LT= 800 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 1 3 3 3 1 1 3 0 0 0 1 4 0 7 7 7 7 3 < v v v v > 215 -^ 176 -^ 200 -> 200 -> 300 -v < < ^ ^ ^ ^ > 4 3 1 1 1 4 6 1 1 1 7 0 0 0 0 0 0				Step 7. SUM OF CRITICAL VOLUMES 215(B1B2)+580(A2)+440(B4B3)+813(A4) = 2048 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.49			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND ELK GROVE-FLORIN RD
Problem Statement: ALTA

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:ELK GROVE-FLO		-----Approach-----				Possible Volume Adjusted			
1 3 2 ^		: -1- -2- -3- -4-				Critical Volume Critical			
R L N		a.No. of change				Prob- able Phase			
R T T T L		: 0 0 0 0				Critical Volume in vph			
T H H H T		: 0 0 0 0				Volume to next phase			
Approach 1 < < > > ^--RT 1		: 0 0 0 0				B2B1 77(B2) 94- 77= 17(B1) 77			
2 LT--^ v v v <^--RTH		: 0 0 0 0				A2B1 17(B1) 315- 17= 298(A2) 17			
LTH--> <--TH 2		: 0 0 0 0				A1A2 595(A1) OR 298(A2) 595			
2 TH--> <v--LTH		: 0 0 0 0				B4B3 193(B3) 253- 193= 60(B4) 193			
RTH-v> ^ ^ ^ v--LT 2		: 0 0 0 0				A3B4 60(B4) 817- 60= 757(A3) 60			
1 RT--v < < > > Approach 2		: 0 0 0 0				A3A4 757(A3) OR 500(A4) 757			
L L T R R		f.LT capacity in vph (b+e)							
T T H T T GERBER RD		: 0 0 0 0							
H H		g.Left turn volume in vph							
2 3 1		: 0 0 0 0							
Approach 4:ELK GROVE-FLO		h.Is volume > cap. (g>f) ?							

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		3 8 8 8 2 2		94(B2B1)+595(A1)+253(B3B4)+757(A3)	
3: LT= 460		2 1 1 1 0 5		= 1699 vph	
TH=2450		0 7 7 7 7 3			
RT= 320 v				Step 8. INTERSECTION LEVEL OF SERVICE	
		< v v v v >		(compare step 7 with table 6)	
				F	
Approach 1-->		77 -^			
		63 -^			
		565 ->			
		< < ^ ^ ^ ^ >			
		565 ->			
		850 -v			
		1 1 5 5 5 1		Step 9. RECALCULATE	
		9 5 0 0 0 5		Geometric Change:	
		3 8 0 0 0 0		Signal Change:	
				Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS	
--^ v-- B2B1		Approach 3			
--^ AND <-- A1B2 AND					
--> OR v-- /OR A2B1					
--> <-- A1A2					
< B4B3		Approach 1			
>		See Step 6b.			
AND < ^ A3B4 AND		Approach 2			
v > OR /OR A4B3					
^ A3A4					
v					
A1 --> A3 B1 v-- B3 <		Approach 4		Exclusive right turns reduced 30 %	
A2 <-- A4 B2 --^ B4 >				V/C Ratio = 1.24	

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND WATERMAN RD
Problem Statement: ALTA

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: WATERMAN RD 1 2 2 ^ R L N GERBER RD R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 0 0 d.Opposing volume in vph : 0 0 0 0 e.LT capacity on green (vph) : 0 0 0 0 f.LT capacity in vph (b+e) : 0 0 0 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? :				Prob- able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph B2B1 33(B1) 50- 33= 17(B2) 33 A1B2 17(B2) 230- 17= 213(A1) 17 A1A2 485(A2) OR 213(A1) 485 B4B3 33(B4) 226- 33= 193(B3) 33 A4B3 193(B3) 565- 193= 372(A4) 193 A3A4 372(A4) OR 170(A3) 372			
Approach 1 << >> ^--RT 1 2 LT--^ v v v <^RTH LTH--> <--TH 2 2 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 2 2 1 Approach 4: WATERMAN RD									
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 60 TH= 340 RT= 160 Approach 3 2: RT= 170 TH= 970 LT= 60 <--Approach 2 Approach 1--> 1: LT= 90 TH= 460 RT= 80 Approach 4 4: RT= 240 TH= 1130 LT= 410		Step 5. ASSIGN LANE VOLUMES, in vph 1 1 1 ----- 6 7 7 2 3 0 0 0 7 3 < v v v > 50 -^ 41 -^ 230 -> < < ^ ^ ^ > 230 -> 80 -v ----- 2 1 5 5 2 2 8 6 6 4 6 5 5 5 0				Step 7. SUM OF CRITICAL VOLUMES 50(B1B2)+485(A2)+226(B4B3)+372(A4) ^- 170 <- 485 <- 485 v- 27 v- 33 = 1133 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- D ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .82			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND VINEYARD RD
Problem Statement: ALTA

Design Hour: PM

<p>Step 1. IDENTIFY LANE GEOMETRY</p> <p>Approach 3: VINEYARD RD 1 2 2 ^ R L N R T T T L T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--^> <--TH 2 2 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 2 2 1 Approach 4: VINEYARD RD</p>	<p>Step 4. LEFT TURN CHECK</p> <p>-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :</p>	<p>Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Prob-Phase</th> <th>Possible Critical Volume in vph</th> <th>Volume Carryover to next phase</th> <th>Adjusted Critical Volume in vph</th> </tr> </thead> <tbody> <tr> <td>B2B1</td> <td>61(B1)</td> <td>83- 61=</td> <td>22(B2) 61</td> </tr> <tr> <td>A1B2</td> <td>22(B2)</td> <td>140- 22=</td> <td>118(A1) 22</td> </tr> <tr> <td>A1A2</td> <td>118(A1) OR</td> <td>75(A2)</td> <td>118</td> </tr> <tr> <td>B4B3</td> <td>22(B4)</td> <td>39- 22=</td> <td>17(B3) 22</td> </tr> <tr> <td>A4B3</td> <td>17(B3)</td> <td>30- 17=</td> <td>13(A4) 17</td> </tr> <tr> <td>A3A4</td> <td>400(A3) OR</td> <td>13(A4)</td> <td>400</td> </tr> </tbody> </table>	Prob-Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph	B2B1	61(B1)	83- 61=	22(B2) 61	A1B2	22(B2)	140- 22=	118(A1) 22	A1A2	118(A1) OR	75(A2)	118	B4B3	22(B4)	39- 22=	17(B3) 22	A4B3	17(B3)	30- 17=	13(A4) 17	A3A4	400(A3) OR	13(A4)	400
Prob-Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph																											
B2B1	61(B1)	83- 61=	22(B2) 61																											
A1B2	22(B2)	140- 22=	118(A1) 22																											
A1A2	118(A1) OR	75(A2)	118																											
B4B3	22(B4)	39- 22=	17(B3) 22																											
A4B3	17(B3)	30- 17=	13(A4) 17																											
A3A4	400(A3) OR	13(A4)	400																											
<p>Step 2. IDENTIFY VOLUMES, in vph</p> <p>3: LT= 40 Approach 3 2:RT= 50 TH= 800 TH= 150 RT= 60 v LT= 110 ----- Approach 2 ----- Approach 1--> ----- 1:LT= 150 ^ 4: RT= 10 TH= 140 TH= 60 RT= 200 LT= 70 Approach 4 </p>	<p>Step 5. ASSIGN LANE VOLUMES, in vph</p> <p>4 4 ----- 6 0 0 1 2 0 0 0 8 2 < v v >> 83 -^ 68 -^ 70 -> 70 -> < < ^ ^ ^ > 200 -v ----- 3 3 3 3 1 9 2 0 0 0 </p>	<p>Step 7. SUM OF CRITICAL VOLUMES</p> <p>83(B1B2)+118(A1)+39(B4B3)+400(A3) = 640 vph</p> <p>Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)</p> <p style="text-align: center;">A</p> <p>Step 9. RECALCULATE</p> <p>Geometric Change: Signal Change: Volume Change:</p>																												
<p>Step 3. IDENTIFY PHASING</p> <p>--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v </p> <p>A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 > </p>	<p>Step 6a. CRITICAL VOLUMES, in vph (two phase signal)</p> <p>Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4</p>	<p>COMMENTS</p> <p>Exclusive right turns reduced 30 % V/C Ratio = .47</p>																												

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND BRADSHAW RD
Problem Statement: ALTA

Design Hour: PM

<p>Step 1. IDENTIFY LANE GEOMETRY</p> <p>Approach 3:BRADSHAW RD</p> <pre> 1 3 2 ^ R L N R T T T L T H H H T < < > > ^--RT 1 v v v <^-RTH LTH-^> <--TH 2 2 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v < < > > Approach 2 L L T R R T T H T T GERBER RD H H 2 3 1 Approach 4:BRADSHAW RD </pre>	<p>Step 4. LEFT TURN CHECK</p> <p>-----Approach-----</p> <pre> : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? : </pre>	<p>Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Prob-Phase</th> <th>Possible Critical Volume in vph</th> <th>Volume Carryover to next phase</th> <th>Adjusted Critical Volume in vph</th> </tr> </thead> <tbody> <tr> <td>B2B1</td> <td>88(B1)</td> <td>165- 88=</td> <td>77(B2)</td> </tr> <tr> <td>A1B2</td> <td>77(B2)</td> <td>270- 77=</td> <td>193(A1)</td> </tr> <tr> <td>A1A2</td> <td>245(A2) OR</td> <td>193(A1)</td> <td>245</td> </tr> <tr> <td>B4B3</td> <td>45(B4)</td> <td>61- 45=</td> <td>16(B3)</td> </tr> <tr> <td>A4B3</td> <td>16(B3)</td> <td>263- 16=</td> <td>247(A4)</td> </tr> <tr> <td>A3A4</td> <td>618(A3) OR</td> <td>247(A4)</td> <td>618</td> </tr> </tbody> </table>	Prob-Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph	B2B1	88(B1)	165- 88=	77(B2)	A1B2	77(B2)	270- 77=	193(A1)	A1A2	245(A2) OR	193(A1)	245	B4B3	45(B4)	61- 45=	16(B3)	A4B3	16(B3)	263- 16=	247(A4)	A3A4	618(A3) OR	247(A4)	618
Prob-Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph																											
B2B1	88(B1)	165- 88=	77(B2)																											
A1B2	77(B2)	270- 77=	193(A1)																											
A1A2	245(A2) OR	193(A1)	245																											
B4B3	45(B4)	61- 45=	16(B3)																											
A4B3	16(B3)	263- 16=	247(A4)																											
A3A4	618(A3) OR	247(A4)	618																											
<p>Step 2. IDENTIFY VOLUMES, in vph</p> <pre> 3: LT= 82 Approach 3 2:RT= 80 TH=1855 TH= 490 RT= 561 v LT= 160 <--Approach 2 Approach 1--> 1:LT= 300 ^ 4: RT= 200 TH= 540 TH= 790 RT= 300 LT= 110 Approach 4 </pre>	<p>Step 5. ASSIGN LANE VOLUMES, in vph</p> <pre> 5 6 6 6 6 1 1 1 3 4 1 8 8 8 7 5 < v v v > > 165 -^ 135 -^ 270 -> < < ^ ^ ^ ^ > 270 -> 300 -v 2 2 2 2 6 5 6 6 6 0 1 0 3 3 3 0 </pre>	<p>Step 7. SUM OF CRITICAL VOLUMES</p> <p>165(B1B2)+245(A2)+61(B4B3)+618(A3)</p> <p>= 1089 vph</p> <p>Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)</p> <p style="text-align: center;"> C </p> <p>Step 9. RECALCULATE</p> <p>Geometric Change: Signal Change: Volume Change:</p>																												
<p>Step 3. IDENTIFY PHASING</p> <pre> --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 > </pre>	<p>Step 6a. CRITICAL VOLUMES, in vph (two phase signal)</p> <p>Approach 3</p> <p>Approach 1</p> <p>See Step 6b.</p> <p>Approach 2</p> <p>Approach 4</p>	<p>COMMENTS</p> <p>Exclusive right turns reduced 30 % V/C Ratio = .79</p>																												

APPENDIX E

**Cumulative Plus Dispersed Core Alternative
Level of Service Calculations**

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND ELK GROVE-FLORIN RD
Problem Statement: ALTD

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:ELK GROVE-FLO 1 3 2 ^ R L N R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 121(B1) 286- 121= 165(B2) 121 A1B2 165(B2) 187- 165= 22(A1) 165 A1A2 520(A2) OR 22(A1) 520 B4B3 44(B4) 341- 44= 297(B3) 44 A4B3 297(B3) 1040- 297= 743(A4) 297 A3A4 743(A4) OR 320(A3) 743			
Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH-^> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 2 3 1 Approach 4:ELK GROVE-FLO									

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
3: LT= 80 TH= 960 RT= 220 v Approach 3 2:RT= 700 TH=1560 LT= 220 <--Approach 2		2 3 3 3 2 2 2 2 3 4 0 0 0 0 6 4 < v v v v >>		286(B1B2)+520(A2)+341(B4B3)+743(A4) = 1890 vph	
Approach 1--> 1:LT= 520 TH= 560 RT= 200 Approach 4 4: RT= 80 TH=3120 LT= 620		286 -^ 234 -^ 187 -> 187 -> 187 -> 200 -v << ^ ^ ^ ^ > 1 1 1 3 2 0 0 0 4 7 4 4 4 8 1 9 0 0 0 0		^- 700 <- 520 <- 520 <- 520 v- 99 v- 121 Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F	
				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND ELK GROVE-FLORIN RD
Problem Statement: ALTD

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:ELK GROVE-FLO 1 3 2 ^ R L N R T T T L T H H H T Approach 1 << >> ^-RT 1 2 LT--^ v v v <^-RTH LTH-^> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 2 3 1 Approach 4:ELK GROVE-FLO		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 121(B1) 143- 121= 22(B2) 121 A1B2 22(B2) 543- 22= 521(A1) 22 A1A2 521(A1) OR 310(A2) 521 B4B3 182(B3) 226- 182= 44(B4) 182 A3B4 44(B4) 850- 44= 806(A3) 44 A3A4 806(A3) OR 503(A4) 806			
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
3: LT= 410 Approach 3 TH=2550 2:RT= 230 RT= 570 v TH= 930 LT= 220 <--Approach 2 Approach 1--> 1:LT= 260 ^ 4: RT= 110 TH=1630 TH=1510 RT= 480 LT= 330 Approach 4		5 8 8 8 1 2 ----- 7 5 5 5 8 2 0 0 0 0 5 6 < v v v v > 143 -^ 117 -^ 543 -> 543 -> < < ^ ^ ^ ^ > 543 -> 480 -v ----- 1 1 5 5 5 1 8 4 0 0 0 1 2 9 3 3 3 0				143(B1B2)+521(A1)+226(B3B4)+806(A3) = 1696 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- F ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				Exclusive right turns reduced 30 % V/C Ratio = 1.23			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND WATERMAN RD
Problem Statement: ALTD

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: WATERMAN RD		-----Approach-----				Possible Volume Adjusted			
R L N		: -1- -2- -3- -4-				Critical Volume Adjusted			
R T T T L		a.No. of change intervals/hour				Prob- able Phase			
T H H H T		: 0 0 0 0				Critical Volume in vph			
-----		b.LT capacity on change (vph)				-----			
Approach 1 << >> ^--RT		: 0 0 1 0				A2B1 226(B1) 170- 226= 0(A2) 226			
LT--^ v v v <^--RTH		: 0 0 0 0				A1A2 667(A1) OR 0(A2) 667			
LTH-^>		c.G/C ratio				A4B3 369(B3) OR 259(A4) 369			
3 TH-->		d.Opposing volume in vph							
RTH-v>		e.LT capacity on green (vph)							
1 RT--v << >> ^--LT 2		: 0 0 0 0							
-----		f.LT capacity in vph (b+e)							
L L T R R		: 0 0 0 0							
T T H T T		g.Left turn volume in vph							
H H		: 0 0 0 0							
2 1		h.Is volume > cap. (g>f) ?							
Approach 4: WATERMAN RD									

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3				893(B1A1)+369(B3)+0()+0()	
3: LT= 0		2:RT= 0		= 1262 vph	
TH= 0		TH= 510			
RT= 0		LT= 410			
-----		-----		-----	
<--Approach 2		<- 170		Step 8. INTERSECTION LEVEL OF SERVICE	
		<- 170		(compare step 7 with table 6)	
		<- 170		-----	
		v- 185		D	
		v- 226		-----	
Approach 1-->		667 ->		-----	
		667 ->		Step 9. RECALCULATE	
		667 ->		Geometric Change:	
1:LT= 0		<< >>		Signal Change:	
TH=2000				Volume Change:	
RT= 900		3 3 3			
-----		6 0 7			
Approach 4		9 2 0			

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS	
<-- A2B1		Approach 3			
v--					
--> <-- A1A2					
< ^ A4B3		Approach 1			
		See Step 6b.			
		Approach 2			

A1 --> A3		Approach 4		Exclusive right turns reduced 30 %	
v ^				V/C Ratio = .89	
B1 v-- B3 <					
A2 <-- A4					
B2 --^ B4 >					

Critical Movement Analysis: PLANNING
Calculation Form 1

CUMULATIVE

Intersection: FLORIN RD AND BRADSHAW RD
Problem Statement: ALTD

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD 1 3 2 ^ R L N R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- Possible Critical Adjusted able Volume Carryover Critical Phase in vph to next Volume in vph phase in vph ----- B2B1 28(B1) 385- 28= 357(B2) 28 A1B2 357(B2) 63- 357= 0(A1) 357 A1A2 637(A2) OR 0(A1) 637 B4B3 6(B4) 451- 6= 445(B3) 6 A4B3 445(B3) 807- 445= 362(A4) 445 A3A4 362(A4) OR 240(A3) 362			
Approach 1 << >> ^-RT 1 2 LT--^ v v v <^-RTH LTH-> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 2 3 1 Approach 4: BRADSHAW RD									

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES	
Approach 3 3: LT= 10 2:RT= 910 TH= 720 TH=1070 RT= 280 v LT= 50 <--Approach 2		2 2 2 2 8 4 4 4 0 0 0 0 5 6 < v v v v > 385 -^ 315 -^ 63 -> 63 -> << ^ ^ ^ ^ > 63 -> 90 -v 4 3 8 8 8 3 5 6 0 0 0 8 1 9 7 7 7 0				385(B1B2)+637(A2)+451(B4B3)+362(A4) = 1835 vph	
Approach 1--> 1:LT= 700 ^ 4: RT= 380 TH= 190 TH=2420 RT= 90 LT= 820 Approach 4						Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- F ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND BRADSHAW RD
Problem Statement: ALTD

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP									
Approach 3:BRADSHAW RD 1 3 2 ^ R L N FLORIN RD R T T T L T H H H T Approach 1 << >> ^-RT 1 2 LT--^ v v v <^-RTH LTH-^> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 2 3 1 Approach 4:BRADSHAW RD		a.No. of change intervals/hour	: 0	: 0	: 0	: 0	b.LT capacity on change (vph)	: 0	: 0	: 0	: 0	Prob-able Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph
		c.G/C ratio	: 0	: 0	: 0	: 0	d.Opposing volume in vph	: 0	: 0	: 0	: 0	B2B1	127(B2)	182- 127= 55(B1)	127
		e.LT capacity on green (vph)	: 0	: 0	: 0	: 0	f.LT capacity in vph (b+e)	: 0	: 0	: 0	: 0	A2B1	55(B1)	182- 55= 127(A2)	55
		g.Left turn volume in vph	: 0	: 0	: 0	: 0	h.Is volume > cap. (g>f) ?	:	:	:	:	A1A2	518(A1) OR 127(A2)		518
												B4B3	66(B3)	457- 66= 391(B4)	66
												A3B4	391(B4)	817- 391= 426(A3)	391
												A3A4	426(A3) OR 383(A4)		426

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
3: LT= 830 TH=2450 RT= 610 Approach 3 2:RT= 260 TH= 340 LT= 330 <--Approach 2		6 8 8 8 3 4 1 1 1 1 7 5 0 7 7 7 4 7 < v v v >		182(B2B1)+518(A1)+457(B3B4)+426(A3) = 1583 vph	
Approach 1--> 1:LT= 230 TH=1060 RT= 740 Approach 4 4: RT= 60 TH=1150 LT= 120		127 -^ 104 -^ 353 -> 353 -> 353 -> 740 -v 3 3 3 6 5 8 8 8 6 6 4 3 3 3 0		^- 260 <- 113 <- 113 v- 149 v- 182 Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F	
				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		Exclusive right turns reduced 30 % V/C Ratio = 1.15
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND VINEYARD RD
Problem Statement: ALTD

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: VINEYARD RD		Approach				Possible Critical Volume			
R L N		-1- -2- -3- -4-				Carryover to next phase			
FLORIN RD		a.No. of change intervals/hour				Adjusted Critical Volume in vph			
R T T T L		: 0 0 0 0				Prob- able Phase			
T H H H T		b.LT capacity on change (vph)				72(B1) 180- 72= 108(A2) 72			
Approach 1 << >> ^-RT		: 0 0 0 0				A1A2 210(A1) OR 108(A2) 210			
LT--^ v v v <^-RTH		c.G/C ratio				A4B3 567(A4) OR 424(B3) 567			
LTH-^ <--TH 3		: 0 0 1 0							
3 TH--> <v-LTH		d.Opposing volume in vph							
RTH-v> ^ ^ ^ v--LT 2		: 540 0 810 0							
1 RT--v << >> Approach 2		e.LT capacity on green (vph)							
L L T R R		: 0 0 390 0							
T T H T T FLORIN RD		f.LT capacity in vph (b+e)							
H H		: 0 0 0 0							
2 1		g.Left turn volume in vph							
Approach 4: VINEYARD RD		h.Is volume > cap. (g>f) ?							
		: NO NO							

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3				282(B1A1)+567(A4)+0()+0()	
3: LT= 0		2:RT= 0		= 849 vph	
TH= 0		TH= 540			
RT= 0		LT= 130			
v				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)	
<--Approach 2				A	
Approach 1-->		210 ->			
		210 ->			
		<< >>		Step 9. RECALCULATE	
1:LT= 0		4: RT= 810		Geometric Change:	
TH= 630		TH= 0		Signal Change:	
RT= 20		LT= 770		Volume Change:	
Approach 4					
		4 3 8			
		2 4 1			
		4 7 0			

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
<-- A2B1		Approach 3		
v--				
--> <-- A1A2		Approach 1		See Step 6b.
< ^ A4B3				
		Approach 2		
A1 --> A3 B1 v-- B3 <		Approach 4		Exclusive right turns reduced 30 % V/C Ratio = .6
v ^				
A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND VINEYARD RD
Problem Statement: ALTD

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: VINEYARD RD R L N R T T T L T H H H T Approach 1 << >> ^--RT LT--^ v v v <^--RTH LTH-^> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 2 1 Approach 4: VINEYARD RD		-----Approach----- : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 1 0 d.Opposing volume in vph : 590 0 250 0 e.LT capacity on green (vph) : 0 0 950 0 f.LT capacity in vph (b+e) : 0 0 950 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? : NO NO :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph A2B1 347(B1) 197- 347= 0(A2) 347 A1A2 462(A1) OR 0(A2) 462 A4B3 175(A4) OR 50(B3) 175			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 0 TH= 0 RT= 0 Approach 3 v 2: RT= 0 TH= 590 LT= 630 <--Approach 2 Approach 1--> 1: LT= 0 TH= 500 RT= 660 Approach 4 4: RT= 250 TH= 0 LT= 90		Step 5. ASSIGN LANE VOLUMES, in vph <- 197 <- 197 <- 197 v- 284 v- 347 167 -> 167 -> 167 -> 660 -v << > 2 5 4 5 0 1 0				Step 7. SUM OF CRITICAL VOLUMES 809(B1A1)+175(A4)+0()+0() = 984 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) B Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING <-- A2B1 v-- --> <-- A1A2 < ^ A4B3 		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .69			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND ELK GROVE-FLORIN RD
Problem Statement: ALTD

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:ELK GROVE-FLO		-----Approach-----				Possible Volume Adjusted			
1 3 2 ^		: -1- -2- -3- -4-				Critical Volume Critical			
R L N		a.No. of change				Prob- able Volume to next phase in vph			
R T T T L		: 0 0 0 0				Volume in vph			
T H H H T		b.LT capacity on				B2B1 94(B1) 204- 94= 110(B2) 94			
-----		: 0 0 0 0				A1B2 110(B2) 220- 110= 110(A1) 110			
Approach 1 < < > > ^--RT 1		: 0 0 0 0				A1A2 570(A2) OR 110(A1) 570			
2 LT--^ v v v <^--RTH		: 0 0 0 0				B4B3 143(B4) 440- 143= 297(B3) 143			
LTH-^> <--TH 2		: 0 0 0 0				A4B3 297(B3) 1080- 297= 783(A4) 297			
2 TH--> <v-LTH		: 0 0 0 0				A3A4 783(A4) OR 307(A3) 783			
RTH-v> ^ ^ ^ v--LT 2		: 0 0 0 0							
1 RT--v < < > > Approach 2		f.LT capacity in							
L L T R R		: 0 0 0 0							
T T H T T GERBER RD		g.Left turn volume							
H H		: 0 0 0 0							
2 3 1		: 0 0 0 0							
Approach 4:ELK GROVE-FLO		h.Is volume > cap.							
		: (g>f) ?							

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		1 3 3 3 1 1		204(B1B2)+570(A2)+440(B4B3)+783(A4)	
3: LT= 260		2:RT= 510		^~ 510	
TH= 920		TH=1140		<- 570	
RT= 120		LT= 170		<- 570	
-----		< v v v >		v- 77	
				v- 94	
<--Approach 2				Step 8. INTERSECTION LEVEL OF SERVICE	
				(compare step 7 with table 6)	

				F	

Approach 1-->		204 -^			
		167 -^			
		220 ->		< < ^ ^ ^ >	
		220 ->			
1:LT= 370		4: RT= 90		1 1 1	
TH= 440		TH=3240		4 3 0 0 0	
RT= 300		LT= 800		4 6 8 8 8 9	
-----		-----		0 0 0 0 0 0	
Approach 4				Geometric Change:	
				Signal Change:	
				Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1		Approach 3		
--^ AND <-- A1B2 AND				
--> OR v-- /OR A2B1				
--> <-- A1A2				
< B4B3		Approach 1		See Step 6b.
>				
AND < ^ A3B4 AND		Approach 2		
v > OR /OR A4B3				
^ A3A4				
v				
-----		-----		Exclusive right turns reduced 30 % V/C Ratio = 1.45
A1 --> A3 B1 v-- B3 <		Approach 4		
A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND ELK GROVE-FLORIN RD
Problem Statement: ALTD

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE-FLO		-----Approach-----				Possible Volume Adjusted			
1 3 2 ^		: -1- -2- -3- -4-				Critical Volume Critical			
R L N		a.No. of change				Prob- able Phase			
R T T T L		: 0 0 0 0				Volume to next phase			
T H H H T		: 0 0 0 0				in vph			
Approach 1 << >> ^--RT 1		b.LT capacity on change (vph)				-----			
2 LT--^ v v v <^--RTH		: 0 0 0 0				B2B1 72(B2) 99- 72= 27(B1) 72			
LTH--> <--TH 2		: 0 0 0 0				A2B1 27(B1) 330- 27= 303(A2) 27			
2 TH--> <v-LTH		: 0 0 0 0				A1A2 595(A1) OR 303(A2) 595			
RTH-v> ^ ^ ^ v--LT 2		: 0 0 0 0				B4B3 193(B3) 253- 193= 60(B4) 193			
1 RT--v << >> Approach 2		: 0 0 0 0				A3B4 60(B4) 920- 60= 860(A3) 60			
L L T R R		f.LT capacity in vph (b+e)				A3A4 860(A3) OR 510(A4) 860			
T T H T T GERBER RD		: 0 0 0 0							
H H		g.Left turn volume in vph							
2 3 1		: 0 0 0 0							
Approach 4: ELK GROVE-FLO		h.Is volume > cap. (g>f) ?							

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		3 9 9 9 2 2		99(B2B1)+595(A1)+253(B3B4)+860(A3)	
3: LT= 460		2 2 2 2 0 5		= 1807 vph	
TH=2760		0 0 0 0 7 3			
RT= 320		< v v v v >		-----	
				^-- 360	
				<-- 330	
				<-- 330	
				v-- 81	
				v-- 99	
				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)	

				F	

Approach 1-->		72 -^			
		59 -^			
		550 ->			
		<< ^ ^ ^ ^ >			
		550 ->			
		850 -v			

		1 1 5 5 5		Step 9. RECALCULATE	
		9 5 1 1 1 9		Geometric Change:	
		3 8 0 0 0 0		Signal Change:	
				Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1		Approach 3		
--^ AND <-- A1B2 AND				
--> OR v-- /OR A2B1				
--> <-- A1A2				
< B4B3		Approach 1		
>		See Step 6b.		
AND < ^ A3B4 AND		Approach 2		
v > OR /OR A4B3				
^ A3A4				
v				

A1 --> A3 B1 v-- B3 <		Approach 4		
v ^				
A2 <-- A4 B2 --^ B4 >				Exclusive right turns reduced 30 % V/C Ratio = 1.31

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND WATERMAN RD
Problem Statement: ALTD

Design Hour: AM

<p>Step 1. IDENTIFY LANE GEOMETRY</p> <p>Approach 3: WATERMAN RD</p> <pre> 1 2 2 ^ R L N R T T T L T H H H T << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--> <--TH 2 2 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 2 2 1 Approach 4: WATERMAN RD </pre>	<p>Step 4. LEFT TURN CHECK</p> <p>-----Approach-----</p> <pre> : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? : </pre>	<p>Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Prob-Phase</th> <th>Possible Critical Volume in vph</th> <th>Volume Carryover to next phase</th> <th>Adjusted Critical Volume in vph</th> </tr> </thead> <tbody> <tr> <td>B2B1</td> <td>33(B1)</td> <td>66- 33=</td> <td>33(B2) 33</td> </tr> <tr> <td>A1B2</td> <td>33(B2)</td> <td>240- 33=</td> <td>207(A1) 33</td> </tr> <tr> <td>A1A2</td> <td>470(A2) OR</td> <td>207(A1)</td> <td>470</td> </tr> <tr> <td>B4B3</td> <td>33(B4)</td> <td>231- 33=</td> <td>198(B3) 33</td> </tr> <tr> <td>A4B3</td> <td>198(B3)</td> <td>545- 198=</td> <td>347(A4) 198</td> </tr> <tr> <td>A3A4</td> <td>347(A4) OR</td> <td>175(A3)</td> <td>347</td> </tr> </tbody> </table>	Prob-Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph	B2B1	33(B1)	66- 33=	33(B2) 33	A1B2	33(B2)	240- 33=	207(A1) 33	A1A2	470(A2) OR	207(A1)	470	B4B3	33(B4)	231- 33=	198(B3) 33	A4B3	198(B3)	545- 198=	347(A4) 198	A3A4	347(A4) OR	175(A3)	347
Prob-Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph																											
B2B1	33(B1)	66- 33=	33(B2) 33																											
A1B2	33(B2)	240- 33=	207(A1) 33																											
A1A2	470(A2) OR	207(A1)	470																											
B4B3	33(B4)	231- 33=	198(B3) 33																											
A4B3	198(B3)	545- 198=	347(A4) 198																											
A3A4	347(A4) OR	175(A3)	347																											
<p>Step 2. IDENTIFY VOLUMES, in vph</p> <pre> 3: LT= 60 Approach 3 TH= 350 RT= 180 v 2: RT= 200 TH= 940 LT= 60 <--Approach 2 Approach 1--> 1: LT= 120 ^ TH= 480 RT= 70 Approach 4 4: RT= 260 TH=1090 LT= 420 </pre>	<p>Step 5. ASSIGN LANE VOLUMES, in vph</p> <pre> 1 1 1 8 7 7 2 3 0 5 5 7 3 < v v v > 66 -^ 54 -^ 240 -> < < ^ ^ ^ > 240 -> 70 -v 2 1 5 5 2 3 8 4 4 6 1 9 5 5 0 </pre>	<p>Step 7. SUM OF CRITICAL VOLUMES</p> <p>66(B1B2)+470(A2)+231(B4B3)+347(A4)</p> <p>= 1114 vph</p> <p>Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)</p> <p style="text-align: center;">D</p> <p>Step 9. RECALCULATE</p> <p>Geometric Change: Signal Change: Volume Change:</p>																												
<p>Step 3. IDENTIFY PHASING</p> <pre> --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 > </pre>	<p>Step 6a. CRITICAL VOLUMES, in vph (two phase signal)</p> <p>Approach 3</p> <p>Approach 1</p> <p style="text-align: center;">See Step 6b.</p> <p>Approach 2</p> <p>Approach 4</p>	<p>COMMENTS</p> <p>Exclusive right turns reduced 30 % V/C Ratio = .81</p>																												

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND BRADSHAW RD
Problem Statement: ALTD

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY	Step 4. LEFT TURN CHECK	Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP
<p>Approach 3:BRADSHAW RD</p> <pre> 1 3 2 ^ R L N R T T T L T H H H T < < > > ^--RT 1 2 LT--^ v v v <^--RTH LTH-^> <--TH 2 2 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v < < > > Approach 2 L L T R R T T H T T GERBER RD H H 2 3 1 Approach 4:BRADSHAW RD </pre>	<p>-----Approach-----</p> <pre> : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? : </pre>	<p>Possible Volume Adjusted Critical Volume to next Phase in vph</p> <pre> B2B1 88(B1) 231- 88= 143(B2) 88 A1B2 143(B2) 150- 143= 7(A1) 143 A1A2 320(A2) OR 7(A1) 320 B4B3 22(B4) 160- 22= 138(B3) 22 A4B3 138(B3) 647- 138= 509(A4) 138 A3A4 509(A4) OR 163(A3) 509 </pre>
<p>Step 2. IDENTIFY VOLUMES, in vph</p> <pre> 3: LT= 40 Approach 3 TH= 490 2:RT= 200 RT= 220 v TH= 640 LT= 160 <--Approach 2 Approach 1--> 1:LT= 420 ^ 4: RT= 110 TH= 300 TH=1940 RT= 100 LT= 290 Approach 4 </pre>	<p>Step 5. ASSIGN LANE VOLUMES, in vph</p> <pre> 2 1 1 1 2 6 6 6 1 2 0 3 3 3 8 2 < v v v > > </pre>	<p>Step 7. SUM OF CRITICAL VOLUMES</p> <pre> 231(B1B2)+320(A2)+160(B4B3)+509(A4) = 1220 vph </pre>
		<p>Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)</p> <pre> D </pre>
		<p>Step 9. RECALCULATE</p> <p>Geometric Change: Signal Change: Volume Change:</p>
<p>Step 3. IDENTIFY PHASING</p> <pre> --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v </pre>	<p>Step 6a. CRITICAL VOLUMES, in vph (two phase signal)</p> <pre> Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4 </pre>	<p>COMMENTS</p> <p>Exclusive right turns reduced 30 % V/C Ratio = .89</p>

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND BRADSHAW RD
Problem Statement: ALTD

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW RD 1 3 2 ^ R L N GERBER RD R T T T L T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--> <--TH 2 2 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 2 3 1 Approach 4:BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph ----- B2B1 88(B1) 176- 88= 88(B2) 88 A1B2 88(B2) 280- 88= 192(A1) 88 A1A2 205(A2) OR 192(A1) 205 B4B3 83(B3) 105- 83= 22(B4) 83 A3B4 22(B4) 623- 22= 601(A3) 22 A3A4 601(A3) OR 263(A4) 601			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 190 Approach 3 2:RT= 70 TH=1870 TH= 410 RT= 570 v LT= 160 ----- <--Approach 2 Approach 1--> 1:LT= 320 ^ 4: RT= 200 TH= 560 TH= 790 RT= 370 LT= 150 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 5 6 6 6 1 ----- 7 2 2 2 8 0 0 3 3 3 6 5 < v v v v > ----- 176 -^ 144 -^ 280 -> 280 -> 370 -v ----- 2 2 2 2 8 6 6 6 6 0 3 8 3 3 3 0				Step 7. SUM OF CRITICAL VOLUMES 176(B1B2)+205(A2)+105(B3B4)+601(A3) ----- = 1087 vph ----- Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- C ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v ----- A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS ----- Exclusive right turns reduced 30 % V/C Ratio = .79			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND VINEYARD RD
Problem Statement: ALTD

Design Hour: AM

<p>Step 1. IDENTIFY LANE GEOMETRY</p> <p>Approach 3: VINEYARD RD</p> <pre> 1 2 2 ^ R L N R T T T L T H H H T << >> ^--RT 1 v v v <^-RTH LTH-^> <--TH 2 2 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 2 2 1 Approach 4: VINEYARD RD </pre>	<p>Step 4. LEFT TURN CHECK</p> <p>-----Approach-----</p> <pre> : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? : </pre>	<p>Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Prob-Phase</th> <th>Possible Critical Volume in vph</th> <th>Volume Carryover to next phase</th> <th>Adjusted Critical Volume in vph</th> </tr> </thead> <tbody> <tr> <td>B2B1</td> <td>6(B1)</td> <td>281- 6= 275(B2)</td> <td>6</td> </tr> <tr> <td>A1B2</td> <td>275(B2)</td> <td>80- 275= 0(A1)</td> <td>275</td> </tr> <tr> <td>A1A2</td> <td>112(A2) OR 0(A1)</td> <td></td> <td>112</td> </tr> <tr> <td>B4B3</td> <td>6(B4)</td> <td>77- 6= 71(B3)</td> <td>6</td> </tr> <tr> <td>A4B3</td> <td>71(B3)</td> <td>530- 71= 459(A4)</td> <td>71</td> </tr> <tr> <td>A3A4</td> <td>459(A4) OR 70(A3)</td> <td></td> <td>459</td> </tr> </tbody> </table>	Prob-Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph	B2B1	6(B1)	281- 6= 275(B2)	6	A1B2	275(B2)	80- 275= 0(A1)	275	A1A2	112(A2) OR 0(A1)		112	B4B3	6(B4)	77- 6= 71(B3)	6	A4B3	71(B3)	530- 71= 459(A4)	71	A3A4	459(A4) OR 70(A3)		459
Prob-Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph																											
B2B1	6(B1)	281- 6= 275(B2)	6																											
A1B2	275(B2)	80- 275= 0(A1)	275																											
A1A2	112(A2) OR 0(A1)		112																											
B4B3	6(B4)	77- 6= 71(B3)	6																											
A4B3	71(B3)	530- 71= 459(A4)	71																											
A3A4	459(A4) OR 70(A3)		459																											
<p>Step 2. IDENTIFY VOLUMES, in vph</p> <p>3: LT= 10 TH= 10 RT= 100</p> <p>Approach 3</p> <p>2: RT= 160 TH= 180 LT= 10</p> <p>Approach 2</p> <p>Approach 1--></p> <p>1: LT= 510 TH= 160 RT= 30</p> <p>Approach 4</p> <p>4: RT= 20 TH= 1060 LT= 140</p>	<p>Step 5. ASSIGN LANE VOLUMES, in vph</p> <pre> 1 0 0 5 5 5 6 < v v v > </pre> <p>281 -^ 230 -^ 80 -> 80 -> 30 -v</p> <p>Approach 1</p> <pre> < < ^ ^ ^ > 5 5 7 6 3 3 2 7 3 0 0 0 </pre>	<p>Step 7. SUM OF CRITICAL VOLUMES</p> <p>281(B1B2)+112(A2)+77(B4B3)+459(A4)</p> <p>= 929 vph</p> <p>Step 8. INTERSECTION LEVEL OF SERVICE</p> <p>(compare step 7 with table 6)</p> <p style="text-align: center;"> B </p> <p>Step 9. RECALCULATE</p> <p>Geometric Change: Signal Change: Volume Change:</p>																												
<p>Step 3. IDENTIFY PHASING</p> <pre> --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v </pre> <p>A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 > </p>	<p>Step 6a. CRITICAL VOLUMES, in vph (two phase signal)</p> <p>Approach 3</p> <p>Approach 1</p> <p>See Step 6b.</p> <p>Approach 2</p> <p>Approach 4</p>	<p>COMMENTS</p> <p>Exclusive right turns reduced 30 % V/C Ratio = .68</p>																												

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND VINEYARD RD
Problem Statement: ALTD

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY	Step 4. LEFT TURN CHECK	Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP
<p>Approach 3: VINEYARD RD</p> <pre> 1 2 2 ^ R L M R T T T L T H H H T < < > > ^--RT 1 2 LT--^ v v v <^--RTH LTH--> <--TH 2 2 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 2 1 RT--v < < > > Approach 2 L L T R R T T H T T GERBER RD H H 2 2 1 Approach 4: VINEYARD RD </pre>	<p>-----Approach-----</p> <pre> : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? : </pre>	<p>Possible Volume Adjusted Critical Volume to next Critical Volume in vph</p> <pre> B2B1 61(B1) 83- 61= 22(B2) 61 A1B2 22(B2) 119- 22= 97(A1) 22 A1A2 97(A1) OR 95(A2) 97 B4B3 6(B4) 28- 6= 22(B3) 6 A4B3 22(B3) 25- 22= 3(A4) 22 A3A4 405(A3) OR 3(A4) 405 </pre>
<p>Step 2. IDENTIFY VOLUMES, in vph</p> <pre> 3: LT= 10 Approach 3 2:RT= 50 TH= 810 TH= 190 RT= 60 v LT= 110 <--Approach 2 Approach 1--> 1:LT= 150 ^ 4: RT= 10 TH= 170 TH= 50 RT= 170 LT= 50 Approach 4 </pre>	<p>Step 5. ASSIGN LANE VOLUMES, in vph</p> <pre> 4 4 6 0 0 0 5 5 5 6 < v v v > 83 --^ 68 --^ 85 --> 85 --> 170 -v 2 2 2 2 1 8 3 5 5 0 </pre>	<p>Step 7. SUM OF CRITICAL VOLUMES</p> <p>83(B1B2)+97(A1)+28(B4B3)+405(A3)</p> <p>= 613 vph</p> <p>Step 8. INTERSECTION LEVEL OF SERVICE</p> <p>(compare step 7 with table 6)</p> <p style="text-align: center;">A</p> <p>Step 9. RECALCULATE</p> <p>Geometric Change:</p> <p>Signal Change:</p> <p>Volume Change:</p>
<p>Step 3. IDENTIFY PHASING</p> <pre> --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v </pre>	<p>Step 6a. CRITICAL VOLUMES, in vph (two phase signal)</p> <p>Approach 3</p> <p>Approach 1</p> <p>See Step 6b.</p> <p>Approach 2</p> <p>Approach 4</p>	<p>COMMENTS</p> <p>Exclusive right turns reduced 30 % V/C Ratio = .45</p>

APPENDIX A-2

NVSSP FEIR: Final Technical Appendices Vol. III

FINAL REPORT
Transportation Analysis for the
North Vineyard Station Specific Plan

Prepared for:
The North Vineyard Station Property Owners

October 2, 1996

Revised Pages 31/32 (February 6, 1998)



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EXECUTIVE SUMMARY

The North Vineyard Station Specific Plan area is a 1,595-acre residential development with supporting office and commercial uses and various public facilities and amenities. A total of 6,339 dwelling units are proposed, in addition to approximately 38 acres of retail and office uses. Public facilities include neighborhood parks, a community park, a library, and elementary schools.

The transportation study analyzed the impacts of the proposed project on the existing and planned roadway, transit, bicycle, and pedestrian circulation systems. The key findings of the study are summarized below.

- Currently, fourteen of the study intersections operate at acceptable levels of service. The following intersections operate at unacceptable levels (LOS F) during the a.m. and/or p.m. peak hour.
 - South Watt Avenue at Jackson Road;
 - Bradshaw Road at Jackson Road;
 - Elk Grove-Florin Road at Florin Road; and
 - Elk Grove-Florin Road at Gerber Road.
- Five roadway segments currently experience physical deficiencies based on substandard travel lane and shoulder widths.
- The proposed project will generate 84,037 daily trips, 5,460 a.m. peak hour trips, and 8,663 p.m. peak hour trips. Approximately 20 percent of these trips are expected to remain within the project boundaries.
- Without roadway improvements, the addition of project traffic would result in operational deficiencies on five roadway segments and at eight study intersections. Improvements under existing plus project conditions include widening the following roadway segments from two to four lanes:
 - Elk Grove-Florin Road from Jackson Road (SR 16) to Gerber Road
 - Bradshaw Road from Jackson Road (SR 16) to Gerber Road
 - Florin Road from Elk Grove-Florin Road to Waterman Road
 - Gerber Road from Elk Grove-Florin Road to Bradshaw Road
 - Jackson Road (SR 16) from South Watt Avenue to Bradshaw Road

Existing deficiencies resulting from substandard roadway cross-sections are exacerbated with the addition of project traffic.

Intersection modifications (e.g., lane additions, traffic signal installations) would be required under existing plus project conditions at the following locations:

- South Watt Avenue at Jackson Road (SR 16);
 - Bradshaw Road at Jackson Road (SR 16);
 - Bradshaw Road at Elder Creek Road;
 - Bradshaw Road at Florin Road;
 - Elk Grove-Florin Road at Gerber Road;
 - Bradshaw Road at Gerber Road;
 - South Watt Avenue at Elder Creek Road; and
 - Elk Grove-Florin Road at Florin Road.
- Under cumulative no project conditions, eight of the study intersections are projected to operate at LOS F during the a.m. peak hour and/or p.m. peak hour. Segments of Excelsior, Elder Creek, and Gerber Roads will experience physical deficiencies based on substandard travel lane and shoulder widths.
 - Implementation of the project under cumulative conditions will degrade acceptable operations to unacceptable levels at three additional locations.

The intersection modifications listed below are required to provide acceptable peak hour operations under cumulative plus project conditions.

Construct dual right turn lanes on the eastbound approach and triple left turn lanes on the northbound approach to the Bradshaw Road/Jackson Road intersection. Implementation of this improvement would result in LOS E operation at this location during the a.m. and p.m. peak hour.

Construct Excelsior Road as a four-lane facility between Jackson Road and Florin Road. At the Jackson Road/Excelsior Road intersection, this modification would improve the cumulative plus project LOS from F to E during the a.m. peak hour. The p.m. peak hour LOS with this improvement would still be F; however, the V/C ratio with the project would be 1.04 or 19 percent less than the ratio of 1.24 under no project conditions. The Elder Creek Road widening should include two northbound and southbound through lanes at the Elder Creek Road/Excelsior Road intersection. Implementation of this improvement would result in LOS C or better operations during the a.m. and p.m. peak hours.

Construct dual left turn lanes on the eastbound approach and two through lanes on the northbound approach at the Florin Road/Excelsior Road intersection. This modification, which would require widening of the northbound departure leg to accommodate two lanes, would result in LOS C operation during the a.m. and p.m. peak hour.

Construct dual right turn lanes on the westbound approach to the Bradshaw Road/Elder Creek Road intersection. Although this modification does not provide acceptable operations during the p.m. peak hour, it does improve intersection operations with the project by 25 percent.

Construct dual right turn lanes on the westbound approach to the Bradshaw Road/Florin Road intersection. Although this modification does not provide acceptable operations during either peak hour with the project, it does result in a.m. peak hour operations that are better than conditions without the project.

The following study locations would require the construction of eight-lane arterial roadway segments to provide acceptable traffic operations based on either daily or peak hour volumes:

- Bradshaw Road between Florin Road and Elder Creek Road (daily);
- South Watt Avenue between Jackson Road and Florin Road (peak hour); and
- Elk Grove-Florin Road between Florin Road and Calvin Road (peak hour).

Since Sacramento County does not typically construct eight-lane arterial roadways except under special circumstances (e.g., adjacent to a freeway interchange), these improvements are not considered feasible. Thus, these locations would continue to operate at unacceptable levels under cumulative plus project conditions.

I.**INTRODUCTION****STUDY PURPOSE**

The purpose of this study is to analyze the impacts of the North Vineyard Station Specific Plan on the adjacent transportation system. This study assesses the short- and long-term impacts of the project on the local circulation network and describes the improvements required to mitigate operational deficiencies of the transportation system.

PROJECT LOCATION AND DESCRIPTION

The North Vineyard Station Specific Plan area is located in southeastern Sacramento County and is bounded by Florin Road to the north, Elder Creek to the west, the planned Vineyard Road extension to the east, and Gerber Road to the south. Figure 1 shows the location of the plan area and the adjacent roadway network.

The plan area is a 1,595-acre residential development with supporting office and commercial uses and various public facilities and amenities. A total of 6,339 dwelling units are proposed, in addition to approximately 38 acres of retail and office uses. Public facilities include neighborhood parks, a community park, a library, and elementary schools.

The proposed on-site street system consists of a network of two-lane collector and local streets interconnected by several major collector roadways that will provide access to the existing and future arterial roadways: Florin Road, Gerber Road, Bradshaw Road and the extensions of Vineyard Road and Waterman Road. As part of the project, improvements to the existing transit, bicycle, and pedestrian facilities, as well as new facilities, will be implemented to encourage the use of alternative travel modes.

EVALUATION CRITERIA

Criteria for evaluating project impacts were established for roadways (including segments and intersections), transit facilities, and bicycle/pedestrian facilities. Each element of the transportation system is described below.

Arterial Roadway Segments

For purposes of this study, roadway segments were analyzed by comparing average daily traffic volumes to capacity thresholds that were developed for the Sacramento County General Plan Update. These thresholds are used as guidelines by the County to master plan the transportation network by projecting the need for new or upgraded facilities.

The effect of substandard roadway cross-sections was also used to identify deficiencies. Some of the study roadways, including Elk Grove-Florin Road, Jackson Road, and Bradshaw Road, include 12-foot travel lanes and minimum 6-foot usable shoulders. Other roadways such as Gerber Road and Excelsior Road provide 10- or 11-foot travel lanes and usable shoulders that are less than two-feet wide.

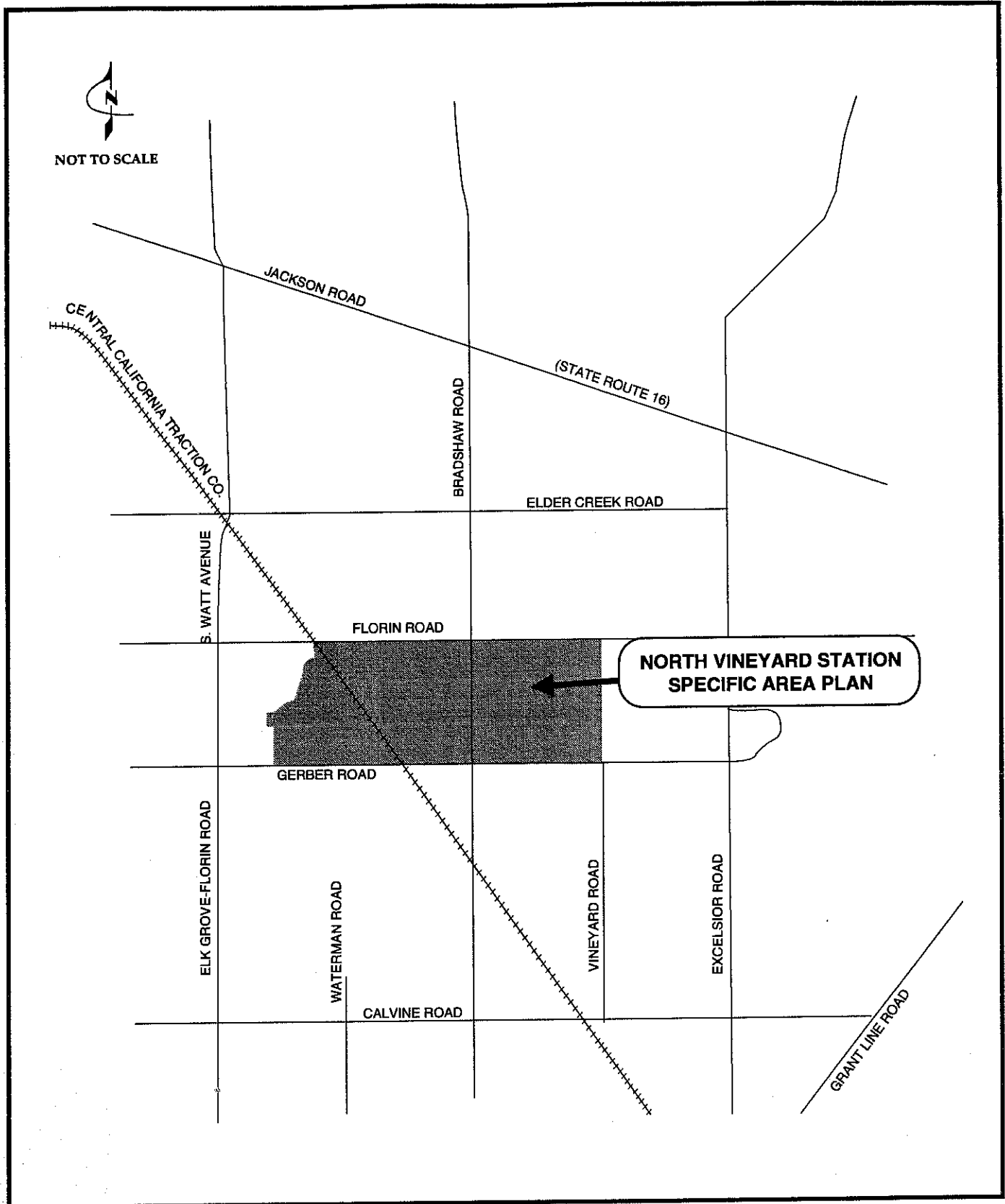


FIGURE 1

PROJECT LOCATION MAP

Operational Deficiencies

According to the Sacramento County General Plan, the streets within the study area are ultimately planned to be medium-access arterial roadways, with two to four intersections per mile, a limited number of driveways, and a speed limit between 35 and 55 miles per hour. For this type of facility, the daily roadway capacity is approximately 18,000, 36,000, and 54,000 VPD for streets with two lanes, four lanes and six lanes, respectively. These capacities were applied to all facilities under existing and cumulative conditions.

The project results in an operational deficiency if the addition of project-generated traffic causes the demand on a facility to exceed its ultimate capacity, thus requiring additional travel lanes beyond those delineated in the Transportation Plan of the *General Plan Circulation Element* dated December 15, 1993.

Physical Deficiencies

As noted above, several study roadways include substandard cross-sections. According to standards set forth in *A Policy on Geometric Design of Highways and Streets* (1994) published by the American Association of State Highway and Transportation Officials (AASHTO), the minimum lane width and shoulder width is 12 feet and 6 feet, respectively, for rural arterials serving more than 400 VPD. Thus, a physical deficiency is defined to occur when a substandard roadway segment serves more than 400 VPD.

Signalized Intersections

Level of service (LOS) is a term which qualitatively describes operating conditions for intersections. There are six levels of service, A through F, which represent driving conditions from best to worst, respectively. In general, LOS A represents free-flow conditions with no congestion and LOS F represents severe congestion or delay under stop-and-go conditions.

The County of Sacramento has defined the level of service standard for urban area roadways to be LOS E (i.e., LOS F is considered unacceptable). For intersections of rural collector roadways, the minimum acceptable LOS is D. These standards have been set forth in Policy CI-22 in the County's *General Plan Circulation Element*. As set forth in Policy CI-23, if implementation of a project results in a level of service worse than those specified in Policy CI-22, traffic impacts must be mitigated by enhancing the capacity of the roadway and transit system or reducing the demand generated by the project.

Analysis of signalized intersections was completed using the methods described in *Interim Materials on Highway Capacity* (Circular No. 212, Transportation Research Board, January 1980). The characteristics of traffic operations for each LOS with respect to signalized intersections are shown in Table 1. Corresponding to each LOS is a volume-to-capacity (V/C) ratio. This is the ratio of the existing or projected volume to the theoretical capacity of the intersection. An intersection is defined to be "at capacity" at LOS E when the V/C ratio is 1.00.

Level of Service	Interpretation	Volume-to-Capacity Ratio
A	Uncongested operations; all queues clear in a single cycle.	Less than 0.61
B	Very light congestion; an occasional phase is fully utilized.	0.61-0.70
C	Light congestion; occasional queues on approaches.	0.71-0.80
D	Significant congestion on critical approaches, but intersection is functional. Cars required to wait through more than one cycle during short peaks. No longstanding queues formed.	0.81-0.90
E	Severe congestion with some longstanding queues on critical approaches. Traffic queue may block nearby intersection(s) upstream of critical approach(es).	0.91-1.00
F	Total breakdown, stop-and-go operation.	Greater than 1.00

Source: *Interim Materials on Highway Capacity* (Circular 212, Transportation Research Board, 1980).

The project results in an operational deficiency when the addition of project-generated traffic changes the LOS from E or better to LOS F. For facilities operating at LOS F without the project, an operational deficiency is identified if the addition of project traffic increases the V/C ratio by 0.05 or more.

Unsignalized Intersections

For those intersections that are unsignalized, two methodologies were used to analyze operating conditions. For those intersections with stop sign control on the minor street approach only, the methods described in Chapter 10 of the *Highway Capacity Manual* (Special Report 209, Transportation Research Board, 1985). Level of service is determined for each movement based on the reserve capacity available. Reserve capacity is a function of demand and the critical gaps in traffic on the major street. The criteria for level of service at stop sign-controlled intersections are shown in Table 2.

Level of Service	Interpretation	Reserve Capacity
A	Little or no delay.	≥400
B	Short traffic delays.	300 to 399
C	Average traffic delays.	200 to 299
D	Long traffic delays.	100 to 199
E	Very long traffic delays.	0 to 99
F	Stop-and-go conditions.	<0

Source: *Highway Capacity Manual* (Special Report 209, Transportation Research Board, 1985).

At intersections with all-way stop control (i.e., all traffic stops), procedures described in *Interim Materials on Unsignalized Intersection Capacity* (Circular No. 373, Transportation Research Board, 1991) were utilized. These methods determine an overall level of service based on the estimated delay for traffic on each of the approaches. The range of delay for each level of service is summarized in Table 3.

Level of Service	Interpretation	Average Vehicle Delay (in seconds)
A	Little or no delay.	≤5
B	Short traffic delays.	>5 and ≤10
C	Average traffic delays.	>10 and ≤20
D	Long traffic delays.	>20 and ≤30
E	Very long traffic delays.	>30 and ≤45
F	Stop-and-go conditions.	>45

Source: *Interim Materials on Unsignalized Intersection Capacity* (Circular No. 373, Transportation Research Board, 1991).

The project results in an operational deficiency if the addition of project traffic changes the LOS from E or better to F. For those intersections that are affected, an analysis of peak hour volumes is conducted to determine if a traffic signal is warranted.

Transit Facilities

The effect of a project on a transit system can be quantified using several different criteria including quantitative changes in ridership demand, headways, and routing, etc. Due to the general nature of this analysis and the limited existing service in proximity to the project site, the North Vineyard Station Specific Plan will result in an operational deficiency if:

- Implementation of the project disrupts or interferes with existing or planned transit operations and facilities of Regional Transit, or
- Implementation of the project conflicts with the transit-related goals, objectives and policies of the *Sacramento County General Plan - Circulation Element, Section V - Transportation Policy Plan, (December 15, 1993)*.

Bicycle and Pedestrian Facilities

Impacts to bicycle and pedestrian facilities can be determined in the same manner as impacts to transit operations and facilities. Thus, the North Vineyard Station Specific Plan results in an operational deficiency if:

- Implementation of the project disrupts or interferes with existing or planned bicycle facilities of the *Sacramento City/County 2010 Bikeways Master Plan, or*
- Implementation of the project conflicts with the bicycle- and pedestrian-related goals, objectives and policies of the *Sacramento County General Plan - Circulation Element, Section V- Transportation Policy Plan, (December 15, 1993)*.

REPORT ORGANIZATION

Chapter II describes the existing transportation system serving the project site and the operating characteristics of each facility without the project. The impact of the North Vineyard Station Specific Plan on existing conditions is presented in Chapter III, "Existing Plus Project Conditions." Chapter IV presents cumulative conditions with and without the project in place and describes measures required to improve operational deficiencies. Chapter V presents a discussion of several related transportation issues including on-site circulation, Congestion Management Plan consistency, trip and emissions reduction measures, and land use alternatives analysis.

II. EXISTING CONDITIONS

In the vicinity of the project site, the existing transportation system is almost exclusively represented by roadways. Automobiles are the primary travel mode for most trips in this area, while designated facilities for bicycles and pedestrians are limited. Each of the potentially affected travel modes is discussed in this chapter.

ROADWAY NETWORK

Since most trips in this area are made using automobiles, the roadway system is the primary focus of this analysis. Figure 2 shows the lane configurations of the study roadway segments under existing conditions and the location of the study intersections.

The project site is located in southeastern Sacramento County. Access to the study area is provided by a system of arterial roadways including Florin Road, Elk Grove-Florin Road, Bradshaw Road, and Calvine Road. These roadways provide access to regional facilities such as State Route 99 (SR 99) four miles to the west, U.S. Highway 50 four miles to the north, and Jackson Road/SR 16 located two miles to the north.

Most of the existing roads serving the site are two-lane arterial roadways. These facilities have narrow shoulders (some unpaved), speed limits of 35 to 55 miles per hour, and in some cases, travel lanes less than 12 feet wide. These roads provide access to agricultural and low-density residential uses on the project site. According to the Sacramento-Solano-Sutter-Yolo Counties California Road System Map (1992), the following road segments are classified as urban facilities:

- SR 16 from Elk Grove-Florin Road to Bradshaw Road;
- Elk Grove-Florin Road from SR 16 to Calvine Road;
- Gerber Road from Elk Grove-Florin Road to the Central California Traction Railroad;
- Calvine Road from Elk Grove-Florin Road to Bradshaw Road; and
- Waterman Road north of Calvine Road.

Several roadway segments do not meet Sacramento County standards for roadway cross-sections that call for 12-foot travel lanes and 6-foot usable shoulders. These segments include:

- Excelsior Road from SR 16 to Calvine Road;
- Elder Creek Road from South Watt Avenue to Excelsior Road;
- Gerber Road from Vineyard Road to Excelsior Road;
- Vineyard Road from Gerber Road to Vineyard Road; and
- Calvine Road from Waterman Road to Grant Line Road.

Similar standards are presented in *A Policy on Geometric Design of Highways and Streets* (1994) published by the American Association of State Highway and Transportation Officials (AASHTO) for roadway segments serving more than 400 VPD and a design speed of 50 miles per hour or more.

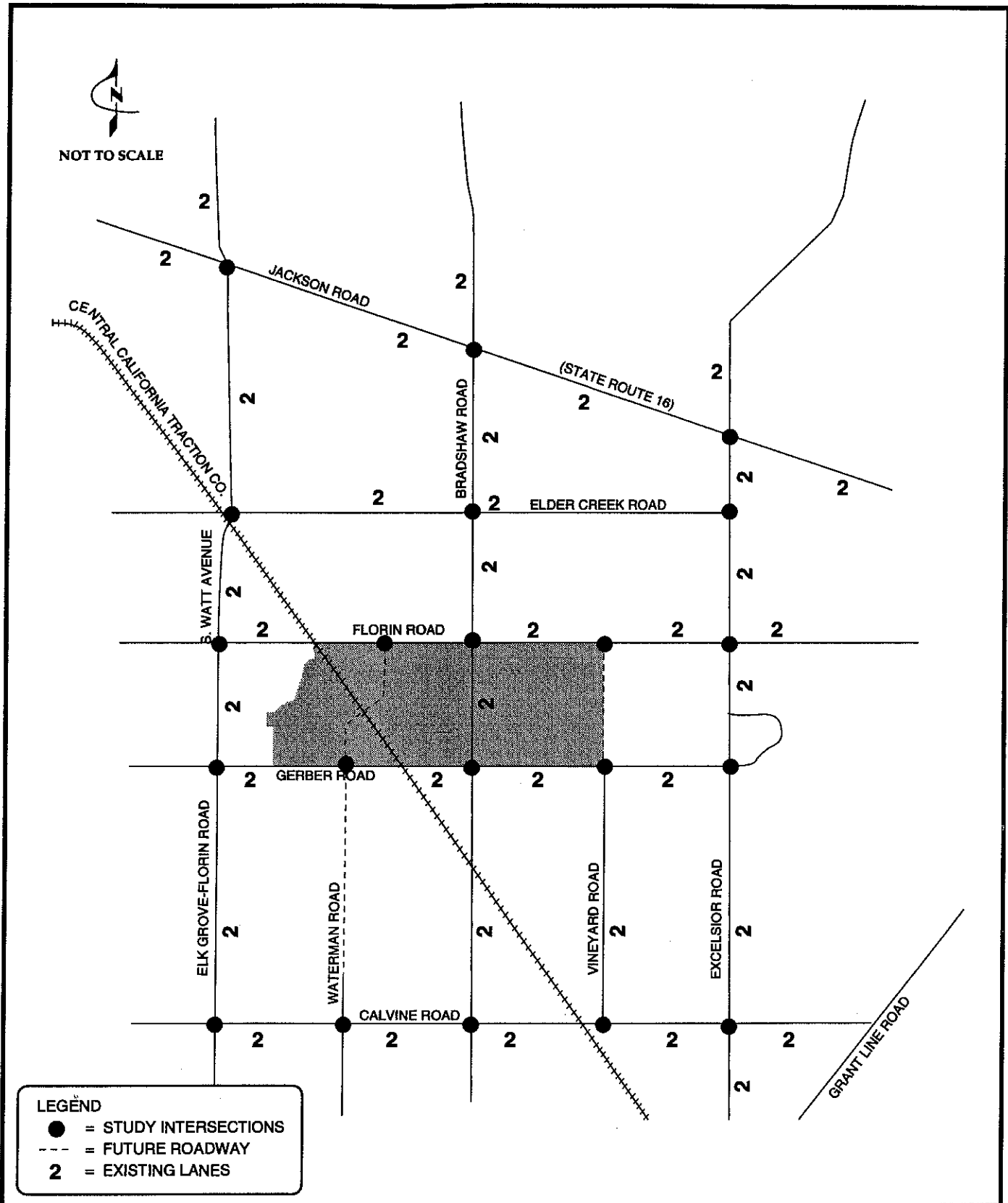


FIGURE 2

STUDY INTERSECTIONS AND ROADWAYS

EXISTING TRAFFIC VOLUMES

Average daily traffic (ADT) volumes were obtained from two sources: 1) count data compiled by the County Transportation Division, and 2) estimated daily volumes based on peak hour turning movement counts. ADT volumes for roadways are shown on Figure 3.

Fehr & Peers Associates, Inc. conducted intersection turning movement counts for both the a.m. and p.m. peak periods during May, 1995. Existing peak hour volumes and lane configurations at each intersection are presented on Figure 4.

EXISTING TRAFFIC OPERATIONS

Arterial Roadway Segments

With the exception of four roadway segments near SR 16, all of the existing roadway segments are operating at an acceptable level according to Sacramento County ADT volume guidelines. The segments of South Watt Avenue (which is designated Elk Grove-Florin Road south of Florin Road) and Bradshaw Road immediately north and south of SR 16 currently carry traffic volumes in excess of 15,000 to 18,000 VPD. Although these facilities are more urban in character, the existing volumes ranging from approximately 15,400 to 26,100 VPD exceed the theoretical capacities.

All of the roadway segments that do not meet Sacramento County/AASHTO standards for roadway cross-sections (see Page 7) currently serve more than 400 VPD. Therefore, all of these segments experience physical deficiencies under existing conditions.

Study Intersections

Levels of service for study intersections were determined by comparing existing peak hour traffic volumes on Figure 4 to the evaluation criteria defined in Chapter I. The results are presented in Table 4 and the calculation worksheets are included as Appendix A.

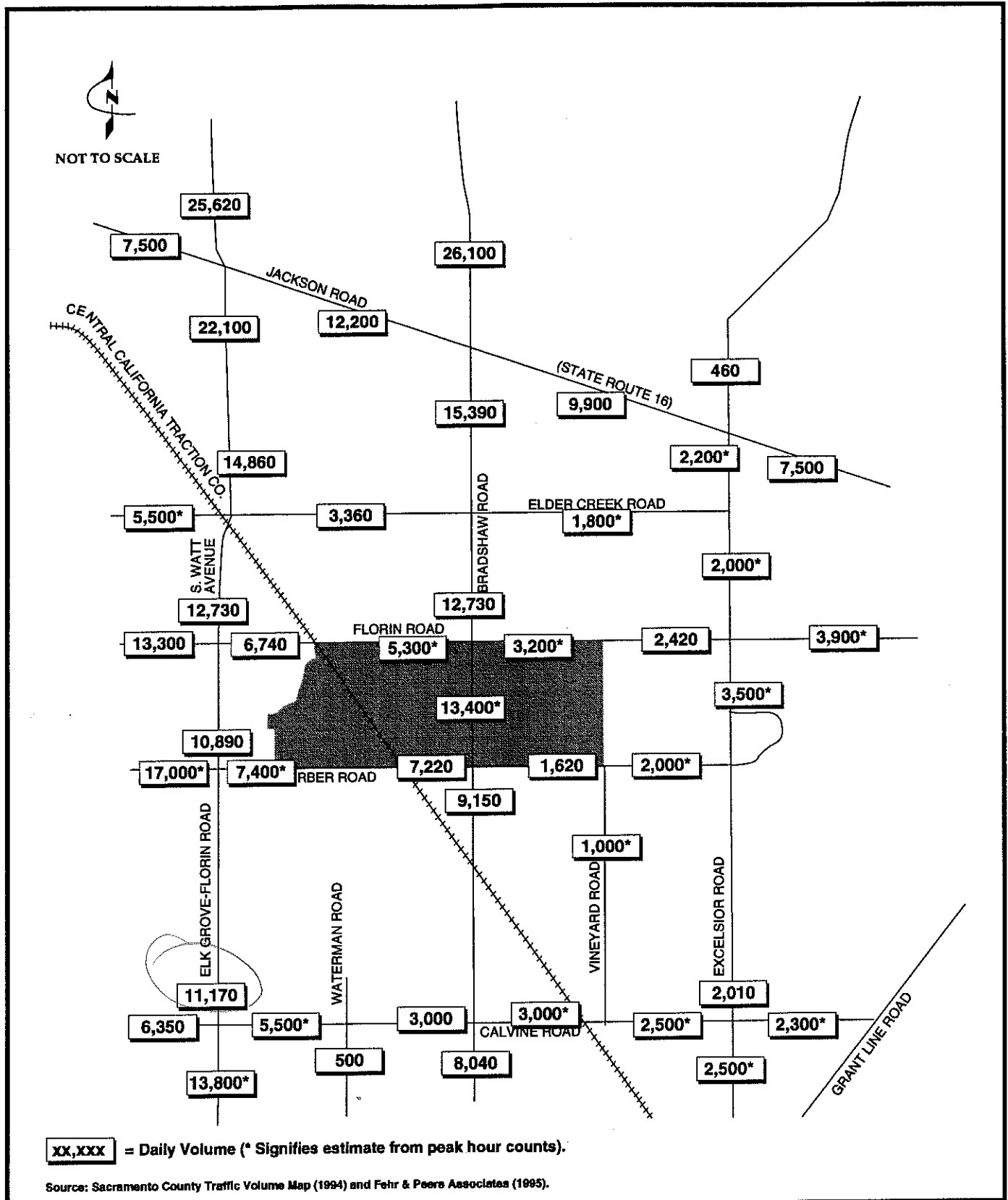
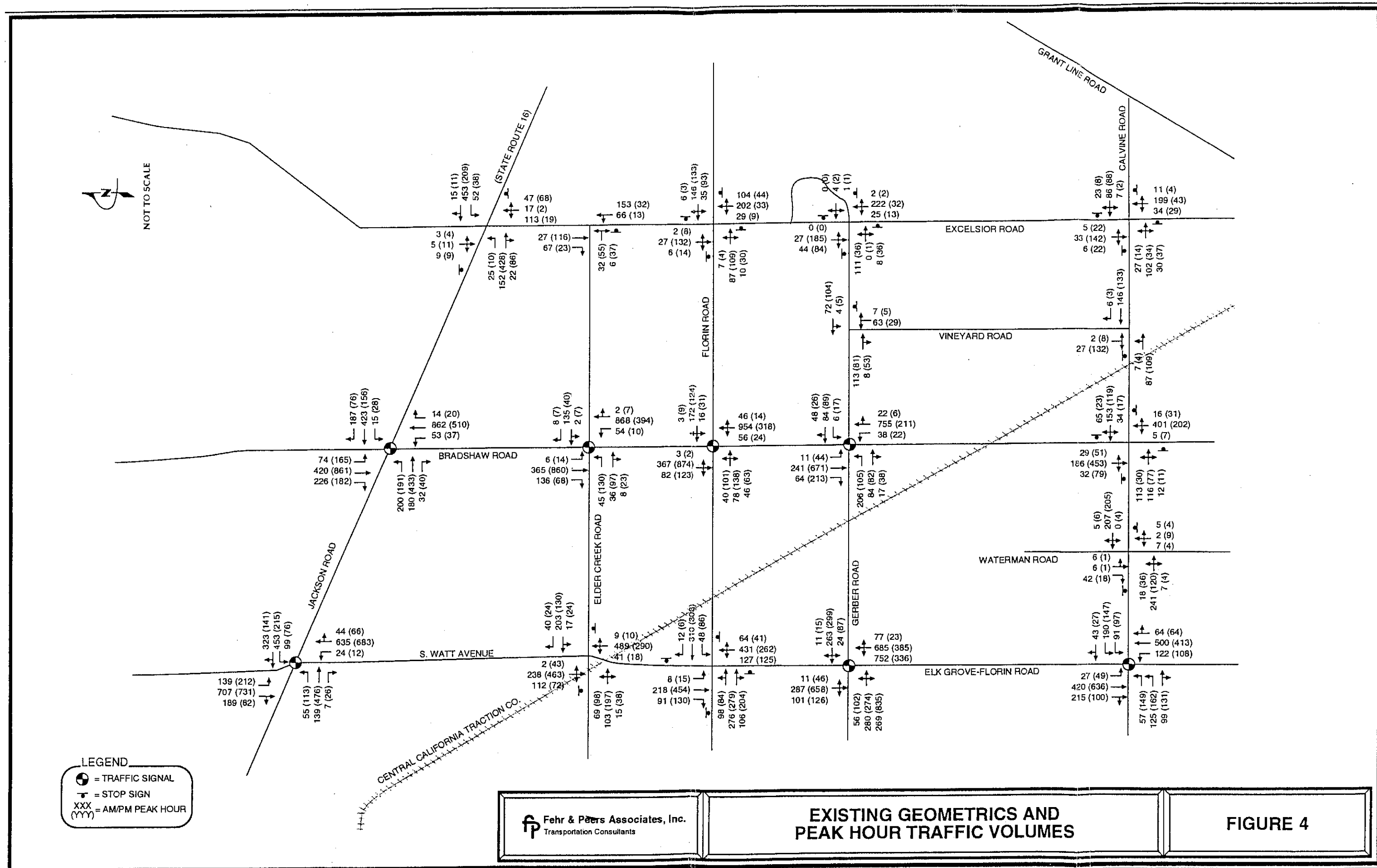


FIGURE 3

EXISTING DAILY VOLUMES

fp Fehr & Peers Associates, Inc.
Transportation Consultants



**Table 4
Existing Intersection Level of Service**

Intersection	Traffic Control	AM Peak Hour		PM Peak Hour	
		V/C Ratio, Delay or RC ¹	LOS ²	V/C Ratio or Delay or RC	LOS
S. Watt Avenue/Jackson Road	Signal	1.04	F	1.12	F
Bradshaw Road/Jackson Road	Signal	1.13	F	0.99	E
Bradshaw Road/Elder Creek Road	Signal	0.77	C	0.76	C
Bradshaw Road/Florin Road	Signal	0.89	D	0.82	D
Elk Grove-Florin Road/Gerber Road	Signal	1.05	F	1.30	F
Bradshaw Road/Gerber Road	Signal	0.82	D	0.66	B
Elk Grove-Florin Road/Calvine Road	Signal	0.52	A	0.60	A
Excelsior Road/Jackson Road	Two-way Stop	239	C	378	B
Excelsior Road/Elder Creek Road	Two-way Stop	709	A	616	A
Vineyard Road/Gerber Road	Two-way Stop	562	A	596	A
Excelsior Road/Gerber Road	Two-way Stop	432	A	547	A
Waterman Road/Calvine Road	Two-way Stop	489	A	677	A
Vineyard Road/Calvine Road	Two-way Stop	881	A	767	A
S. Watt Avenue/Elder Creek Road	All-way Stop	15.0 sec	C	37.0 sec	E
Excelsior Road/Florin Road	All-way Stop	6.0 sec	B	3.0 sec	A
Elk Grove-Florin Road/Florin Road	All-way Stop	36.0 sec	E	66.0 sec	F
Bradshaw Road/Calvine Road	All-way Stop	15.0 sec	C	14.0 sec	C
Excelsior Road/Calvine Road	All-way Stop	4.0 sec	A	3.0 sec	A

Notes: ¹ V/C Ratio = Volume-to-Capacity Ratio (0.00 applies to signalized control); Delay = Average Total Delay (0.0 seconds applies to all-way stop control); and RC= Reserve Capacity (000 vehicles applies to two-way stop control).
² LOS = Level of Service

Under existing conditions, four of the study intersections operate at unacceptable levels (LOS F) during the a.m. and/or p.m. peak hours. These locations include:

- South Watt Avenue at Jackson Road (a.m. and p.m. peak hour);
- Bradshaw Road at Jackson Road (a.m. peak hour);
- Elk Grove-Florin Road at Gerber Road (a.m. and p.m. peak hour); and
- Elk Grove-Florin Road at Florin Road (p.m. peak hour).

Of the two-way stop-controlled locations, only the intersection of Excelsior Road at Jackson Road operates at worse than LOS A. This location operates at LOS D and LOS C during a.m. and p.m. peak hours, respectively. Three of the all-way stop controlled intersections operate at LOS C or worse including the Elk Grove-Florin road/Florin road intersection.

Signal warrant analyses were conducted at all unsignalized intersections operating at LOS C or worse. Utilizing the Peak Hour Signal Warrant described in the Caltrans Traffic Manual, warrant criteria were met at the South Watt Avenue/Elder Creek Road and Elk Grove-Florin Road/Florin Road intersections.

TRANSIT SYSTEM

In the greater Sacramento area, transit service is provided by Regional Transit (RT). The project site is located on the outskirts of the urbanized Sacramento area. At present, there are no transit services within the study area. The nearest bus routes operate on Kiefer Boulevard two miles to the north and on Power Inn Road two miles to the west of the project site.

BICYCLE AND PEDESTRIAN SYSTEMS

Bicycle facilities in the vicinity of the project site are limited. Within the study area, the only existing bicycle facility is a Class II bikeway on Elk Grove-Florin Road from Florin Road to south of Calvine Road. Class II bikeways include a separate lane delineated for bicycles by striping and signs.

Although bicycles can legally travel on all of the study roadways, cyclists must share the travel lanes on the two-lane facilities adjacent to the project site. Since these roads typically have 11-foot to 12-foot travel lanes with narrow, unpaved shoulders and vehicles travelling at relatively high speeds, bicycle travel is not a well-utilized mode within the study area.

OTHER TRANSPORTATION FACILITIES

A rail line operated and maintained by Central California Traction Railroad crosses the western portion of the project site between Hedge Avenue (east of Elk Grove-Florin Road) and Bradshaw Road. The railroad runs diagonally from northwest to southeast and includes existing at-grade crossings at Gerber Road and Florin Road. Current operations include one northbound train and one southbound train per day. The length of each train varies from two to 50 cars depending on demand.

III. EXISTING PLUS PROJECT CONDITIONS

This chapter describes the effect project-generated trips would have on the operation of the existing local transportation system and its users.

PROJECT DESCRIPTION

As described in Chapter I, the North Vineyard Station Specific Plan is a 1,595-acre residential development with supporting commercial and office uses, schools, and parks. The site for the plan area is bounded by Florin Road to the north, Gerber Road to the south, Elder Creek to the west, and the planned Vineyard Road extension to the east. The project will have access to all three roadways listed above and will be built to the east and west of the planned Waterman Road extension between Gerber and Florin Roads. The internal roadway system will include a series of interconnected collector streets to distribute traffic to the adjacent arterial roadways.

Project Trip Generation

Project-generated traffic from the North Vineyard Station Specific Plan area was estimated using the following process:

- Estimate vehicle trip ends for various land uses using standard trip generation rates;
- Adjust the total number of trips to account for internal and pass-by trips;
- Estimate transit utilization; and
- Determine net project trip generation.

Each step is explained in detail below.

Estimation Of Vehicle Trip Ends

The number of trips generated for each land use was estimated by applying trip rates from *Trip Generation*, Institute of Transportation Engineers, 5th Edition, 1991. These rates were applied to the proposed land uses to yield the total number of trips generated by each land use. It should be noted that similar rates were used in the East Elk Grove Specific Plan traffic analysis. The daily, a.m. peak hour, and p.m. peak hour trip rates are shown in Table 5 for each land use.

Land Use	Units	Daily	AM Peak Hour	PM Peak Hour
Low Density Residential	d.u.	9.55	0.74	1.01
Medium Density Residential	d.u.	9.45	0.74	1.01
High Density Residential	d.u.	6.47	0.56	0.69
Elementary School	students	1.09	0.28	0.25
Business Park	ksf GFA	14.37	1.62	1.48
Commercial	ksf GLA	Depends on Size of Parcel		

Internalization and Pass-By Adjustments

The total number of trips generated by each land use was adjusted to account for internal and pass-by trips. Internal trips are those which begin and end within the project boundary. An example is a trip from home to the neighborhood supermarket. Since these trips do not leave the project area, they do not affect operation of the external street system.

For the purposes of this study, internalization rates developed for the East Elk Grove Specific Plan were reviewed. After review and some modification, the following set of internalization rates were developed for the North Vineyard Station Specific Plan:

- Residential 14%
- Neighborhood Commercial 40%
- Community Commercial 30%
- Business Park 15%
- Elementary School 90%

For residential trips, the internalization rate was determined by assuming 80 percent of all non-residential internal trips would be home-based. This equates to a 14 percent internalization rate for residential areas of the project.

Pass-by trips reflect those trips that are attracted from passing traffic already on the external street system. These types of trips occur for retail and commercial land uses. According to *Trip Generation*, 5th Edition, Institute of Transportation Engineers, 1991, pass-by trip percentages vary based on the size of a retail or commercial land use and the daily traffic volume on the adjacent street. Given the types of retail and commercial land use proposed for the North Vineyard Station Specific Plan and the expected daily traffic volumes on the adjacent streets, pass-by factors of 20 percent for neighborhood commercial and 30 percent for community commercial were applied to the gross trip generation. These factors are consistent with values used in other traffic studies in the Sacramento region.

Transit Utilization

Transit utilization for the project was estimated using a two-step process. First, the number of vehicle-trips was converted to person-trips using a ratio of 1.15 persons per vehicle. The second step involved projecting the percentage of person-trips that would utilize transit. For the East Elk Grove Specific Plan, up to five percent of all person-trips would be completed using transit. This same percentage is considerable applicable for the North Vineyard Station Specific Plan, which will result in approximately 3,620 daily transit trips. About 250 of these trips will occur in the a.m. peak hour while about 375 will occur during the p.m. peak hour.

Net Project Trip Generation

Tables B-1 through B-3 in Appendix B summarize the project trip generation under daily, a.m. peak hour, and p.m. peak hour conditions for the following categories:

- Total vehicle-trips;
- External and internal vehicle-trips;
- External person-trips; and
- Total transit trips.

The project is expected to generate about 84,000 daily vehicle trips. About 16,780 or 20 percent of these trips will remain within the project area while about 63,000 will travel on the external street system. The single highest hour of trip generation will occur during the p.m. peak hour when about 10 percent of the total daily trips will be generated.

Residential trips represent just over 70 percent of the total gross daily vehicle trips with neighborhood and community commercial uses representing 29 percent. Because of internalization and pass-by trips, the commercial uses represent only 20 percent of the net external vehicle-trips, whereas, residential, school, and park uses comprise 80 percent of this total.

Project Traffic Distribution

The initial directional distribution of project traffic onto the surrounding road network was based on travel patterns generated by the MINUTP-based Sacramento County travel demand model. These patterns were reviewed with County Transportation Division staff and adjusted to reflect the fact that 1) some of the facilities coded in the 2010 model would not be widened to their ultimate cross-section, and 2) planned employment centers represented in the model would not be developed under existing plus project conditions. The resulting directional distribution for existing plus project conditions is shown on Figure 5.

As indicated in the figure, traffic distribution under existing conditions is generally expected to favor areas to the north. This pattern is expected given that the project site is located in the southern area of the developed Sacramento region and, more importantly, is south of the employment base in downtown Sacramento.

Project Traffic Assignment

To accurately trace the expected paths of project-generated traffic at each intersection, a computer model of the project's internal roadways and the study area roadway system was developed using the TRAFFIX software package. This mechanism provided an efficient way of accounting for traffic generated by different areas within the project and tracing these trips at each study intersection. Based on the trip distribution percentages in Figure 5, project trips were added to the existing traffic counts at the study intersections listed in Chapter II.

Figures 6 and 7 present daily and peak hour traffic volumes, respectively, under existing plus project conditions. The resulting existing plus project volumes were analyzed to determine the impact of adding project traffic to the roadway network. Impacts to each component of the transportation system are presented below.

ROADWAY SYSTEM OPERATIONS

Arterial Roadway Segments

Daily traffic volumes shown on Figure 6 were compared to the capacity criteria for arterial roadway segments presented in Chapter I. The result of this comparison shows that the operation of several study roadway segments would be affected with the addition of project traffic. Each arterial roadway segment deficiency is discussed below.

- EP-1:** Implementation of the project would result in daily traffic volumes on the existing two-lane section Elk Grove-Florin Road between Gerber Road and SR 16 that would exceed the theoretical capacity. The projected daily volume between SR 16 and Gerber Road ranges from 23,500 to 34,700 vehicles per day (VPD) with the project. North of SR 16, the volume is projected to be 35,100 VPD. These volumes exceed both the rural and urban two-lane capacities of 15,000 and 18,000 VPD, respectively.
- EP-2:** Implementation of the project would result in daily traffic volumes on Bradshaw Road between Gerber Road and SR 16 ranging from 32,900 to 28,000 VPD. North of SR 16, the daily volume with the project is estimated to be 32,400 VPD. All volumes exceed both the rural and urban two-lane capacities of 15,000 and 18,000 VPD, respectively.
- EP-3:** Implementation of the project would result in a daily volume on Florin Road between Elk Grove-Florin Road (South Watt Avenue) and the proposed Waterman Road extension of 28,800 VPD. This volume exceeds both the rural and urban two-lane capacities of 15,000 and 18,000 VPD, respectively.
- EP-4:** Implementation of the project would result in daily volumes on Gerber Road between Elk Grove-Florin Road and Bradshaw road ranging from 21,900 to 25,000 VPD. West of Elk Grove-Florin Road, the daily volume is projected to be 23,500 VPD with the project. These volumes exceed both the rural and urban two-lane capacities of 15,000 and 18,000 VPD, respectively.

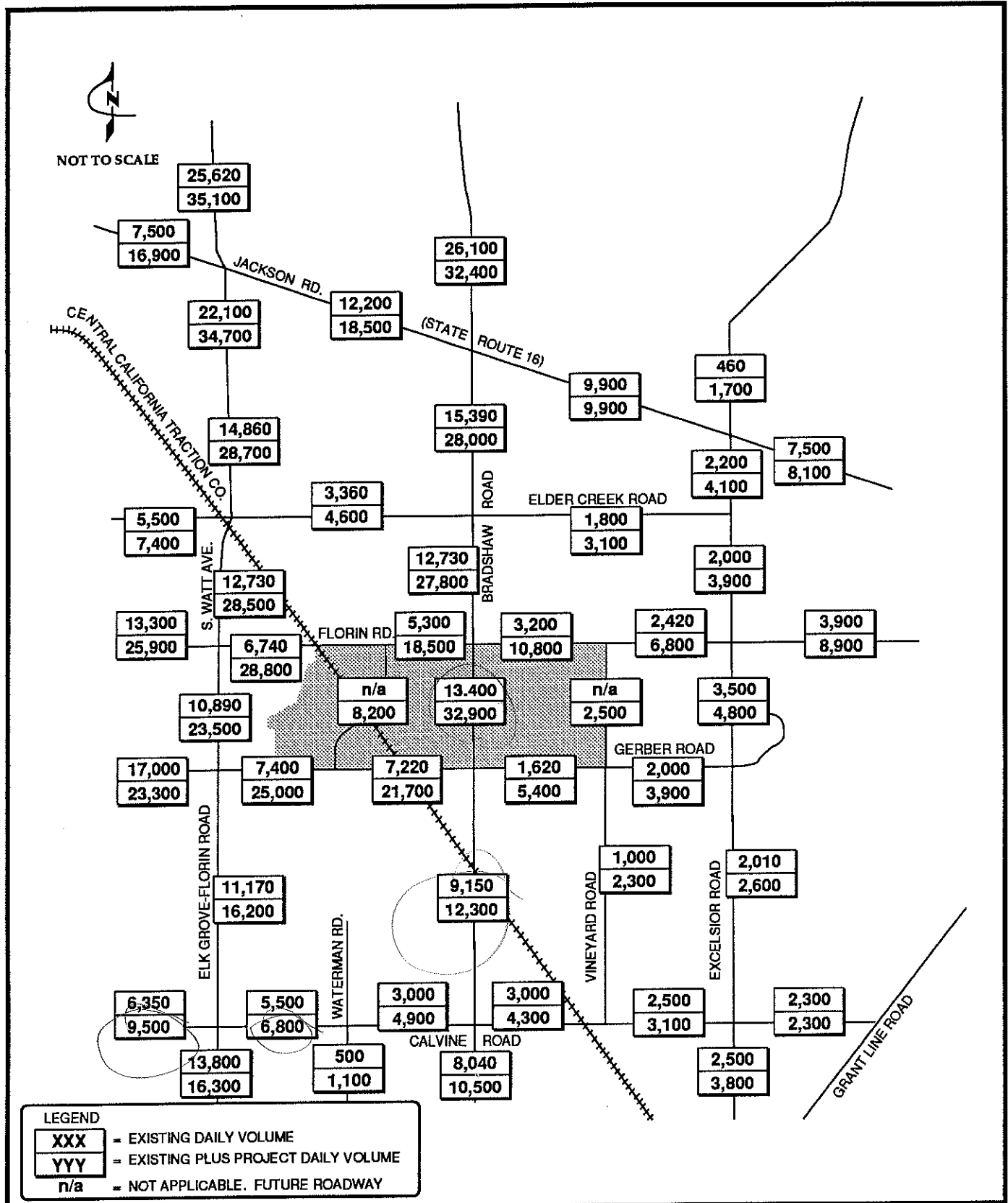
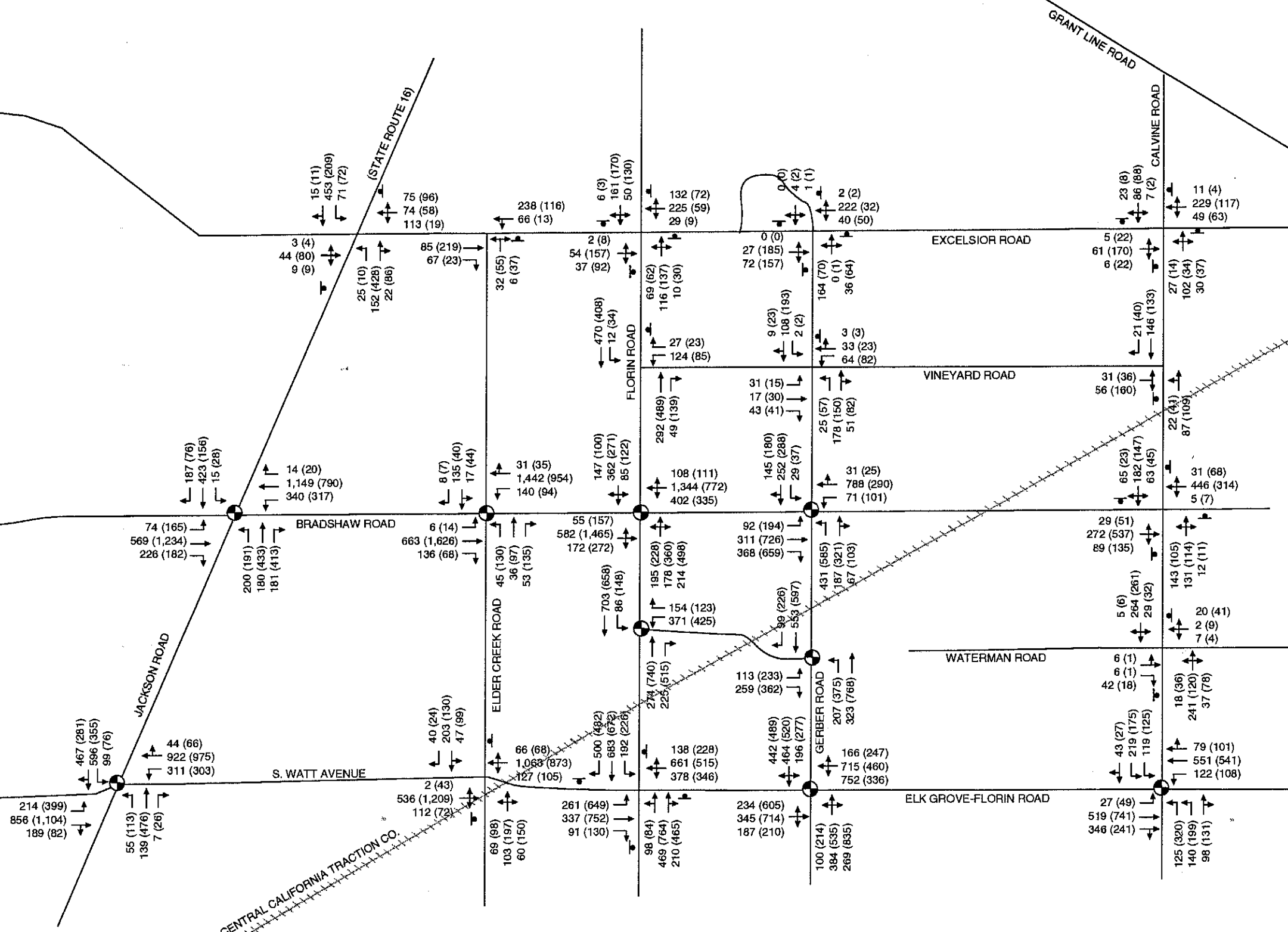
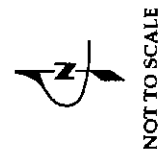


FIGURE 6 **EXISTING PLUS PROJECT DAILY VOLUMES** **fp** Fehr & Peers Associates, Inc. Transportation Consultants



LEGEND
 ⊕ = TRAFFIC SIGNAL
 T = STOP SIGN
 XXX = AM/PM PEAK HOUR
 (YYY)

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 Transportation Consultants

**EXISTING PLUS PROJECT GEOMETRICS AND
 PEAK HOUR TRAFFIC VOLUMES**

FIGURE 7

- EP-5:** Implementation of the project would result in a daily volume of 18,500 VPD on SR 16 between Bradshaw Road and South Watt Avenue. Although this volume exceeds the urban roadway capacity of 18,000 VPD by only 500 vehicles, operations on this segment would be deficient under existing plus project conditions.
- EP-6:** Implementation of the project would add approximately 1,300 VPD to Elder Creek Road between South Watt Avenue and Excelsior Road. The addition of this traffic would exacerbate the existing physical deficiency caused by the substandard travel lane and shoulder widths.
- EP-7:** Implementation of the project would add approximately 600 to 1,900 VPD to Excelsior Road between Jackson Road (SR 16) and Calvine Road. Except in the vicinity of Birch Ranch Road, the addition of this traffic would exacerbate the existing physical deficiency caused by the substandard travel lane and shoulder widths.
- EP-8:** Implementation of the project would add approximately 4,400 to 7,600 VPD to Florin Road between Bradshaw Road and Excelsior Road. The addition of this traffic would exacerbate the existing physical deficiency caused by the substandard travel lane and shoulder widths.
- EP-9:** Implementation of the project would add approximately 1,900 to 3,800 VPD to Gerber Road between Bradshaw Road and Excelsior Road. The addition of this traffic would exacerbate the existing physical deficiency caused by the substandard travel lane and shoulder widths.
- EP-10:** Implementation of the project would add approximately 1,300 VPD to Vineyard Road between Gerber Road and Calvine Road. Except in the vicinity of the residential subdivision immediately north of Calvine Road, the addition of this traffic would exacerbate the existing physical deficiency caused by the substandard travel lane and shoulder widths.
- EP-11:** Implementation of the project would add approximately 600 to 1,900 VPD to Calvine Road between Elk Grove-Florin Road and Excelsior Road. Except in the vicinity of residential subdivisions near Waterman Road and Vineyard Road, the addition of this traffic would exacerbate the existing physical deficiency caused by the substandard travel lane and shoulder widths.

Study Intersections

Existing plus project traffic volumes shown on Figure 7 were used to calculate peak hour levels of service at the study intersections. Intersection LOS at each location is presented in Table 6. The corresponding calculation worksheets are included in Appendix C.

As shown on Figures 6 and 7, a new four-lane, north-south arterial will be constructed between Gerber and Florin Roads. The alignment for this roadway will be located approximately halfway between Elk Grove-Florin Road and Bradshaw Road and will ultimately be connected to the planned extension of Waterman Road from Vintage Park Drive north to Gerber Road. The proposed roadway will include a new at-grade crossing of the Central California Traction Railroad and will require the alignment to curve such that the roadway crosses the railroad tracks at nearly a perpendicular angle.

In addition, Vineyard Road will be extended from its current terminus at Gerber Road to Florin Road. The Vineyard Road extension will be constructed to accommodate four travel lanes according to the Sacramento County General Plan Transportation Map. All of the new intersections are included in Table 6.

Table 6
Existing Plus Project Intersection Levels of Service

Intersection	Traffic Control	Existing AM (PM)		Existing Plus Project AM (PM)	
		V/C Ratio, Delay or RC ¹	LOS ²	V/C Ratio, Delay or RC	LOS
S. Watt Avenue/Jackson Road	Signal	1.04 (1.12)	F (F)	1.46 (1.91)	F (F)
Bradshaw Road/Jackson Road	Signal	1.13 (0.99)	F (E)	1.34 (1.46)	F (F)
Bradshaw Road/Elder Creek Road	Signal	0.77 (0.76)	C (C)	1.25 (1.48)	F (F)
Bradshaw Road/Florin Road	Signal	0.89 (0.82)	D (D)	1.61 (2.02)	F (F)
Elk Grove-Florin Road/Gerber Road	Signal	1.05 (1.30)	F (F)	1.36 (1.58)	F (F)
Bradshaw Road/Gerber Road	Signal	0.82 (0.66)	D (B)	1.26 (1.37)	F (F)
Elk Grove-Florin Road/Calvine Road	Signal	0.52 (0.60)	A (A)	0.64 (0.73)	B (C)
Florin Road/Waterman Road	Signal	N/A	N/A	0.75 (0.92)	C (E)
Gerber Road/Waterman Road	Signal	N/A	N/A	0.66 (0.86)	B (D)
Excelsior Road/Jackson Road	Two-way Stop	239 (378)	C (B)	117 (158)	D (D)
Excelsior Road/Elder Creek Road	Two-way Stop	709 (616)	A (A)	628 (467)	A (A)
Vineyard Road/Gerber Road	Two-way Stop	567 (596)	A (A)	501 (406)	A (A)
Vineyard Road/Florin Road	Two-way Stop	N/A	N/A	209 (215)	C (C)
Excelsior Road/Gerber Road	Two-way Stop	432 (547)	A (A)	329 (397)	B (B)
Waterman Road/Calvine Road	Two-way Stop	489 (677)	A (A)	671 (737)	A (A)
Vineyard Road/Calvine Road	Two-way Stop	881 (767)	A (A)	658 (605)	A (A)
S. Watt Avenue/Elder Creek Road	All-way Stop	15.0 s (37.0 s)	C (E)	See Note 3	F (F)
Excelsior Road/Florin Road	All-way Stop	6.0 s (3.0 s)	B (A)	9.0 s (7.0 s)	B (B)
Elk Grove-Florin Road/Florin Road	All-way Stop	36.0 s (66.0 s)	E (F)	See Note 3	F (F)
Bradshaw Road/Calvine Road	All-way Stop	15.0 s (14.0 s)	C (C)	27.0 s (45.0 s)	D (E)
Excelsior Road/Calvine Road	All-way Stop	4.0 s (3.0 s)	A (A)	5.0 s (3.0 s)	A (A)

Notes: ¹ V/C Ratio = Volume-to-Capacity Ratio (0.00 applies to signalized control); Delay = Average Total Delay (0.0 seconds applies to all-way stop control); and RC= Reserve Capacity (000 vehicles applies to two-way stop control).
² LOS = Level of Service.
³ Intersection is substantially over capacity and delay calculation is meaningless.
N/A = Not applicable.

Twelve of the 19 study intersections under existing plus project conditions are projected to operate at LOS E or better. The remaining seven intersections would operate at LOS F or worse under the a.m. and/or p.m. peak hour with the proposed project. All of the unsignalized intersections operating at LOS C or worse were analyzed to determine if traffic signals would be warranted under existing plus project conditions. Based on the Caltrans Peak Hour Signal Warrant criteria, this analysis showed that traffic signals would be warranted at Bradshaw Road and Calvine Road with the addition of project traffic. As presented in Chapter II, signals are already warranted without the project at the South Watt Avenue/Elder Creek Road and Elk Grove-Florin Road/Florin Road intersections and the addition of project traffic would further degrade operations.

Each intersection deficiency under existing plus project conditions is identified in detail below.

- EP-12:** Implementation of the project would cause the intersection of South Watt Avenue at Jackson Road (SR 16) to operate at LOS F during both the a.m. and p.m. peak hour.
- EP-13:** Implementation of the project would exacerbate LOS F conditions at the Bradshaw Road/Jackson Road intersection during the a.m. peak hour and would worsen conditions from LOS E to F in the p.m. peak hour.
- EP-14:** Implementation of the project would cause the intersection of Bradshaw Road at Elder Creek Road to operate at LOS F during both the a.m. and p.m. peak hour.
- EP-15:** Implementation of the project would cause operating conditions at the Bradshaw Road/Florin Road intersection to worsen from LOS D to F during both the a.m. and p.m. peak hours.
- EP-16:** Implementation of the project would exacerbate LOS F operating conditions at the Elk Grove-Florin Road/Gerber Road intersection during the a.m. and p.m. peak hours.
- EP-17:** Implementation of the project would exacerbate LOS F operating conditions at the Bradshaw Road/Gerber Road intersection during the a.m. and p.m. peak hours.
- EP-18:** Implementation of the project would result in LOS F operations at the South Watt Avenue/Elder Creek Road intersection during the a.m. and p.m. peak hours.
- EP-19:** Implementation of the project would worsen operating conditions at the Elk Grove-Florin Road/Florin Road intersection from LOS E to F in the a.m. peak hour and would exacerbate LOS F operations in the p.m. peak hour.

TRANSIT SYSTEM OPERATIONS

Transit system operations under existing plus project conditions were analyzed by considering the potential effects of the project on existing or planned transit services and facilities. As discussed in Chapter II, there is currently no transit service in the vicinity of the project site.

Planned facilities are developed by Sacramento Regional Transit (RT), which maintains a 20-year master plan of transit facilities for the Sacramento region. This plan (last updated in April 1992), along with the County's General Plan, shows that feeder bus service (i.e., a major transit corridor) is planned for the entire length of Elk Grove-Florin Road, Bradshaw Road, and Calvine Road within the study area. Feeder service is also planned for Florin Road from west of Elk Grove-Florin Road to the planned Vineyard Road extension. Planned corridors are shown on Figure 8.

Besides providing area-wide bus service, these corridors are intended to support "trunk" transit service along the Central California Traction Railroad alignment. This corridor is currently planned for a future light rail extension, which would include stations at or near crossings of all major arterial streets; however, an interim facility on this alignment may include a high occupancy vehicle (HOV) thoroughfare for buses according to RT planning staff. RT staff recommended that bus stop locations should not be identified as part of the transportation analysis. Future planning studies will be conducted to determine transit routes and schedules.

The transit facilities included as part of the NVSSP are designed to coordinate with and maximize the potential of those transit corridors identified by RT and Sacramento County. RT guidelines recommend bus stops at one-quarter mile intervals in suburban settings, and they recommend turnouts when traffic volumes, speeds, and service frequency warrant safe stopping areas for buses. Both RT guidelines and County standards recommend locating bus stops on the far side of intersections.

Roadways within the specific plan area will be designed to accommodate transit facilities such as turnouts, bus stops, and shelters should individual routes be designated on major collector streets. Thus, implementation of the proposed project will not disrupt or interfere with existing or planned transit operations in the area and no operational deficiencies were identified.

BICYCLE AND PEDESTRIAN SYSTEM OPERATIONS

The *Draft Environmental Impact Report for the 2010 Sacramento City/County Bikeway Master Plan* (September 1992) identifies planned bicycle facilities within the study area. To maintain consistency with this document and to provide a substantive network of bikeways for alternative transportation, the circulation plan for the NVSSP proposes on-street bike lanes for all streets bordering the plan area, as well as on the major arterial and collector street segments within the project. In addition, sidewalks will be constructed on all streets within the plan area and on the roadways bordering the project site. Several off-street bicycle/pedestrian trails will be constructed within the specific plan area to encourage alternative modes of travel and to improve internal circulation. These facilities would enhance the existing bicycle and pedestrian systems and result in a beneficial impact to the study area. Planned bikeways and pedestrian paths identified in the *2010 Bikeway Master Plan* and those proposed as part of the NVSSP are illustrated on Figure 9.

Similar to the transit system, the project does not directly disrupt, interfere or conflict with existing or planned bicycle or pedestrian facilities; consequently, no operational deficiencies were identified.

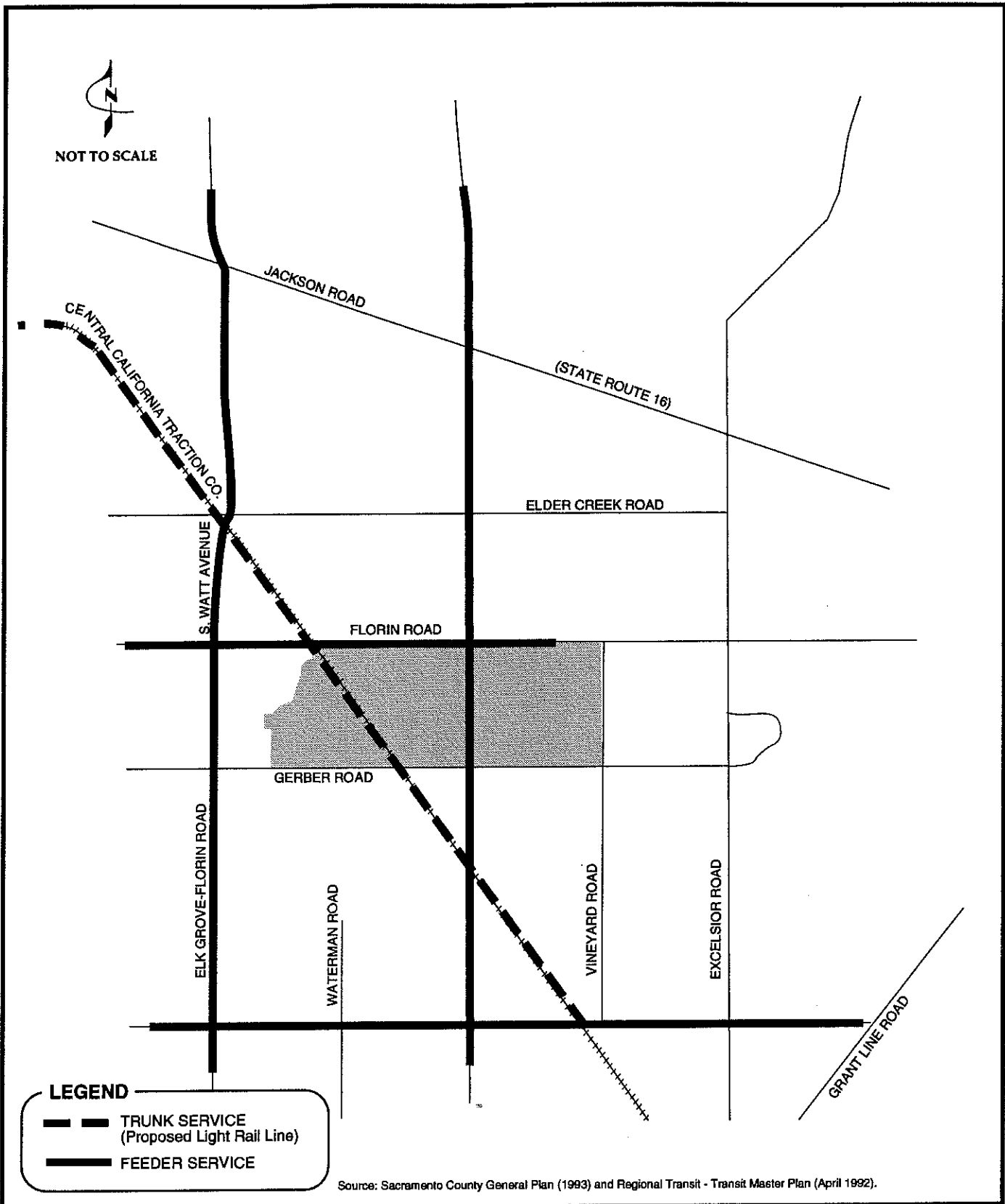


FIGURE 8

FUTURE TRANSIT SERVICE

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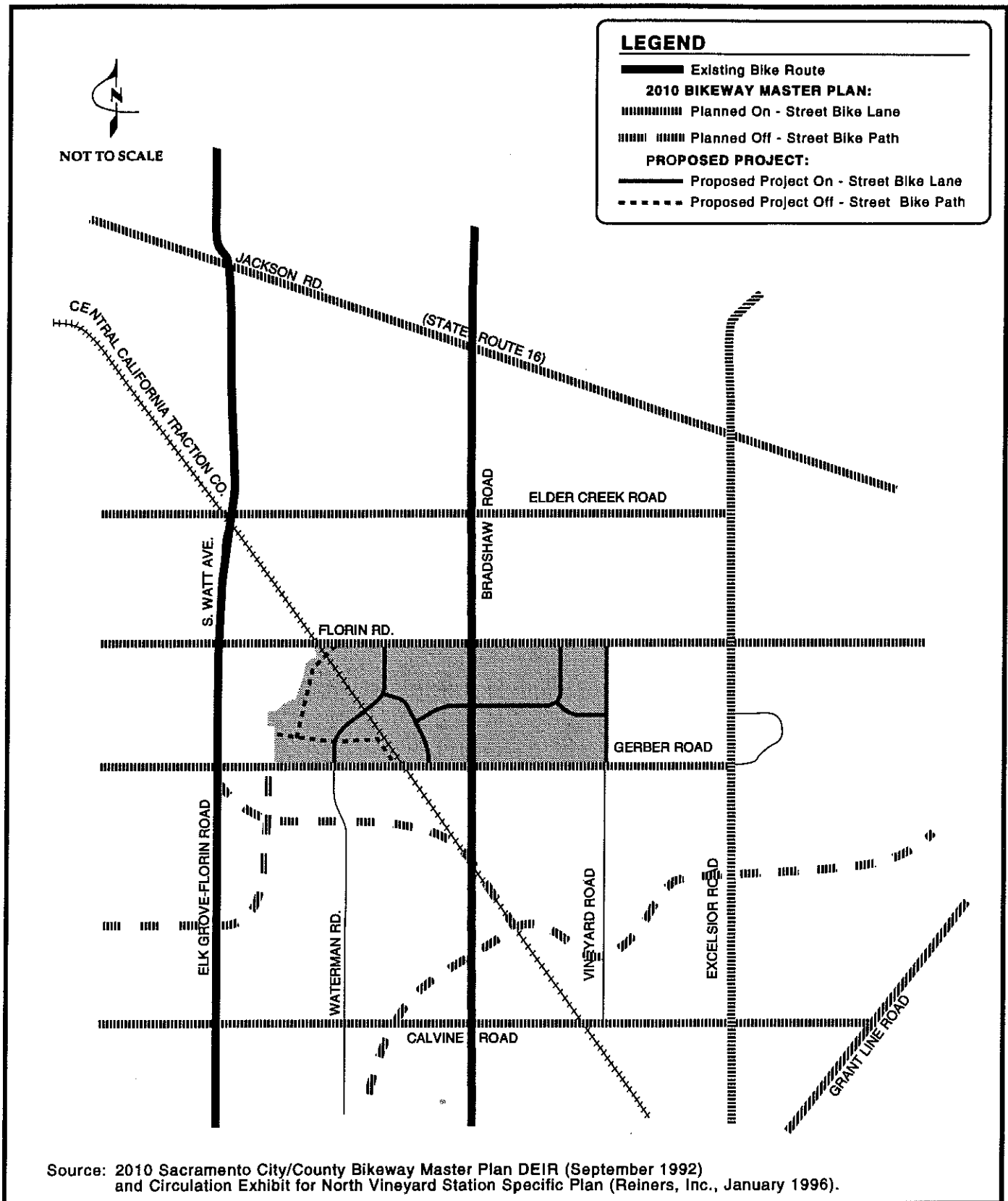


FIGURE 9

FUTURE BICYCLE FACILITIES

PROPOSED IMPROVEMENTS

The following improvements are proposed to enhance operating conditions or to eliminate the projected deficiencies identified above. Each improvement is listed according to its reference number.

- Improvement EP-1: Widen the section of Elk Grove-Florin Road/South Watt Avenue from Gerber Road to SR 16 from two to four lanes. Based on Sacramento County guidelines, this improvement would provide sufficient capacity to accommodate the projected daily volume. This improvement should be constructed when the daily volume on this segment reaches 16,200 VPD (90 percent of urban arterial capacity).
- Improvement EP-2: Widen the section of Bradshaw Road from Gerber Road to SR 16 from two to four lanes. Based on Sacramento County guidelines, this improvement would provide sufficient capacity to accommodate the projected daily volume. This improvement is part of a funded project that will be constructed by year 2001 according to Sacramento County's *1996 Transportation Improvement Plan*.
- Improvement EP-3: Widen the section of Florin Road from South Watt Avenue to Bradshaw Road from two to four lanes. Based on Sacramento County guidelines, this improvement would provide sufficient capacity to accommodate the projected daily volume. This improvement should be constructed when the daily volume on this segment reaches 16,200 VPD (90 percent of urban arterial capacity).
- Improvement EP-4: Widen the section of Gerber Road from Elk Grove-Florin Road to Bradshaw Road from two to four lanes. Based on Sacramento County guidelines, this improvement would provide enough capacity to accommodate the projected daily volume. This improvement should be constructed when the volume on this segment reaches 16,200 VPD (90 percent of urban arterial capacity).
- Improvement EP-5: Widen the section of SR 16 from South Watt Avenue to Bradshaw Road from two to four lanes. Based on Sacramento County guidelines, this improvement would provide sufficient capacity to accommodate the projected daily volume. This improvement should be constructed when the daily volume on this segment reaches 16,200 VPD or 90 percent of urban arterial capacity.
- Improvement EP-6: Widen Elder Creek Road between South Watt Avenue and Excelsior Road to include 12-foot travel lanes and minimum 6-foot shoulders to meet Sacramento County design standards for rural arterials. This improvement will eliminate the existing physical deficiency and the deficiency with the project.

- Improvement EP-7: Widen Excelsior Road between Jackson Road and Calvine Road to include 12-foot travel lanes and minimum 6-foot shoulders to meet Sacramento County design standards for rural arterials. This improvement will eliminate the existing physical deficiency and the deficiency with the project.
- Improvement EP-8: Widen Florin Road between Bradshaw Road and Excelsior Road to include 12-foot travel lanes and minimum 6-foot shoulders to meet Sacramento County design standards for rural arterials. This improvement will eliminate the existing physical deficiency and the deficiency with the project.
- Improvement EP-9: Widen Gerber Road Bradshaw Road and Excelsior Road to include 12-foot travel lanes and minimum 6-foot shoulders to meet Sacramento County design standards for rural arterials. This improvement will eliminate the existing physical deficiency and the deficiency with the project.
- Improvement EP-10: Widen Vineyard Road between Gerber Road and Calvine Road to include 12-foot travel lanes and minimum 6-foot shoulders to meet Sacramento County design standards for rural arterials. This improvement will eliminate the existing physical deficiency and the deficiency with the project. Widening of this roadway is anticipated as part of frontage improvements in the Vineyard Springs Comprehensive Planning Area.
- Improvement EP-11: Widen Calvine Road between approximately 1,300 feet east of Waterman Road to Excelsior Road to include 12-foot travel lanes and minimum 6-foot shoulders to meet Sacramento County design standards for rural arterials. The section of Calvine Road from Elk Grove-Florin Road to approximately 1,300 feet east of Waterman Road will be widened from two to four lanes as part of a funded project included in Sacramento County's *1996 Transportation Improvement Plan*. This improvement will eliminate the existing physical deficiency and the deficiency with the project.
- Improvement EP-12: Modify the intersection of South Watt Avenue at Jackson Road to include a separate left turn lane, two through lanes, and a separate right turn lane on all approaches. These measures would improve operations at this intersection to LOS D and LOS E in the a.m. and p.m. peak hours, respectively. Improvements should be implemented when the intersection operates unacceptably based on Sacramento County standards.
- Improvement EP-13: Modify the Bradshaw Road/Jackson Road intersection to include the following lane configurations:

- Dual left turn lanes, two through lanes, and a separate right turn lane on the northbound and southbound approaches; and
- One left turn lane, one through lane, and a separate right-turn lane on the eastbound and westbound approaches.

The provision of northbound and southbound dual left turn lanes will require widening of the eastbound and westbound departure legs on Jackson Road to two lanes before merging to a single travel lane. The length and merging distance for these lanes will be approved by Caltrans since Jackson Road is a state highway.

These improvements would improve operations at this intersection to LOS E in the a.m. and p.m. peak hours. According to Sacramento County's *1996 Transportation Improvement Plan*, two northbound through lanes and one southbound through lane will be constructed by 1997. The remaining improvements should be implemented when the intersection operates unacceptably based on Sacramento County standards.

Improvement EP-14:

Modify the Bradshaw Road/Elder Creek Road intersection to include the following lane configurations:

- One left turn lane, two through lanes, and a separate right turn lane on the southbound approach;
- One left turn lane, one through lane, and a shared through/right-turn lane on the northbound approach;
- One left turn lane and one shared through/right-turn lane on the eastbound approach; and
- One shared left-turn/through lane and one separate right turn lane on the westbound approach.

These improvements would improve operations at this intersection to LOS C and LOS D in the a.m. and p.m. peak hours, respectively. Improvements should be implemented when the intersection operates unacceptably based on Sacramento County standards.

Improvement EP-15:

Modify the Bradshaw Road/Florin Road intersection to include the following lane configurations:

- Dual left turn lanes, two through lanes, and a separate right turn lane on the northbound and southbound approaches; and

The provision of southbound dual left turn lanes will require widening of the eastbound departure legs on Florin Road to two lanes before merging to a single travel lane. The length and merging distance for these lanes will be approved by Sacramento County Transportation Division staff. The westbound departure leg will be widened as part of Improvement EP-3.

These improvements would improve operations at this intersection to LOS C and LOS E in the a.m. and p.m. peak hours, respectively. Improvements should be implemented when the intersection operates unacceptably based on Sacramento County standards.

Improvement EP-16: Modify the Elk Grove-Florin Road/Gerber Road intersection to include the following lane configurations:

- Dual left turn lanes, two through lanes, and a separate right turn lane on the northbound and southbound approaches;
- One left turn lane, one through lane, and a separate right-turn lane on the eastbound approach; and
- One left turn lane, one through lane, and a shared through/right-turn lane on the westbound approach.

The provision of northbound dual left turn lanes will require widening of the westbound departure leg on Gerber Road to two lanes before merging to a single travel lane in each direction. The length and merging distance for these lanes will be approved by Sacramento County Transportation Division staff.

These improvements would improve operations at this intersection to LOS D and LOS E in the a.m. and p.m. peak hours, respectively. Improvements should be implemented when the intersection operates unacceptably based on Sacramento County standards.

Improvement EP-17: Modify the Bradshaw Road/Gerber Road intersection to include the following lane configurations:

- Dual left-turn lanes, ~~two~~ one through lanes, and a separate right-turn lane on the eastbound and westbound approaches; and
- One left-turn lane, two through lanes, and a separate right-turn lane on the northbound and southbound approaches.

The provision of **two southbound ~~dual~~ left-turn through lanes and dual westbound left-turn lanes** will require widening of the **eastbound southbound** departure leg on **Gerber Road Bradshaw Road** to two lanes before merging to a single travel lane in each direction. The length and merging distance for these lanes shall be approved by Sacramento County Transportation Division staff.

These improvements would improve operations at this intersection to LOS D and LOS C in the a.m. and p.m. peak hours, respectively. Improvements should be implemented when the intersection operates unacceptably based on Sacramento County standards.

Improvement EP-18: Install a signal at the South Watt Avenue/Elder Creek Road intersection and modify the lane configurations to include the following:

- One left turn lane, two through lanes, and a separate right turn lane on the northbound and southbound approaches; and
- One left turn lane and a shared through/right-turn lane on the eastbound and westbound approaches.

These improvements would improve operations at this intersection to LOS B and LOS D in the a.m. and p.m. peak hours, respectively. (It should be noted that a signal is warranted at this location under existing conditions without the NVSSP). Improvements should be implemented with construction of the project.

Improvement EP-19: Install a signal at the Elk Grove-Florin Road/Florin Road intersection and modify the lane configurations to include:

- Dual left-turn lanes, two through lanes, and a separate right turn lane on the northbound and southbound approaches; and
- One left-turn lane, two through lanes, and a separate right-turn lane on the eastbound and westbound approaches.

These improvements would improve operations at this intersection to LOS B and LOS D in the a.m. and p.m. peak hours, respectively. (It should be noted that a signal is warranted at this location under existing conditions without the NVSSP). The traffic signal is a funded project that will be constructed by 1998 according to Sacramento County's *1996 Transportation Improvement Plan*.

OPERATIONS WITH PROPOSED IMPROVEMENTS

Following implementation of the proposed improvements, the associated operational deficiencies would be eliminated. Table 7 shows the LOS at the study intersections after implementation of the proposed improvements.

**Table 7
Existing Plus Project Intersection Levels of Service with Improvements**

Intersection	Traffic Control	AM Peak Hour		PM Peak Hour	
		V/C Ratio, Delay or RC ¹	LOS ²	V/C Ratio, Delay or RC	LOS
S. Watt Avenue/Jackson Road	Signal	0.82	D	0.94	E
Bradshaw Road/Jackson Road	Signal	0.90	E	0.91	E
Bradshaw Road/Elder Creek Road	Signal	0.71	C	0.89	D
Bradshaw Road/Florin Road	Signal	0.78	C	0.99	E
Elk Grove-Florin Road/Gerber Road	Signal	0.85	D	1.00	E
Bradshaw Road/Gerber Road	Signal	0.85	D	0.72	C
Elk Grove-Florin Road/Calvine Road	Signal	0.64	B	0.73	C
Florin Road/Waterman Road	Signal	0.75	C	0.92	E
Gerber Road/Waterman Road	Signal	0.66	B	0.86	D
Excelsior Road/Jackson Road	Two-way Stop	117	D	158	D
Excelsior Road/Elder Creek Road	Two-way Stop	628	A	467	A
Vineyard Road/Gerber Road	Two-way Stop	501	A	406	A
Vineyard Road/Florin Road	Two-way Stop	209	C	215	C
Excelsior Road/Gerber Road	Two-way Stop	329	B	397	B
Waterman Road/Calvine Road	Two-way Stop	671	A	737	A
Vineyard Road/Calvine Road	Two-way Stop	658	A	605	A
S. Watt Avenue/Elder Creek Road	Signal	0.62	B	0.84	D
Excelsior Road/Florin Road	All-way Stop	9.0 s	B	7.0 s	B
Elk Grove-Florin Road/Florin Road	Signal	0.67	B	0.89	D
Bradshaw Road/Calvine Road	All-way Stop	27.0 s	D	45.0 s	E
Excelsior Road/Calvine Road	All-way Stop	5.0 s	A	3.0 s	A

Notes: ¹ V/C Ratio = Volume-to-Capacity Ratio (0.00 applies to signalized control); Delay = Average Total Delay (0.0 seconds applies to all-way stop control); and RC= Reserve Capacity (000 vehicles applies to two-way stop control).
² LOS = Level of Service.
³ Intersection is substantially over capacity and delay calculation is meaningless.
N/A = Not applicable

IV. CUMULATIVE OPERATING CONDITIONS

The purpose of the cumulative analysis is to determine if implementation of the proposed project, in addition to planned cumulative growth, will adversely affect the planned transportation system.

PLANNED TRANSPORTATION IMPROVEMENTS

Roadway Improvements

The cumulative analysis determines if planned roadway improvements in the study area are sufficient to accommodate anticipated traffic growth. The planned roadway improvements in the vicinity of the project site were obtained from the Sacramento County General Plan Transportation Map (December 15, 1993) and are listed below.

- Jackson Road/State Route (SR) 16 will be widened to six lanes from west of South Watt Avenue to Bradshaw Road. From Bradshaw Road to east of Excelsior Road, SR 16 is planned to be four lanes.
- Elk Grove-Florin Road/South Watt Avenue will be widened to six lanes from south of Calvine Road to north of Jackson Road/SR 16.
- Bradshaw Road will be widened to six lanes from Calvine Road to north of Jackson Road/SR 16. South of Calvine Road, Bradshaw Road will be a four-lane facility.
- Vineyard Road will be widened to four lanes from Calvine Road to Gerber Road. Vineyard Road will be extended to Florin Road as a four-lane facility.
- Florin Road will be widened to six lanes from west of Elk Grove-Florin Road/South Watt Avenue to the planned Vineyard Road extension. From the Vineyard Road extension to east of Excelsior Road, Florin Road will be widened to four lanes.
- Gerber Road will be widened to four lanes from west of Elk Grove-Florin Road to the planned Vineyard Road extension. From the Vineyard Road extension to east of Excelsior Road, Gerber Road will include two travel lanes. This two lane section is planned to be widened to four lanes after 2010.
- Calvine Road will be widened to six lanes from west of Elk Grove-Florin Road to Grant Line Road.
- Elder Creek Road will be widened to four lanes from west of South Watt Avenue to Bradshaw Road. East of Bradshaw Road, Elder Creek Road will continue to function as a two-lane roadway.

Figure 10 illustrates the planned number of lanes on each roadway segment under cumulative conditions. All of the study intersections are assumed to be signalized under this scenario.

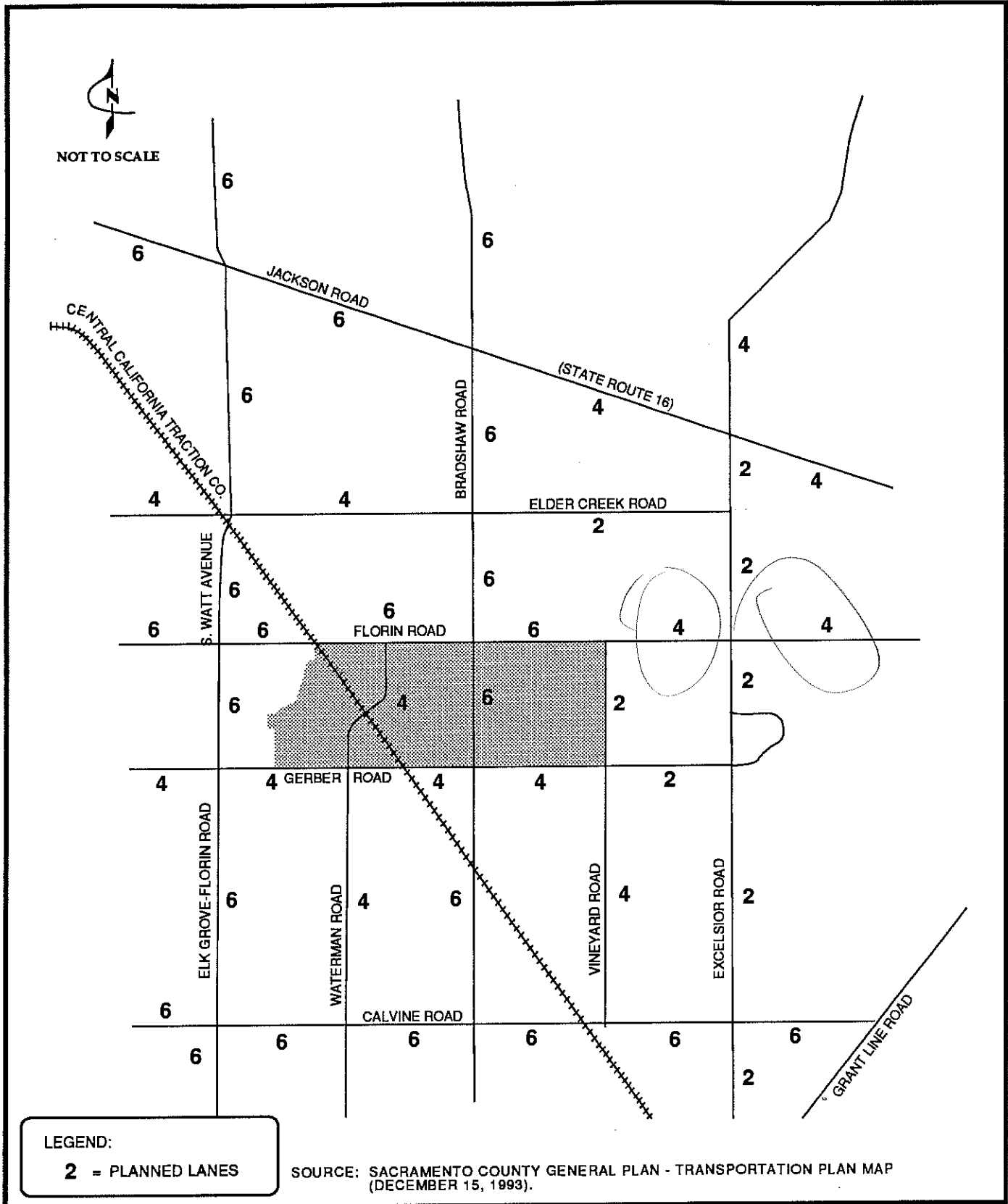


FIGURE 10

PLANNED CUMULATIVE ROADWAY NETWORK

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Funding for most of the improvements listed above will ultimately be provided by a financing plan for the study area, other developer fees, and public sources. Public funds will be necessary for a fair share contribution to improvements that eliminate existing deficiencies.

Other Improvements

- Transit facilities will be integrated as part of the NVSSP project. The study area is expected to be served by Sacramento Regional Transit (RT) and planned facilities will include bus stops, bus shelters, and turnouts. A multimodal transfer facility and park and ride lot is planned within the project site to accommodate light rail service if it is constructed within the Central California Traction Railroad (CCTR) right-of-way.
- The NVSSP area will include various on-street bike lanes and off-street bike paths and pedestrian trails. These facilities will complement the continuous system of on-street bike lanes planned for the study area roadways shown on Figure 6. In addition, off-street pedestrian and bicycle trails will be constructed along Elder Creek and Gerber Creek west of the CCTR tracks. All bicycle and pedestrian facilities will interconnect parks, residential areas, and commercial developments.

ROADWAY SYSTEM OPERATIONS

Cumulative No Project Volumes

The Sacramento County General Plan traffic model was used to generate background intersection traffic forecasts without the NVSSP. Forecasts were prepared using the latest version of the General Plan model, which was recently updated for the East Elk Grove Specific Plan study. Based on additional review of the traffic model, the County requested that certain areas in the immediate vicinity of the NVSSP area be updated to better reflect more recent and detailed land use data. This information was provided by the Sacramento County Planning Department for the Vineyard Springs Comprehensive Planning Area to the southeast (between Gerber Road, Calvine Road, Bradshaw Road, and Excelsior Road) and for the Florin Station Comprehensive Planning Area to the west (between Florin Road, Gerber Road, Gardner Avenue, and Elder Creek). Inclusion of this land use was necessary to better reflect potential development and to improve master planning of the ultimate roadway system. In addition, land use updates were incorporated in the traffic model for the eastern part of Sacramento County near the El Dorado County line, as well as for the Mather Field Reuse Area and Zinfandel Villages projects north of the project site.

The County's traffic model was originally developed and calibrated based on daily conditions only. However, Transportation Division staff requested that peak hour turning movement forecasts be prepared to determine if any non-standard traffic control (e.g., dual right turn lanes, triple left turn lanes) would be required under cumulative conditions. Peak hour forecasts were prepared by factoring daily trip data by trip purpose. This method is consistent with the approach used for other long-range planning studies in the vicinity of the project.

The cumulative no project daily and peak hour traffic volumes are shown on Figures 11 and 12, respectively. The intersection lane configurations shown on Figure 12 are based on Sacramento County design standards for thoroughfares, arterials, and collector streets.

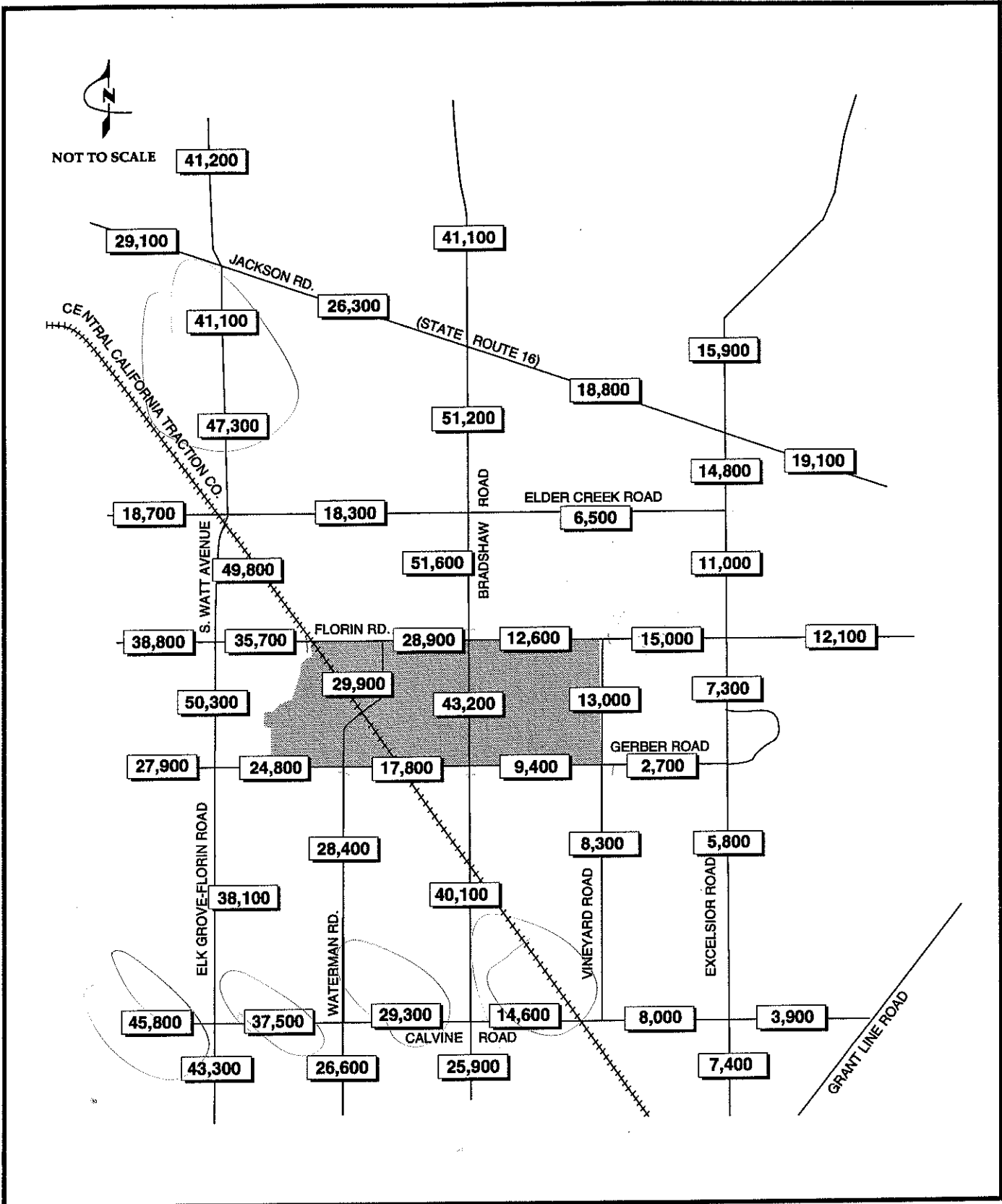


FIGURE 11

CUMULATIVE NO PROJECT DAILY VOLUMES

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Cumulative Plus Project Volumes

Cumulative plus project volumes were developed by adding the proposed land uses to the County's traffic model and preparing a new set of traffic forecasts. The same peak hour factors used to develop the cumulative no project forecasts were applied for this scenario.

At the direction of County Transportation Division staff, the daily and peak hour trips generated by the proposed project were factored to match the trip generation information presented in Tables A-1 through A-3. This process provided a conservative trip generation estimate for purposes of identifying project-related impacts and is more consistent with standard trip rates.

The trip distribution of project-generated trips on the study roadway network was obtained from the traffic model output. This information will be used primarily to identify project impacts for roadway segments. Figure 13 shows the project trip distribution generated by the traffic model.

Figures 14 and 15 present the daily and peak hour traffic volumes, respectively, for cumulative conditions with the NVSSP in place. Impacts to each element of the study roadway system are presented below.

Arterial Roadway Segments

Under cumulative no project conditions, none of the study roadway segments is projected to carry volumes beyond the theoretical urban roadway capacities of 18,000 VPD, 36,000 VPD, and 54,000 VPD for two-lane, four-lane, and six-lane facilities, respectively. The highest daily volume without the project is 51,600 VPD, which is projected for the segment of Bradshaw Road between Florin Road and Elder Creek Road. Assuming a maximum threshold of 54,000 VPD according to County guidelines, this section of roadway would operate under capacity.

Figure 14 shows that the proposed project would increase traffic on most roadway segments in the study area. These increases would be caused by: 1) the addition of new trips within the study area, and 2) the displacement of traffic from some segments caused by the addition of new trips. However, only the segment of Bradshaw Road between Florin Road and Elder Creek Road is projected to serve a volume in excess of its theoretical operating capacity under cumulative plus project conditions. This deficiency is described below.

CP-1: The project volume of 58,500 VPD on Bradshaw Road between Florin Road and Elder Creek Road exceeds the theoretical capacity of 54,000 VPD for a six-lane roadway.

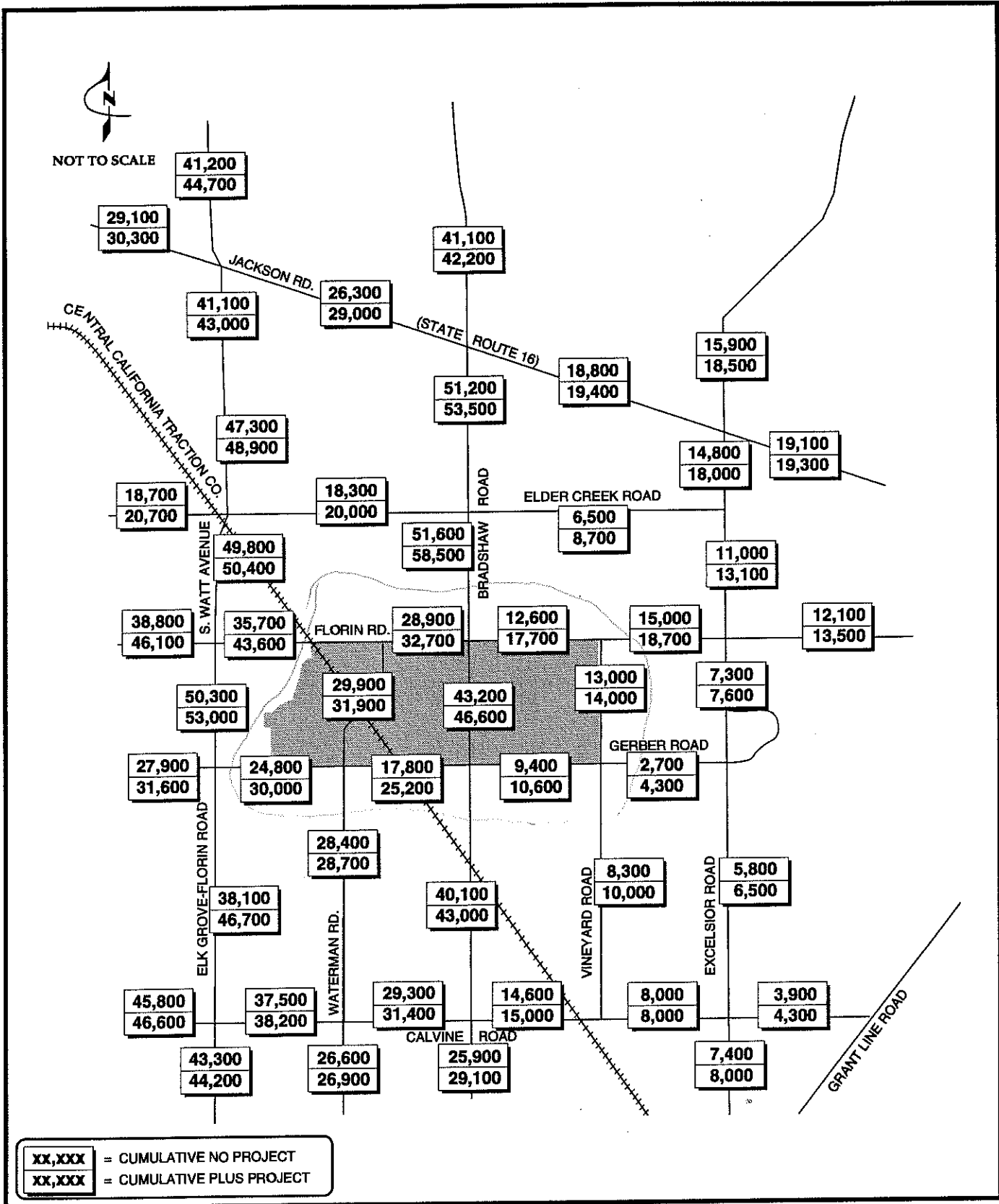
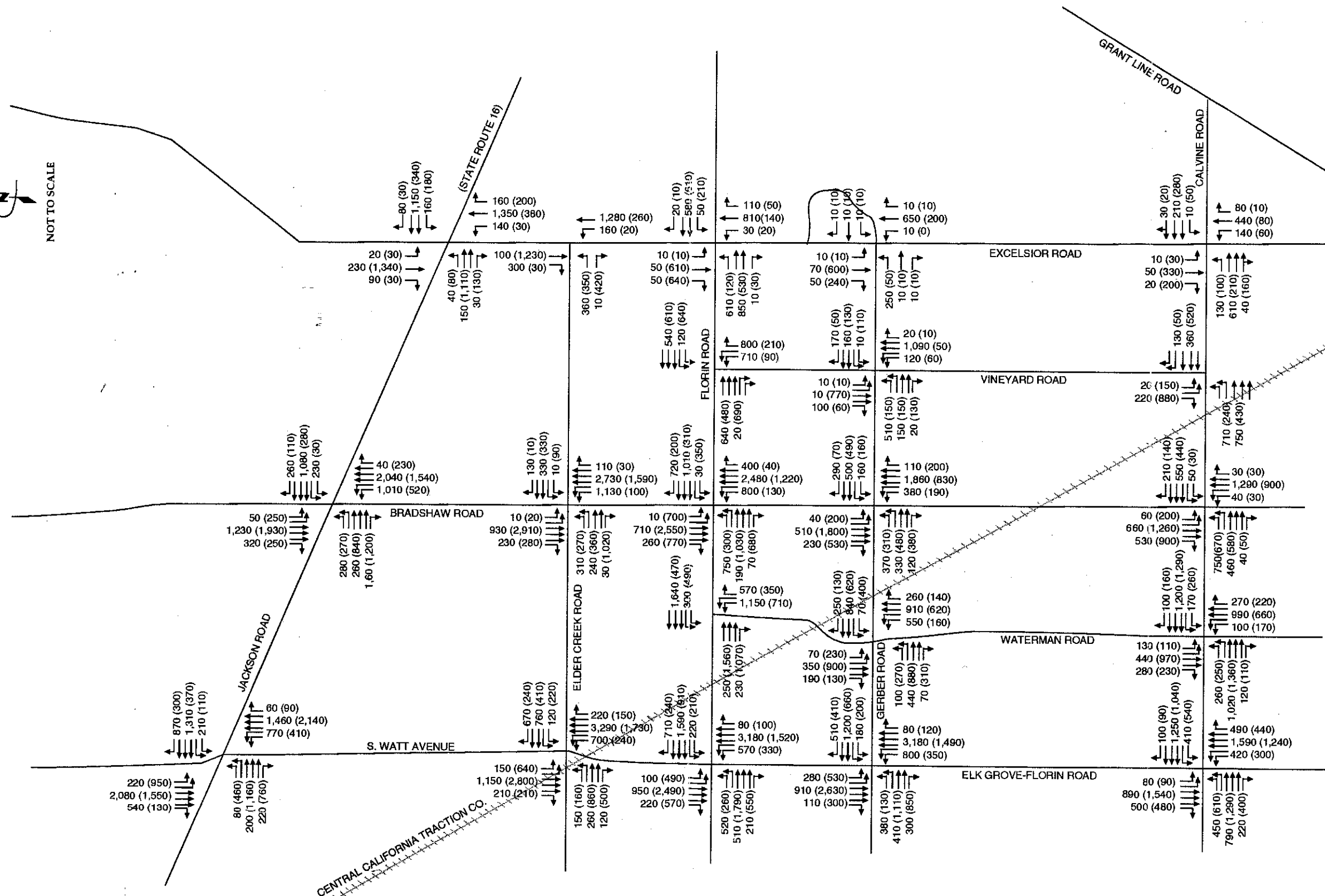


FIGURE 14

CUMULATIVE PLUS PROJECT DAILY VOLUMES

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LEGEND
 XXX = AM PEAK HOUR
 (YYY) = PM PEAK HOUR

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 Transportation Consultants

**CUMULATIVE PLUS PROJECT
 PEAK HOUR TRAFFIC VOLUMES**

FIGURE 15

Physical deficiencies are projected to occur on those segments that are planned to be two-lane facilities and will not be improved through 2010. These deficiencies are presented below.

- CP-2: Implementation of the project would add approximately 2,200 VPD to Elder Creek Road between Bradshaw Road and Excelsior Road. The addition of this traffic would exacerbate the existing physical deficiency caused by the substandard travel lane and shoulder widths.
- CP-3: Implementation of the project would add approximately 300 to 3,200 VPD to Excelsior Road between Jackson Road (SR 16) and Calvine Road. Except in the vicinity of Birch Ranch Road, the addition of this traffic would exacerbate the existing physical deficiency caused by the substandard travel lane and shoulder widths.
- CP-4: Implementation of the project would add approximately 1,600 VPD to Gerber Road between Vineyard Road and Excelsior Road. The addition of this traffic would exacerbate the existing physical deficiency caused by the substandard travel lane and shoulder widths.

Study Intersections

The peak hour volumes presented on Figures 12 and 15 were used to determine the level of service for cumulative no project and cumulative plus project conditions, respectively. The results of the a.m. and p.m. peak hour analysis are shown in Table 8 and the corresponding technical calculations are contained in Appendices D and E.

**Table 8
Cumulative Intersection Levels of Service**

Intersection	Traffic Control	Cumulative No Project AM (PM)		Cumulative Plus Project AM (PM)	
		V/C Ratio ¹	LOS ²	V/C Ratio	LOS
S. Watt Avenue/Jackson Road	Signal	1.26 (1.21)	F (F)	1.29 (1.33)	F (F)
Bradshaw Road/Jackson Road	Signal	1.01 (1.10)	F (F)	1.08 (1.30)	F (F)
Jackson Road/Excelsior Road	Signal	1.30 (1.24)	F (F)	1.44 (1.53)	F (F)
S. Watt Avenue/Elder Creek Road	Signal	1.17 (0.99)	F (E)	1.26 (1.18)	F (F)
Bradshaw Road/Elder Creek Road	Signal	0.85 (1.02)	D (F)	0.92 (1.30)	E (F)
Excelsior Road/Elder Creek Road	Signal	0.95 (0.78)	E (C)	1.15 (1.12)	F (F)
Elk Grove-Florin Road/Florin Road	Signal	1.28 (1.04)	F (F)	1.40 (1.25)	F (F)
Florin Road/Waterman Road	Signal	0.62 (0.89)	B (D)	0.67 (0.99)	B (E)
Bradshaw Road/Florin Road	Signal	1.17 (1.10)	F (F)	1.27 (1.16)	F (F)
Florin Road/Vineyard Road	Signal	0.59 (0.53)	A (A)	0.59 (0.69)	A (B)
Florin Road/Excelsior Road	Signal	0.97 (0.80)	E (C)	1.25 (0.80)	F (C)
Elk Grove-Florin Road/Gerber Road	Signal	1.31 (1.20)	F (F)	1.47 (1.29)	F (F)
Waterman Road/Gerber Road	Signal	0.87 (0.81)	D (D)	0.70 (0.87)	B (D)
Bradshaw Road/Gerber Road	Signal	0.84 (0.88)	D (D)	0.80 (0.82)	C (D)
Vineyard Road/Gerber Road	Signal	0.72 (0.32)	C (A)	0.69 (0.41)	B (A)
Excelsior Road/Gerber Road	Signal	0.65 (0.55)	B (A)	0.67 (0.48)	B (A)
Elk Grove-Florin Road/Calvine Road	Signal ³	0.88 (0.99)	D (E)	0.91 (1.02)	E (F)
Calvine Road/Waterman Road	Signal	0.79 (0.79)	C (C)	0.81 (0.85)	D (D)
Calvine Road/Bradshaw Road	Signal	0.81 (0.77)	D (C)	0.77 (0.85)	C (D)
Calvine Road/Vineyard Road	Signal	0.61 (0.59)	B (A)	0.47 (0.65)	A (B)
Calvine Road/Excelsior Road	Signal	0.48 (0.49)	A (A)	0.49 (0.44)	A (A)

Notes: ¹ V/C Ratio = Volume-to-Capacity Ratio.

² LOS = Level of Service.

³ Although an urban interchange was identified at this location as part of the General Plan, intersection LOS was calculated assuming a standard arterial intersection configuration (two left turn lanes, three through lanes and a separate right turn lane on each approach).

Under cumulative conditions without the project, eight of the 21 study intersections are projected to operate at an unacceptable level (LOS F) during one or both peak hours. These locations and the affected peak hour(s) are listed below .

- South Watt Avenue at Jackson Road (a.m. and p.m. peak hour)
- Bradshaw Road/Jackson Road (a.m. and p.m. peak hour)
- Jackson Road/Excelsior Road (a.m. and p.m. peak hour)
- S. Watt Avenue/Elder Creek Road (a.m. peak hour)
- Bradshaw Road/Elder Creek Road (p.m. peak hour)
- Elk Grove-Florin Road/Florin Road (a.m. and p.m. peak hour)
- Bradshaw Road/Florin Road (a.m. and p.m. peak hour)
- Elk Grove-Florin Road/Gerber Road (a.m. and p.m. peak hour)

All other intersections operate at LOS E or better during both peak hours under cumulative conditions without the proposed project.

With the addition of project-generated traffic, three additional study intersections are projected to operate at LOS F during one or both peak hours. These locations include the following.

- Excelsior Road/Elder Creek road (a.m. and p.m. peak hour)
- Florin Road/Excelsior Road (a.m. peak hour)
- Elk Grove-Florin Road/Calvine Road (p.m. peak hour)

Each intersection deficiency under cumulative plus project conditions is detailed below.

- CP-5:** Implementation of the project would result in LOS F operations in the a.m. peak hour at the South Watt Avenue/Jackson Road intersection; however the volume-to-capacity ratio would be between seven and 15 percent less as compared to operations under cumulative no project conditions.
- CP-6:** Implementation of the project would exacerbate LOS F operations at the Bradshaw Road/Jackson Road intersection during the a.m. and p.m. peak hour.
- CP-7:** Implementation of the project would exacerbate LOS F operations at the Jackson Road/Excelsior Road intersection during the a.m. and p.m. peak hour.
- CP-8:** Implementation of the project would exacerbate LOS F operations at the South Watt Avenue/Elder Creek Road intersection during the a.m. peak hour, and would cause p.m. peak hour operations to deteriorate from LOS E to F.
- CP-9:** Implementation of the project would exacerbate LOS F operations at the Bradshaw Road/Elder Creek Road intersection during the p.m. peak hour.
- CP-10:** Implementation of the project would degrade operations at the Excelsior Road/Elder Creek Road intersection from LOS E to F during the a.m. peak hour and from LOS C to F in the p.m. peak hour.

CP-11: Implementation of the project would exacerbate LOS F operations at the Elk Grove-Florin Road/Florin Road intersection during the a.m. and p.m. peak hour.

CP-12: Implementation of the project would exacerbate LOS F operations at the Bradshaw Road/Florin Road intersection during the a.m. and p.m. peak hour.

CP-13: Implementation of the project would degrade operations at the Florin Road/Excelsior Road intersection from LOS E to F in the a.m. peak hour.

CP-14: Implementation of the project would exacerbate LOS F operations at the Elk Grove-Florin Road/Gerber Road intersection during the a.m. and p.m. peak hour.

CP-15: Implementation of the project would degrade operations at the Elk Grove-Florin Road/Calvine Road intersection from LOS E to F in the p.m. peak hour.

TRANSIT SYSTEM OPERATIONS

Transit system operations under cumulative plus project conditions were analyzed by considering the potential effects of the project on planned transit services and facilities. As part of the NVSSP, various transit services will be provided by Regional Transit (RT) within the study area. Services may include peak and off-peak bus operations, express buses to downtown Sacramento, bus shelters and turnouts.

Facilities and services are anticipated on Florin, Bradshaw, Gerber, Waterman, and Vineyard Roads, as well as on major collectors within the plan area. As noted in Chapter III, the proposed circulation plan will be reviewed by RT to identify potential route corridors within the plan area, as well locations for turnouts and major transit centers. Thus, implementation of the proposed project will not disrupt or interfere with expected transit operations in the area and no cumulative deficiencies were identified.

BIKEWAY AND PEDESTRIAN SYSTEM OPERATIONS

Bikeway and pedestrian system operations under cumulative plus project conditions were analyzed by considering the potential effects that implementation of the project could have on planned bikeway and pedestrian facilities. As previously mentioned, both on-street and off-street bikeway and pedestrian facilities are planned throughout the study area. These facilities include an off-street bicycle/pedestrian trail along Elder and Gerber Creeks west of Bradshaw Road and bicycle lanes on numerous collector and local streets providing connections to major streets adjacent to the plan area. Since these facilities enhance the bicycle and pedestrian system within the study area, no adverse cumulative bikeway and pedestrian system impacts were identified.

PROPOSED IMPROVEMENTS

The following improvements are proposed to enhance operating conditions or to eliminate projected deficiencies discussed above. Each improvement is listed according to its reference number.

Improvement CP-1: Access on Bradshaw Road should be strictly limited between Florin Road and Elder Creek Road. With the exception of signalized intersections, limited driveway access should be permitted along this roadway segment.

Reducing the number of conflicting turning movements helps to maximize the available capacity and improve traffic flow in the corridor. Although this improvement would help to reduce the projected deficiency, this segment of Bradshaw Road would continue to serve traffic volumes that exceed County guidelines for roadway capacity.

Improvement CP-2: Widen Elder Creek Road between South Watt Avenue and Excelsior Road to include 12-foot travel lanes and minimum 6-foot shoulders to meet Sacramento County design standards for rural arterials. This improvement will eliminate the physical deficiency under cumulative no project conditions and the deficiency with the project in place.

Improvement CP-3: Widen Excelsior Road between Jackson Road and Calvine Road to include 12-foot travel lanes and minimum 6-foot shoulders to meet Sacramento County design standards for rural arterials. This improvement will eliminate the physical deficiency under cumulative no project conditions and the deficiency with the project in place.

Improvement CP-4: Widen Gerber Road between Vineyard Road and Excelsior Road to include 12-foot travel lanes and minimum 6-foot shoulders to meet Sacramento County design standards for rural arterials. This improvement will eliminate the physical deficiency under cumulative no project conditions and the deficiency with the project in place.

Improvement CP-5: None of the left or right turn volumes at the South Watt Avenue/Jackson Road intersection warrant non-standard traffic controls (e.g., triple left turn lanes, dual right turn lanes, "free" right turn lanes). Construction of a fourth through lane on the northbound and southbound approaches would improve the LOS to E or better; however, Sacramento County does not typically construct eight-lane roadway segments except under special circumstances (e.g., adjacent to a freeway interchange). Thus, the proposed widening of South Watt Avenue is not considered feasible.

Improvement CP-6: Construct dual right turn lanes on the eastbound approach and triple left turn lanes on the northbound approach of the Bradshaw Road/Jackson Road intersection. Construction of these improvements would result in LOS E operations during both peak hours. Installation of these improvements should occur when the V/C ratio exceeds 1.00 or the peak hour volume for either movement exceeds roughly 950 vehicles.

Improvement CP-7: Modify the northbound and southbound approaches to the Jackson Road/Excelsior Road intersection to include a second through lane. Since the northbound departure leg of this intersection is planned to accommodate two lanes, this improvement would only require modifications to Excelsior Road south of Jackson Road. The southbound departure leg would have to be widened to accommodate two lanes before merging into a single southbound lane. The length and merging distance for the second southbound lane will be approved by Sacramento County Transportation Division staff.

This intersection modification would improve the cumulative plus project LOS from F to E during the a.m. peak hour. The p.m. peak hour LOS with this improvement would still be F; however, the V/C ratio with the project would be 1.04 or 19 percent less than the ratio of 1.24 under no project conditions. These improvements should be implemented when intersection operation exceeds LOS E.

Improvement CP-8: None of the left or right turn volumes at the South Watt Avenue/Elder Creek Road intersection warrant non-standard traffic controls (e.g., triple left turn lanes, dual right turn lanes, "free" right turn lanes). Construction of a fourth through lane on the northbound and southbound approaches would improve the LOS to E or better; however, Sacramento County does not typically construct eight-lane roadway segments except under special circumstances (e.g., adjacent to a freeway interchange). Thus, the proposed widening of South Watt Avenue is not considered feasible and no improvements are recommended.

Improvement CP-9: Construct dual right turn lanes on the eastbound approach of the Bradshaw Road/Elder Creek Road intersection. Construction of this improvement would result in LOS E operations during both the a.m. and p.m. peak hour. Installation of these improvements should occur when the intersection V/C ratio exceeds 1.00 or the peak hour volume for either movement exceeds approximately 900 vehicles.

Improvement CP-10: Modify the northbound and southbound approaches to the Elder Creek Road/Excelsior Road intersection to include a second through lane. Since the projected daily volume is at the capacity of a typical two-lane roadway and the peak hour demand is over capacity, Excelsior Road should be widened to four lanes between Elder Creek Road and Jackson Road. The southbound departure leg of the Elder Creek Road/Excelsior Road intersection would have to be widened to accommodate two lanes to the Florin Road intersection (see Improvement CP-13).

This intersection modification would improve the cumulative plus project LOS from F to C in the a.m. peak hour and to B in the p.m. peak hour. These improvements should be implemented when intersection operation exceeds LOS E.

Improvement CP-11: None of the left or right turn volumes at the Elk Grove-Florin Road/Florin Road intersection warrant non-standard traffic controls (e.g., triple left turn lanes, dual right turn lanes, "free" right turn lanes). Construction of a fourth through lane on the northbound and southbound approaches would improve the LOS to E or better; however, Sacramento County does not typically construct eight-lane roadway segments except under special circumstances (e.g., adjacent to a freeway interchange). Thus, the proposed widening of Elk Grove-Florin Road is not considered feasible and no improvements are recommended.

Improvement CP-12: Construct dual right turn lanes on the eastbound approach of the Bradshaw Road/Florin Road intersection. Although this improvement does not improve the LOS to an acceptable level, the a.m. peak hour V/C ratio (1.15) would be less than the cumulative no project ratio (1.17). This improvement would not change the p.m. peak hour V/C ratio.

Construction of a fourth through lane on the northbound and southbound approaches would improve the LOS to E or better; however, Sacramento County does not typically construct eight-lane roadway segments except under special circumstances (e.g., adjacent to a freeway interchange). Thus, the proposed widening of Elk Grove-Florin Road is not considered feasible and no improvements are recommended.

Improvement CP-13: Modify the eastbound approach to include dual left turn lanes and add a second northbound through lane at the Florin Road/Excelsior Road intersection. These improvements would require widening of Excelsior Road between Florin Road and Elder Creek Road to accommodate two departure lanes. Although the daily volume on the Excelsior Road north of Florin Road does not warrant two-northbound through lanes, these improvements are necessary to provide acceptable traffic operations based on peak hour forecasts. Similarly, two southbound lanes on Excelsior Road should be constructed between Elder Creek and Florin Road (see Improvement CP-10).

This intersection modification would improve the cumulative plus project LOS from F to C in the a.m. and p.m. peak hour. These improvements should be implemented when intersection operation exceeds LOS E.

Improvement CP-14: None of the left or right turn volumes at the Elk Grove-Florin Road/Gerber Road intersection warrant non-standard traffic controls (e.g., triple left turn lanes, dual right turn lanes, "free" right turn lanes). Construction of a fourth through lane on the northbound and southbound approaches would improve the LOS to E or better; however, Sacramento County does not typically construct eight-lane roadway segments except under special circumstances (e.g., adjacent to a freeway interchange). Thus, the proposed widening of Elk Grove-Florin Road is not considered feasible and no improvements are recommended.

Improvement CP-15: None of the left or right turn volumes at the Elk Grove-Florin Road/Calvine Road intersection warrant non-standard traffic controls (e.g., triple left turn lanes, dual right turn lanes, "free" right turn lanes). Construction of a fourth through lane on either the northbound and southbound approaches or the eastbound and westbound approaches would improve the LOS to E or better; however, Sacramento County does not typically construct eight-lane roadway segments except under special circumstances (e.g., adjacent to a freeway interchange). Thus, the proposed widening of Elk Grove-Florin Road is not considered feasible and no improvements are recommended.

OPERATIONS WITH PROPOSED IMPROVEMENTS

Following implementation of the proposed improvements, three of the twelve transportation deficiencies identified would be eliminated. However, deficiencies CP-1, CP-5, CP-7, CP-8, CP-9, CP-11, CP-12, CP-14, and CP-15 would remain under cumulative plus project conditions. Table 9 shows the LOS at the study intersections after implementation of the proposed improvements.

As shown in Table 9, the proposed improvements result in acceptable operating conditions (LOS E or better) at thirteen of the study intersections. Eight intersections are projected to operate at LOS F with the project in place under cumulative conditions. However, at the South Watt Avenue/Jackson Road intersection and the Jackson Road/Excelsior Road intersection, the proposed improvements result in operations that are better than cumulative no project conditions.

Table 9
Cumulative Plus Project Intersection Levels of Service with Improvements

Intersection	Traffic Control	AM Peak Hour		PM Peak Hour	
		V/C Ratio ¹	LOS ²	V/C Ratio	LOS
S. Watt Avenue/Jackson Road	Signal	1.10	F	1.13	F
Bradshaw Road/Jackson Road	Signal	0.95	E	0.92	E
Jackson Road/Excelsior Road	Signal	0.95	E	1.04 <i>01</i>	F
S. Watt Avenue/Elder Creek Road	Signal	1.26	F	1.18	F
Bradshaw Road/Elder Creek Road	Signal	0.92	E	1.04 <i>02</i>	F
Excelsior Road/Elder Creek Road	Signal	0.72	C	0.69	B
Elk Grove-Florin Road/Florin Road	Signal	1.40	F	1.25	F
Florin Road/Waterman Road	Signal	0.67	B	0.99	E
Bradshaw Road/Florin Road	Signal	1.15	F	1.16	F
Florin Road/Vineyard Road	Signal	0.59	A	0.69	B
Florin Road/Excelsior Road	Signal	0.76	C	0.80	C
Elk Grove-Florin Road/Gerber Road	Signal	1.47	F	1.29	F
Waterman Road/Gerber Road	Signal	0.70	B	0.87	D
Bradshaw Road/Gerber Road	Signal	0.80	C	0.82	D
Vineyard Road/Gerber Road	Signal	0.69	B	0.41	A
Excelsior Road/Gerber Road	Signal	0.67	B	0.48	A
Elk Grove-Florin Road/Calvine Road	Signal	0.91	E	1.02	F
Calvine Road/Waterman Road	Signal	0.81	D	0.85	D
Calvine Road/Bradshaw Road	Signal	0.77	C	0.85	D
Calvine Road/Vineyard Road	Signal	0.58	A	0.65	B
Calvine Road/Excelsior Road	Signal	0.48	A	0.44	A

¹ V/C Ratio = Volume-to-Capacity Ratio.
² LOS = Level of Service. Shaded areas represent LOS F conditions.

V. RELATED TRANSPORTATION ISSUES

As part of the North Vineyard Station Specific Plan transportation analysis, several related issues were evaluated. These issues included the following:

- Potential effect of the project on the Congestion Management Plan roadway network;
- Circulation within the North Vineyard Station Specific Plan area;
- Trip reduction measures to meet regional air quality goals and to reduce congestion; and
- Transportation impacts of alternatives to the proposed project.

These issues are discussed below.

CONGESTION MANAGEMENT PROGRAM (CMP) CONFORMITY

The Sacramento County CMP was designed to “create an integrated approach to programming transportation improvements,” as stated in the December 1993 document prepared by the Sacramento Transportation Authority (STA). Based on this premise, studies for new developments must be prepared in a manner consistent with operating standards and plans for regional and local transportation facilities. To this end, the CMP has identified a network of roadways and transit facilities that must maintain a minimum operating standard given an accepted level of congestion or the adoption of a deficiency plan to improve operations.

Within the study area, two roadways are included in the CMP network both of which are designated LOS D standard facilities. These segments include the following:

- Jackson Road/State Route (SR) 16 between South Watt Avenue and Excelsior Road, which is classified as an urban highway; and
- South Watt Avenue between SR 16 and Florin Road, which is classified as an urban arterial.

Under mitigated existing plus project conditions, several intersections and segments of these facilities are still projected to operate worse than LOS D. Although additional mitigation is feasible to achieve the minimum operating standard under existing conditions, CMP documentation states that mitigation measures or deficiency plans “...need only be prepared after a non-conforming LOS has been determined by the STA board to exist on a segment.”

It should also be noted that existing plus project conditions are analyzed for purposes of California Environmental Quality Act compliance, and that this scenario is not likely, since the project will take several years to build out. Accordingly, on-going monitoring should be employed to determine conformity with LOS standards and all new developments should include measures to reduce trips and air emissions. Trip and emission reduction measures for the NVSSP are presented below in a subsequent section.

The cumulative analysis presented in Chapter IV also shows that several intersections and segments of CMP facilities are projected to operate worse than LOS D. However, these facilities would operate below standards with or without the proposed project based on the current plans for transportation infrastructure. Thus, revision of LOS standards, additional area-wide roadway improvements, or changes to land use may be required to achieve conformity. Since the traffic model used in this analysis estimates cumulative forecasts over approximately 20 years and deficiency plans are developed to address current deficiencies, on-going monitoring should be employed to determine conformity and master planning of transportation system elements in the study area should be periodically updated.

INTERNAL CIRCULATION SYSTEM

The proposed circulation system includes a variety of transportation facilities designed to encourage a balanced use of travel modes serving automobile, bicycle, pedestrian, and transit trips. The layout of these facilities is designed to evenly distribute trips to the arterials along the boundaries of the plan area and to limit the need for major roadways within the project.

Figure 16 shows the projected daily traffic volumes on the internal roadway network under cumulative conditions. Based on these volumes and Sacramento County guidelines, the only four-lane facility within the plan area will be the section of Waterman Road between Florin Road and Gerber Road, which is expected to serve 31,900 vehicles per day (VPD). All other roadways within the site (except for Bradshaw Road) are projected to serve fewer than 5,500 VPD, which can be accommodated on a two-lane roadway. For those streets serving residential areas with projected volumes between 4,500 and 5,500 VPD, special landscape and access treatments may be required, and in some cases, front-on housing may be precluded. Vineyard Road, which forms the eastern boundary of the site, is planned to be a four-lane facility and would accommodate a projected daily volume of 14,000 VPD. Bradshaw Road is planned to be a six-lane facility and is projected to serve 46,600 VPD.

Figure 16 also shows the location of signalized intersections that are projected to meet signal warrants under cumulative conditions. Signalized intersections will typically be spaced at 1,200-foot minimum intervals and traffic signals should not be implemented until warranted by standard volume warrants. Except for five locations, all of the collector streets are projected to be signalized. Since tentative maps are not available, the actual loading of traffic to collector streets may vary slightly and may require a traffic signal at one intersection on Gerber Road between Bradshaw Road and Vineyard Road.

With the exception of intersections on Waterman Road and Bradshaw Road, intersection traffic control on internal project streets is expected to be maintained through the use of stop signs. At some locations, especially close to schools and parks, signals may be installed to serve high pedestrian and bicycle traffic volumes and to enhance safety.

The proposed Waterman Road crossing of the Central California Traction Railroad will likely include crossing gates, signals, and striping to stop traffic as trains proceed through the plan area. Under existing conditions, an average of two trains travel along this railway segment each day, and train lengths vary between two and 50 cars.

With the addition of project and background traffic at the proposed crossing, vehicles may be delayed and queues may form on the northbound and southbound approach. The circulation plan shows that the nearest collector street intersections on Waterman Road are located approximately 800 feet north and south of the proposed crossing. Once more detailed tentative maps are available, traffic circulation in the immediate vicinity of the crossing should be reviewed to ensure that temporary queues on Waterman Road will not result in adverse impacts to driveways or minor streets.

The proposed network of on-street bicycle lanes and off-street bicycle/pedestrian paths is designed to interconnect residential neighborhoods, parks, schools, and commercial areas. This system will also provide connections to planned on-street bicycle lanes on all five major roadways serving the site and will be constructed to applicable County and state standards.

The NVSSP area will be designed to accommodate transit facilities and services planned by Sacramento Regional Transit (RT). Primary transit corridors within the plan area include Bradshaw Road and Florin Road according to RT's 20-year master plan. It is anticipated that bus stops will be designated at one-quarter mile intervals along these roadways. Additional transit routes and stops will be identified by RT on other major arterial and collector streets as the area develops and service is extended to the North Vineyard Station, Florin Station Comprehensive, and Vineyard Springs Comprehensive Planning Areas.

VEHICLE TRIP AND EMISSION REDUCTION MEASURES

The Sacramento County General Plan contains policies that are designed to improve air quality through a variety of measures that eliminate or reduce vehicle trips, reduce trip length, or change the time of day when trips are completed. Policy AQ-15 is presented as follows:

All new major indirect sources of emissions shall be reviewed and modified or conditions to achieve a reduction in emissions. This indirect source review program will be developed in coordination with SACOG and Sacramento Metropolitan Air Quality Management District (SMAQMD), and include the following features:

- (a) A 15% reduction in emissions from the level that would be produced by a base-case project assuming full trip generation per the current Institute of Transportation Engineers (ITE) *Trip Generation Manual*.
- (b) A focus on cost-effectiveness measured in terms of cost per ton of pollutant avoided.
- (c) A list of cost-effective measures to be developed, maintained, and annually reviewed by SMAQMD.
- (d) A maximum expenditure cap which will be computed for each indirect source on the basis of factors including, but not limited to, total emissions and project value.
- (e) A process for obtaining a waiver from the 15 percent requirements if it is found that a lower level of reduction is all that can be achieved with cost-effective measures and offsets, or that achieving the full 15 percent reduction would cost more than the expenditure cap.

- (f) An exception for projects which have already undergone indirect source review at some point in the development approval process.
- (g) A procedure to give full credit for other measures required in a project that may also achieve a reduction in emissions.

Sacramento County has developed a preliminary list of measures and corresponding credits that can be applied to the required 15% reduction in emissions. This list is based on data originally prepared by the SMAQMD in a February 1995 report entitled *Indirect Source Review Program: Implementation Guidelines*. Sacramento County is currently developing specific requirements that will be incorporated into the Zoning Code. The following measures, which are incorporated into the NVSSP, are all contained in the preliminary list of acceptable measures:

1. The plan contains a mixture of complementary land uses (residential, commercial, parks, schools) located within the project or within one-half mile of the project boundaries. Approximately half of the plan meets the County's criteria, which allows for up to a 6% credit. NVSSP CREDIT=3%.
2. All parking areas (residential and non-residential) will include electric vehicle charging facilities. Single-family homes will include an outlet for vehicle charging in the garage, multi-family units will include charging facilities in a common area, and non-residential development will provide one electric charging facility for every 50 regular parking spaces. NVSSP CREDIT = 1%
3. The plan is designed to provide a transit stop within a reasonable distance of all land uses. Although specific locations for stops have not been identified at RT staff's recommendation, the proposed roadway network of major collectors would accommodate bus stops within one-quarter mile of all land uses. NVSSP CREDIT=2%.
4. The plan will include easements to accommodate bus stop improvements (route signs, benches, shelters and lighting) at all major transit stops. Current RT policy only requires easements for stops since a private firm provides the shelter and related improvements in exchange for advertising space. NVSSP CREDIT = 2%.
5. The plan is designed to accommodate and provide access to the planned on-street (Class II) bicycle facilities as identified in the 2010 Sacramento City/County Bikeway Master Plan (BMP). On-street facilities within one-half mile of the project site are planned on Florin, Bradshaw, and Gerber Roads and the entire plan area meets the criteria. NVSSP CREDIT = 2.0%
6. In addition to the bikeways included in the BMP, both on-street and off-street facilities are included throughout the plan area and will be located within one-half mile of all major land uses. NVSSP CREDIT = 1.5%

7. The proposed circulation system provides for direct (i.e., - minimum distance) pedestrian connections between adjacent and complementary land uses . All parks, schools, and commercial areas are connected to residential areas by interconnected roads and pathways. NVSSP CREDIT = 2%
8. The circulation system in the plan provides direct automobile access between complementary land uses to minimize the distance traveled, within the limits of physical constraints (i.e., drainage parkways). NVSSP CREDIT = 1%
9. The plan area will operate or participate in a Transportation Management Association to create, administer, and finance on-going programs to reduce vehicle trips. The Financing Plan for the project will include means to fund the TMA. NVSSP = 3%

According to the County's preliminary guidelines, the above measures result in a 16.5 percent reduction in emissions and meet the requirements of General Plan Policy AQ-15.

In addition to the above measures, the Specific Plan will include policies that encourage the use of low-emission heating/cooling measures and equipment, and the provision of child care services within the plan area. Table 10 summarizes the measures, credits, and reference information for the trip and emission reduction measures.

Measure	Specific Plan Reference	Maximum ¹ Credit Allowance	Credit Taken
1. Complementary land uses	Land Use Exhibit	6%	3%
2. Electrical vehicle charging facilities	MacKay & Somps	1%	1%
3. Convenient transit stops	Circulation Plan	2%	2%
4. Physical amenities at major transit stops	Circulation Plan and Regional Transit	2%	2%
5. Bicycle connections to City/County Bikeway Master Plan facilities	Circulation Plan	2%	2%
6. Bicycle lanes/paths within 1/2 mile of all major land uses	Circulation Plan	1.5%	1.5%
7. Multiple/direct pedestrian access between complementary land uses	Circulation Plan	2%	1%
8. Multiple/direct automobile access between complementary land uses	Circulation Plan	1%	1%
9. Participation in Transportation Management Association	MacKay & Somps	3%	3%
TOTAL EMISSION REDUCTION:			16.5%

TRANSPORTATION IMPACTS OF PROJECT ALTERNATIVES

Two alternatives to the preferred land use plan were evaluated as part of the transportation analysis. These alternatives include:

- **10 Percent Low Density Alternative** - This alternative includes approximately 10 percent fewer dwelling units than the preferred plan. Under this alternative, medium-density housing (7 to 14 units per acre) is not proposed and low density residential uses are expanded. The net result is 5,759 dwelling units with minor changes to the total acreage for parks. All other uses (i.e., parks, schools, commercial areas, open space, etc.) are unchanged.
- **15 Percent Low Density Alternative** - This alternative includes approximately 15 percent fewer dwelling units than the preferred plan. Similar to the previous alternative, medium-density housing is not included and low density residential uses are expanded. The net result is 5,384 dwelling units with minor changes to total acreage for parks. All other uses are unchanged.

Trip generation for each of the alternatives was estimated using methods described in Chapter III. Tables F-1 and F-2 in Appendix F summarize the estimated daily trip generation for the 10 Percent and 15 Percent Low Density Alternatives, respectively.

These tables show that the 10 Percent Alternative would result in a total of 78,552 total daily vehicle trips or approximately seven percent fewer trips than estimated for the Preferred Plan. The number of net external daily vehicle trips under this alternative is 58,271, or seven percent fewer trips than the Preferred Plan.

Similarly, the 15 Percent Alternative would result in 74,971 total daily vehicle trips and 55,192 net external vehicle trips. These trip generation estimates are approximately 11 percent and 12 percent fewer than the total daily and net external trip estimates for the Preferred Plan, respectively.

Based on lower trip generation, impacts to the transportation system are expected to be less than those caused by the Preferred Plan; however, the alternative land use plans would still result in operational deficiencies at the critical study locations.

APPENDIX A

Existing Level of Service Calculations

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND S. WATT
Problem Statement: EXIST

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: S. WATT		-----Approach-----				Possible Volume Adjusted			
1 1 ^		: -1- -2- -3- -4-				Critical Volume Critical			
R L N		a.No. of change : 0 0 0 0				Prob- able Phase			
R T T T L		intervals/hour : :				Critical Volume in vph			
T H H H T		b.LT capacity on : 0 0 0 0				Volume to next phase			
-----		change (vph) : :				-----			
Approach 1		c.G/C ratio : 0 0 0 0				B2B1 55(B2) 99- 55= 44(B1) 55			
1 LT--^		d.Opposing volume : 0 0 0 0				A2B1 44(B1) 453- 44= 409(A2) 44			
LTH-^>		in vph : :				A1A2 409(A2) OR 146(A1) 409			
TH-->		e.LT capacity on : 0 0 0 0				B4B3 24(B3) 139- 24= 115(B4) 24			
1 RTH-v>		green (vph) : :				A3B4 115(B4) 896- 115= 781(A3) 115			
RT--v		f.LT capacity in : 0 0 0 0				A3A4 781(A3) OR 679(A4) 781			
<< >> Approach 2		vph (b+e) : :							
L L T R R		g.Left turn volume : 0 0 0 0							
T T H T T JACKSON HWY		in vph : :							
H H		h.Is volume > cap. : :							
1 1		(g>f) ? : :							
Approach 4: S. WATT									

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		1 7 1		99(B2B1)+409(A2)+139(B3B4)+781(A3)	
3: LT= 139		8 0 3		= 1428 vph	
TH= 707		9 7 9		-----	
RT= 189 v		+ +		Step 8. INTERSECTION LEVEL OF SERVICE	
		< v >		(compare step 7 with table 6)	
<--Approach 2				-----	
Approach 1-->		55 ^		-----	
1:LT= 55 ^		139 +>		Step 9. RECALCULATE	
TH= 139		7 +v		Geometric Change:	
RT= 7				Signal Change:	
Approach 4				Volume Change:	
				6	
				2 3 4	
				4 5 4	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS	
--^ v-- B2B1		Approach 3			
--^ AND <-- A1B2 AND					
--> OR v-- /OR A2B1					
--> <-- A1A2		Approach 1			
< B4B3		See Step 6b.			
>		Approach 2			
AND < ^ A3B4 AND					
v > OR /OR A4B3					
^ A3A4					
v					
-----		Approach 4		Exclusive right turns reduced 30 %	
A1 --> A3 B1 v-- B3 <				V/C Ratio = 1.04	
v ^					
A2 <-- A4 B2 --^ B4 >					

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND S. WATT
Problem Statement: EXIST

Design Hour: PM

<p>Step 1. IDENTIFY LANE GEOMETRY</p> <p>Approach 3: S. WATT</p> <pre> 1 1 ^ R L N R T T T L T H H H T < < > > v v v <^-RT 1 <^-RTH <--TH 1 <v-LTH v--LT 1 < < > > Approach 2 L L T R R T T H T T JACKSON HWY H H 1 1 Approach 4: S. WATT </pre>	<p>Step 4. LEFT TURN CHECK</p> <p>-----Approach-----</p> <table style="width:100%; border-collapse: collapse;"> <tr> <td style="width:10%;"></td> <td style="width:10%;"></td> <td style="width:10%; text-align: center;">-1-</td> <td style="width:10%; text-align: center;">-2-</td> <td style="width:10%; text-align: center;">-3-</td> <td style="width:10%; text-align: center;">-4-</td> </tr> <tr> <td>a.No. of change intervals/hour</td> <td>:</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> </tr> <tr> <td>b.LT capacity on change (vph)</td> <td>:</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> </tr> <tr> <td>c.G/C ratio</td> <td>:</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> </tr> <tr> <td>d.Opposing volume in vph</td> <td>:</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> </tr> <tr> <td>e.LT capacity on green (vph)</td> <td>:</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> </tr> <tr> <td>f.LT capacity in vph (b+e)</td> <td>:</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> </tr> <tr> <td>g.Left turn volume in vph</td> <td>:</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> </tr> <tr> <td>h.Is volume > cap. (g>f) ?</td> <td>:</td> <td></td> <td></td> <td></td> <td></td> </tr> </table>			-1-	-2-	-3-	-4-	a.No. of change intervals/hour	:	0	0	0	0	b.LT capacity on change (vph)	:	0	0	0	0	c.G/C ratio	:	0	0	0	0	d.Opposing volume in vph	:	0	0	0	0	e.LT capacity on green (vph)	:	0	0	0	0	f.LT capacity in vph (b+e)	:	0	0	0	0	g.Left turn volume in vph	:	0	0	0	0	h.Is volume > cap. (g>f) ?	:					<p>Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP</p> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th style="width:10%;"></th> <th style="width:10%;"></th> <th style="width:10%;"></th> <th style="width:10%; text-align: center;">Possible</th> <th style="width:10%; text-align: center;">Volume</th> <th style="width:10%; text-align: center;">Adjusted</th> </tr> <tr> <td></td> <td></td> <td></td> <td style="text-align: center;">Critical</td> <td style="text-align: center;">Carryover</td> <td style="text-align: center;">Critical</td> </tr> <tr> <td></td> <td></td> <td></td> <td style="text-align: center;">Volume</td> <td style="text-align: center;">to next</td> <td style="text-align: center;">Volume</td> </tr> <tr> <td></td> <td></td> <td></td> <td style="text-align: center;">in vph</td> <td style="text-align: center;">phase</td> <td style="text-align: center;">in vph</td> </tr> </thead> <tbody> <tr> <td>B2B1</td> <td>76(B1)</td> <td>13- 76=</td> <td>37(B2)</td> <td>76</td> <td></td> </tr> <tr> <td>A1B2</td> <td>37(B2)</td> <td>02- 37=</td> <td>465(A1)</td> <td>37</td> <td></td> </tr> <tr> <td>A1A2</td> <td>465(A1)</td> <td>OR</td> <td>215(A2)</td> <td>465</td> <td></td> </tr> <tr> <td>B4B3</td> <td>12(B3)</td> <td>12- 12=</td> <td>200(B4)</td> <td>12</td> <td></td> </tr> <tr> <td>A3B4</td> <td>200(B4)</td> <td>13- 200=</td> <td>613(A3)</td> <td>200</td> <td></td> </tr> <tr> <td>A3A4</td> <td>749(A4)</td> <td>OR</td> <td>613(A3)</td> <td>749</td> <td></td> </tr> </tbody> </table>				Possible	Volume	Adjusted				Critical	Carryover	Critical				Volume	to next	Volume				in vph	phase	in vph	B2B1	76(B1)	13- 76=	37(B2)	76		A1B2	37(B2)	02- 37=	465(A1)	37		A1A2	465(A1)	OR	215(A2)	465		B4B3	12(B3)	12- 12=	200(B4)	12		A3B4	200(B4)	13- 200=	613(A3)	200		A3A4	749(A4)	OR	613(A3)	749	
		-1-	-2-	-3-	-4-																																																																																																															
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<p>Step 2. IDENTIFY VOLUMES, in vph</p> <pre> Approach 3 3: LT= 212 2:RT= 141 TH= 731 TH= 215 RT= 82 v LT= 76 <--Approach 2 Approach 1--> 1:LT= 113 ^ 4: RT= 66 TH= 476 TH= 683 RT= 26 LT= 12 Approach 4 </pre>	<p>Step 5. ASSIGN LANE VOLUMES, in vph</p> <pre> 7 2 8 3 1 2 1 2 + + < v > 113 ^ 476 +> 26 +v 6 1 8 6 2 3 6 </pre>	<p>Step 7. SUM OF CRITICAL VOLUMES</p> <p>113(B1B2)+465(A1)+212(B3B4)+749(A4)</p> <p>= 1539 vph</p> <p>Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)</p> <p style="text-align: center;">F</p> <p>Step 9. RECALCULATE</p> <p>Geometric Change: Signal Change: Volume Change:</p>																																																																																																																		
<p>Step 3. IDENTIFY PHASING</p> <pre> --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 > </pre>	<p>Step 6a. CRITICAL VOLUMES, in vph (two phase signal)</p> <p>Approach 3</p> <p>Approach 1</p> <p style="text-align: center;">See Step 6b.</p> <p>Approach 2</p> <p>Approach 4</p>	<p>COMMENTS</p> <p>Exclusive right turns reduced 30 % V/C Ratio = 1.12</p>																																																																																																																		

APPENDIX A

Existing Level of Service Calculations

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND S. WATT
Problem Statement: EXIST

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: S. WATT 1 1 ^ R L N JACKSON HWY R T T T L T H H H T Approach 1 << >> ^-RT 1 1 LT--^ v v v <^-RTH LTH-^> <--TH 1 TH--> <v-LTH 1 RTH-v> ^ ^ ^ v--LT 1 RT--v << >> Approach 2 L L T R R T T H T T JACKSON HWY H H 1 1 Approach 4: S. WATT		-----Approach----- : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 0 0 d.Opposing volume in vph : 0 0 0 0 e.LT capacity on green (vph) : 0 0 0 0 f.LT capacity in vph (b+e) : 0 0 0 0 g.Left turn volume in vph : h.Is volume > cap. (g>f) ? :				Possible Volume Adjusted Critical Volume to next phase Prob- able Phase B2B1 55(B2) 99- 55= 44(B1) 55 A2B1 44(B1) 453- 44= 409(A2) 44 A1A2 409(A2) OR 146(A1) 409 B4B3 24(B3) 139- 24= 115(B4) 24 A3B4 115(B4) 896- 115= 781(A3) 115 A3A4 781(A3) OR 679(A4) 781			
Step 2. IDENTIFY VOLUMES, in vph Approach 3 3: LT= 139 2:RT= 323 TH= 707 TH= 453 RT= 189 v LT= 99 <--Approach 2 Approach 1--> 1:LT= 55 ^ 4: RT= 44 TH= 139 TH= 635 RT= 7 LT= 24 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 1 7 1 ----- 8 0 3 9 7 9 ^- 323 + + <- 453 < v > v- 99				Step 7. SUM OF CRITICAL VOLUMES 99(B2B1)+409(A2)+139(B3B4)+781(A3) = 1428 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.04			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND BRADSHAW
Problem Statement: EXIST

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW 1 1 1 ^ R L N JACKSON HWY R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 0 0 d.Opposing volume in vph : 0 0 0 0 e.LT capacity on green (vph) : 0 0 0 0 f.LT capacity in vph (b+e) : 0 0 0 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? :				Possible Volume Adjusted Prob- Critical Carryover Critical able Volume to next Volume Phase in vph phase in vph B2B1 15(B1) 200- 15= 185(B2) 15 A1B2 185(B2) 180- 185= 0(A1) 185 A1A2 423(A2) OR 0(A1) 423 B4B3 53(B3) 74- 53= 21(B4) 53 A3B4 21(B4) 420- 21= 399(A3) 21 A3A4 862(A4) OR 399(A3) 862			
Approach 1 << >> ^--RT 1 1 LT--^ v v v <^--RTH LTH-^> <--TH 1 1 TH--> <v--LTH RTH-v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T JACKSON HWY K H 1 1 1 Approach 4:BRADSHAW									
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
Approach 3 3: LT= 74 2:RT= 187 TH= 420 TH= 423 RT= 226 v LT= 15 <--Approach 2 Approach 1--> 1:LT= 200 ^ 4: RT= 14 TH= 180 TH= 862 RT= 32 LT= 53 Approach 4		2 4 2 2 7 6 0 4 < v > 200 ^ < ^ > 180 -> 32 -v 8 5 6 1 3 2 4				200(B1B2)+423(A2)+74(B3B4)+862(A4) = 1559 vph			
						Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)			
						F			
						Step 9. RECALCULATE			
						Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				Exclusive right turns reduced 30 % V/C Ratio = 1.13			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND BRADSHAW
Problem Statement: EXIST

Design Hour: PM

<p>Step 1. IDENTIFY LANE GEOMETRY</p> <p>Approach 3:BRADSHAW 1 1 1 ^ R L N JACKSON HWY R T T T L T H H H T ----- Approach 1 << >> ^-RT 1 1 LT--^ v v v <^-RTH LTH-^> <--TH 1 1 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T JACKSON HWY H H 1 1 1 Approach 4:BRADSHAW</p>	<p>Step 4. LEFT TURN CHECK</p> <p align="center">-----Approach----- : -1- -2- -3- -4-</p> <p>a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume in vph : 0 0 0 0 e.LT capacity on green (vph) : 0 0 0 0 f.LT capacity in vph (b+e) : g.Left turn volume in vph : h.Is volume > cap. (g>f) ? :</p>	<p>Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th></th> <th>Possible Critical Volume in vph</th> <th>Volume to next phase</th> <th>Adjusted Critical Volume in vph</th> </tr> </thead> <tbody> <tr> <td>B2B1</td> <td>28(B1)</td> <td>191- 28= 163(B2)</td> <td>28</td> </tr> <tr> <td>A1B2</td> <td>163(B2)</td> <td>433- 163= 270(A1)</td> <td>163</td> </tr> <tr> <td>A1A2</td> <td>270(A1) OR 156(A2)</td> <td></td> <td>270</td> </tr> <tr> <td>B4B3</td> <td>37(B3)</td> <td>165- 37= 128(B4)</td> <td>37</td> </tr> <tr> <td>A3B4</td> <td>128(B4)</td> <td>861- 128= 733(A3)</td> <td>128</td> </tr> <tr> <td>A3A4</td> <td>733(A3) OR 510(A4)</td> <td></td> <td>733</td> </tr> </tbody> </table>		Possible Critical Volume in vph	Volume to next phase	Adjusted Critical Volume in vph	B2B1	28(B1)	191- 28= 163(B2)	28	A1B2	163(B2)	433- 163= 270(A1)	163	A1A2	270(A1) OR 156(A2)		270	B4B3	37(B3)	165- 37= 128(B4)	37	A3B4	128(B4)	861- 128= 733(A3)	128	A3A4	733(A3) OR 510(A4)		733
	Possible Critical Volume in vph	Volume to next phase	Adjusted Critical Volume in vph																											
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A1A2	270(A1) OR 156(A2)		270																											
B4B3	37(B3)	165- 37= 128(B4)	37																											
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A3A4	733(A3) OR 510(A4)		733																											
<p>Step 2. IDENTIFY VOLUMES, in vph</p> <p>3: LT= 165 Approach 3 2:RT= 76 TH= 861 TH= 156 RT= 182 v LT= 28 ----- <--Approach 2 Approach 1--> ----- 1:LT= 191 ^ 4: RT= 20 TH= 433 TH= 510 RT= 40 LT= 37 Approach 4 </p>	<p>Step 5. ASSIGN LANE VOLUMES, in vph</p> <table border="1" style="width:100%; border-collapse: collapse;"> <tr><td>1 8 1</td><td></td></tr> <tr><td>8 6 6</td><td></td></tr> <tr><td>2 1 5</td><td>^- 76</td></tr> <tr><td> </td><td><- 156</td></tr> <tr><td>< v ></td><td>v- 28</td></tr> </table>	1 8 1		8 6 6		2 1 5	^- 76		<- 156	< v >	v- 28	<p>Step 7. SUM OF CRITICAL VOLUMES</p> <p>191(B1B2)+270(A1)+165(B3B4)+733(A3) = 1359 vph</p> <p>Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) E </p>																		
1 8 1																														
8 6 6																														
2 1 5	^- 76																													
	<- 156																													
< v >	v- 28																													
<p>Step 3. IDENTIFY PHASING</p> <p>--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v </p> <p>A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 > </p>	<p>Step 6a. CRITICAL VOLUMES, in vph (two phase signal)</p> <table border="1" style="width:100%; border-collapse: collapse;"> <tr><td>Approach 3</td></tr> <tr><td>Approach 1</td></tr> <tr><td>See Step 6b.</td></tr> <tr><td>Approach 2</td></tr> <tr><td>Approach 4</td></tr> </table>	Approach 3	Approach 1	See Step 6b.	Approach 2	Approach 4	<p>COMMENTS</p> <p>Exclusive right turns reduced 30 % V/C Ratio = .99</p>																							
Approach 3																														
Approach 1																														
See Step 6b.																														
Approach 2																														
Approach 4																														

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: BRADSHAW RD AND ELDER CREEK RD
Problem Statement: EXIST

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW 1 1 1 ^ R L N ELDER CREEK R R T T T L T H H H T Approach 1 < < > > ^-RT 1 1 LT--^ v v v <^-RTH LTH-^> <--TH TH--> <v-LTH 1 1 RTH-v> ^ ^ ^ v--LT RT--v < < > > Approach 2 L L T R R T T H T T ELDER CREEK R H H 1 1 Approach 4:BRADSHAW		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Volume Critical Volume to next Volume in vph phase in vph ----- A1B2 45(B2) OR 44(A1) 45 A2B1 136(A2) OR 1(B1) 136 B4B3 6(B4) 54- 6= 48(B3) 6 A4B3 48(B3) 870- 48= 822(A4) 48 A3A4 822(A4) OR 365(A3) 822			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 6 Approach 3 2:RT= 8 TH= 365 TH= 135 RT= 136 v LT= 2 <--Approach 2 Approach 1--> 1:LT= 45 ^ 4: RT= 2 TH= 36 TH= 868 RT= 8 LT= 54 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 1 3 ----- 3 6 6 5 6 ^- 8 <+ 135 < v > v+ 1				Step 7. SUM OF CRITICAL VOLUMES 45(B2)+136(A2)+54(B4B3)+822(A4) = 1057 vph			
		Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- c -----							
		Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:							
Step 3. IDENTIFY PHASING --^ A1B2 --> <-- A2B1 v-- < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .77			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: BRADSHAW RD AND ELDER CREEK RD

Design Hour: PM

Problem Statement: EXIST

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW 1 1 1 ^ R L N ELDER CREEK R R T T T L T H H H T Approach 1 << >> ^-RT 1 1 LT--^ v v v <^-RTH LTH-^> <--TH TH--> <v-LTH 1 1 RTH-v> ^ ^ ^ v--LT RT--v << >> Approach 2 L L T R R T T H T T ELDER CREEK R H H 1 1 Approach 4: BRADSHAW		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph A1B2 130(B2) OR 120(A1) 130 A2B1 46(A2) OR 6(B1) 46 B4B3 10(B3) 14- 10= 4(B4) 10 A3B4 4(B4) 860- 4= 856(A3) 4 A3A4 856(A3) OR 401(A4) 856			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3 3: LT= 14 2:RT= 7 TH= 860 TH= 40 RT= 68 v LT= 7 <--Approach 2		8 6 6 1 8 0 4 < v >		130(B2)+46(A2)+14(B3B4)+856(A3) = 1046 vph	
Approach 1--> 1:LT= 130 ^ 4: RT= 7 TH= 97 TH= 394 RT= 23 LT= 10 Approach 4		130 -^ < ^ > 97 +> } + + 23 +v 3 1 9 0 4 7		Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) C Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ A1B2 --> <-- A2B1 v-- < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				Exclusive right turns reduced 30 % V/C Ratio = .76

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: BRADSHAW RD AND FLORIN RD
Problem Statement: EXIST

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD 1 1 1 ^ R L M R T T T L T H H H T Approach 1 << >> ^-RT 1 LT--^ v v v <^-RTH 1 LTH-^> <--TH TH--> <v-LTH 1 RTH-v> ^ ^ ^ v--LT 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 1 1 Approach 4: BRADSHAW		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 16(B1) 40- 16= 24(B2) 16 A1B2 24(B2) 124- 24= 100(A1) 24 A1A2 175(A2) OR 100(A1) 175 B4B3 3(B4) 56- 3= 53(B3) 3 A4B3 53(B3) 1000- 53= 947(A4) 53 A3A4 947(A4) OR 367(A3) 947			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 3 Approach 3 2:RT= 3 TH= 367 TH= 172 RT= 82 v LT= 16 <--Approach 2 Approach 1--> 1:LT= 40 ^ 4: RT= 46 TH= 78 TH= 954 RT= 46 LT= 56 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 3 8 6 2 7 3 < v > 40 -^ < ^ > 78 +> + + 46 +v 9 5 5 4 6 4 6				Step 7. SUM OF CRITICAL VOLUMES 40(B1B2)+175(A2)+56(B4B3)+947(A4) = 1218 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) D Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 81 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .89			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND BRADSHAW
Problem Statement: EXIST

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW 1 1 1 ^ R L N JACKSON HWY R T T T L T H H H T -----		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph ----- ----- ----- ----- B2B1 28(B1) 191- 28= 163(B2) 28 A1B2 163(B2) 433- 163= 270(A1) 163 A1A2 270(A1) OR 156(A2) 270 B4B3 37(B3) 165- 37= 128(B4) 37 A3B4 128(B4) 861- 128= 733(A3) 128 A3A4 733(A3) OR 510(A4) 733			
Approach 1 1 LT--^ v v v <^--RT 1 LTH--> <--TH 1 1 TH--> <v-LTH RTH--v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R ----- T T H T T JACKSON HWY H H 1 1 1 Approach 4:BRADSHAW									
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 165 Approach 3 TH= 861 2:RT= 76 RT= 182 v TH= 156 LT= 28 <--Approach 2 Approach 1--> 1:LT= 191 ^ 4: RT= 20 TH= 433 TH= 510 RT= 40 LT= 37 Approach 4		1 8 1 ----- ----- ----- ----- 8 6 6 2 1 5 ^- 76 <- 156 < v > v- 28				Step 7. SUM OF CRITICAL VOLUMES 191(B1B2)+270(A1)+165(B3B4)+733(A3) = 1359 vph ----- ----- ----- ----- Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) E ----- ----- ----- ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- ----- ----- ----- Approach 1 See Step 6b. ----- ----- ----- ----- Approach 2 ----- ----- ----- ----- Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .99			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: BRADSHAW RD AND ELDER CREEK RD
Problem Statement: EXIST

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW 1 1 1 ^ R L N ELDER CREEK R R T T T L T H H H T Approach 1 << >> ^-RT 1 1 LT--^ v v v <^-RTH LTH-^> <--TH TH--> <v-LTH 1 1 RTH-v> ^ ^ ^ v--LT RT--v << >> Approach 2 L L T R R T T H T T ELDER CREEK R H H 1 1 Approach 4:BRADSHAW		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph ----- A1B2 45(B2) OR 44(A1) 45 A2B1 136(A2) OR 1(B1) 136 B4B3 6(B4) 54- 6= 48(B3) 6 A4B3 48(B3) 870- 48= 822(A4) 48 A3A4 822(A4) OR 365(A3) 822			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 6 TH= 365 RT= 136 Approach 3 2:RT= 8 TH= 135 LT= 2 Approach 2 Approach 1-->		Step 5. ASSIGN LANE VOLUMES, in vph 1 3 3 6 6 5 6 < v > Approach 2 45 -^ 36 +> 8 +v Approach 4 4: RT= 2 TH= 868 LT= 54				Step 7. SUM OF CRITICAL VOLUMES 45(B2)+136(A2)+54(B4B3)+822(A4) = 1057 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- C ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ A1B2 --> <-- A2B1 v-- < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS ----- Exclusive right turns reduced 30 % V/C Ratio = .77			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: BRADSHAW RD AND FLORIN RD
Problem Statement: EXIST

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY

Approach 3: BRADSHAW RD	
1 1 1	^
R L	N
R T T T L	
T H H H T	
Approach 1 << >>	^-RT
1 LT--^	<^-RTH 1
LTH-^>	<--TH
TH-->	<v-LTH
1 RTH-v>	v--LT 1
RT--v	Approach 2
L L T R R	
T T H T T	FLORIN RD
H H	
1 1	
Approach 4: BRADSHAW	

Step 4. LEFT TURN CHECK

-----Approach-----				
	-1-	-2-	-3-	-4-
a.No. of change intervals/hour	: 0	0	0	0
b.LT capacity on change (vph)	: 0	0	0	0
c.G/C ratio	: 0	0	0	0
d.Opposing volume in vph	: 0	0	0	0
e.LT capacity on green (vph)	: 0	0	0	0
f.LT capacity in vph (b+e)	: 0	0	0	0
g.Left turn volume in vph	: 0	0	0	0
h.Is volume > cap. (g>f) ?	:	:	:	:

Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP

Prob-Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph
B2B1	31(B1)	101- 31= 70(B2)	31
A1B2	70(B2)	201- 70= 131(A1)	70
A1A2	133(A2) OR 131(A1)		133
B4B3	2(B4)	24- 2= 22(B3)	2
A4B3	22(B3)	332- 22= 310(A4)	22
A3A4	874(A3) OR 310(A4)		874

Step 2. IDENTIFY VOLUMES, in vph

Approach 3	
5: LT= 2	2:RT= 9
TH= 874	TH= 124
RT= 123	LT= 31
<--Approach 2	
Approach 1-->	
1:LT= 101	4: RT= 14
TH= 138	TH= 318
RT= 63	LT= 24
Approach 4	

Step 5. ASSIGN LANE VOLUMES, in vph

1 8	
2 7	
3 4 2	^+ 9
	<+ 124
< v >	v- 31
101 -^	< ^ >
138 +>	+ +
63 +v	
	3
	2 1 1
	4 8 4

Step 7. SUM OF CRITICAL VOLUMES

101(B1B2)+133(A2)+24(B4B3)+874(A3)
= 1132 vph

Step 8. INTERSECTION LEVEL OF SERVICE

(compare step 7 with table 6)

| D |

Step 9. RECALCULATE

Geometric Change:
Signal Change:
Volume Change:

Step 3. IDENTIFY PHASING

--^ v--	B2B1
--^ AND <--	A1B2 AND
--> OR v--	/OR A2B1
--> <--	A1A2
<	B4B3
>	
AND < ^	A3B4 AND
v > OR	/OR A4B3
^	A3A4
v	
A1 --> A3	B1 v-- B3 <
	v ^
A2 <-- A4	B2 --^ B4 >

Step 6a. CRITICAL VOLUMES, in vph (two phase signal)

Approach 3
Approach 1
See Step 6b.
Approach 2
Approach 4

COMMENTS

Exclusive right turns reduced 30 %
V/C Ratio = .82

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELK GROVE-FLORIN AND GERBER RD
Problem Statement: EXIST

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE-FLO GERBER RD R T T T L ^ T H H H T Approach 1 << >> ^-RT 1 LT--^ v v v <^-RTH 1 LTH-^> <--TH 1 1 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 1 1 Approach 4: ELK GROVE-FLO		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 24(B1) 56- 24= 32(B2) 24 A1B2 32(B2) 280- 32= 248(A1) 32 A1A2 248(A1) OR 137(A2) 248 B4B3 11(B4) 752- 11= 741(B3) 11 A4B3 741(B3) 762- 741= 21(A4) 741 A3A4 388(A3) OR 21(A4) 388			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 11 Approach 3 TH= 287 2:RT= 11 RT= 101 v TH= 263 LT= 24 <--Approach 2 Approach 1--> 1:LT= 56 ^ 4: RT= 77 TH= 280 TH= 685 RT= 269 LT= 752 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 1 2 ----- 0 8 1 1 7 1 ^+ 11 + + <+ 126 < v > <- 137 v- 24				Step 7. SUM OF CRITICAL VOLUMES 56(B1B2)+248(A1)+752(B4B3)+388(A3) = 1444 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. Approach 2 ----- Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.05			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELK GROVE-FLORIN AND GERBER RD

Design Hour: PM

Problem Statement: EXIST

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE-FLO 1 1 ^ R L N RT TT L TH H H T Approach 1 << >> ^--RT 1 LT--^ v v v <^--RTH 1 LTH--^ <--TH 1 1 TH--> <v--LTH RTH--v ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 1 1 Approach 4: ELK GROVE-FLO		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- Possible Volume Adjusted able Critical Carryover Critical Phase Volume to next Volume in vph phase in vph B2B1 87(B1) 102- 87= 15(B2) 87 A1B2 15(B2) 585- 15= 570(A1) 15 A1A2 570(A1) OR 157(A2) 570 B4B3 46(B4) 336- 46= 290(B3) 46 A4B3 290(B3) 408- 290= 118(A4) 290 A3A4 784(A3) OR 118(A4) 784			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 46 TH= 658 RT= 126 Approach 3 2: RT= 15 TH= 299 LT= 87 <--Approach 2 Approach 1--> 1: LT= 102 TH= 274 RT= 835 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 1 6 ----- 2 5 4 6 8 6 + + < v > ^+ 15 <+ 142 <- 157 v- 87 102 -^ < ^ > 274 -> + + 835 -v ----- 3 3 3 8 2 6 5 3				Step 7. SUM OF CRITICAL VOLUMES 102(B1B2)+570(A1)+336(B4B3)+784(A3) = 1792 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v ----- A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.3			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: BRADSHAW AND GERBER RD
Problem Statement: EXIST

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD GERBER RD 1 1 1 ^ R L N R T T T L T H H H T Approach 1 << > ^--RT 1 LT--^ v v v <^--RTH 1 LTH--^> <--TH TH--> <v-LTH 1 RTH--v ^ ^ ^ v--LT 1 RT--v << > Approach 2 L L T R R T T H T T GERBER RD H H 1 1 Approach 4: BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph B2B1 6(B1) 206- 6= 200(B2) 6 A1B2 200(B2) 101- 200= 0(A1) 200 A1A2 132(A2) OR 0(A1) 132 B4B3 11(B4) 38- 11= 27(B3) 11 A4B3 27(B3) 777- 27= 750(A4) 27 A3A4 750(A4) OR 241(A3) 750			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 11 Approach 3 TH= 241 2:RT= 48 RT= 64 v TH= 84 LT= 6 <--Approach 2 Approach 1-->		Step 5. ASSIGN LANE VOLUMES, in vph 2 ----- 6 4 1 ----- 4 1 1 ^+ 48 <+ 84 < v > v- 6				Step 7. SUM OF CRITICAL VOLUMES 206(B1B2)+132(A2)+38(B4B3)+750(A4) = 1126 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) D			
1:LT= 206 ^ 4: RT= 22 TH= 84 TH= 755 RT= 17 LT= 38 Approach 4		206 -^ < ^ > 84 +> + + 17 +v ----- 7 ----- 3 5 2 8 5 2				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. Approach 2 ----- Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .82			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: BRADSHAW AND GERBER RD
Problem Statement: EXIST

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD 1 1 1 ^ R L N GERBER RD R T T T L T H H H T Approach 1 << >> ^--RT 1 LT--^ v v v <^--RTH 1 LTH--^> <--TH TH--> <v--LTH 1 RTH--v ^ ^ ^ v--LT 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 1 1 Approach 4: BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 0 0 d.Opposing volume in vph : 0 0 0 0 e.LT capacity on green (vph) : 0 0 0 0 f.LT capacity in vph (b+e) : 0 0 0 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? :				Prob- able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph B2B1 17(B1) 105- 17= 88(B2) 17 A1B2 88(B2) 120- 88= 32(A1) 88 A1A2 115(A2) OR 32(A1) 115 B4B3 22(B3) 44- 22= 22(B4) 22 A3B4 22(B4) 671- 22= 649(A3) 22 A3A4 649(A3) OR 217(A4) 649			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 44 TH= 671 RT= 213 Approach 3 2:RT= 26 TH= 89 LT= 17 v <--Approach 2 Approach 1-->		Step 5. ASSIGN LANE VOLUMES, in vph 2 6 1 7 4 3 1 4 < v > 105 -^ < ^ > 82 +> + + 38 +v 2 2 1 2 1 6				Step 7. SUM OF CRITICAL VOLUMES 105(B1B2)+115(A2)+44(B3B4)+649(A3) = 913 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) B Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .66			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELK GROVE-FLORIN AND CALVINE RD
Problem Statement: EXISTING

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP																										
Approach 3: ELK GROVE-FLO 1 1 1 ^ R L N CALVINE ROAD R T T L T H H H T Approach 1 << >> ^--RT 2 LT--^ v v v <^--RTH 1 LTH--^> <--TH TH--> <v--LTH 1 RTH--v> ^ ^ ^ v--LT 2 RT--v << >> Approach 2 L L T R R T T H T T CALVINE ROAD H H 1 1 1 Approach 4: ELK GROVE-FLO		a.No. of change intervals/hour	: 0	: 0	: 0	: 0	b.LT capacity on change (vph)	: 0	: 0	: 0	c.G/C ratio	: 0	: 0	: 0	d.Opposing volume in vph	: 0	: 0	: 0	e.LT capacity on green (vph)	: 0	: 0	: 0	f.LT capacity in vph (b+e)	: 0	: 0	: 0	g.Left turn volume in vph	: 0	: 0	: 0	h.Is volume > cap. (g>f) ?	:
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 27 Approach 3 2:RT= 43 TH= 420 TH= 190 RT= 215 v LT= 91 <--Approach 2 Approach 1--> 1:LT= 57 ^ 4: RT= 64 TH= 125 TH= 500 RT= 98 LT= 122 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 2 1 3 1 0 1 2 5 3 8 7 + + < v v > 32 -^ 26 -^ < ^ ^ > 125 +> + + 98 +v 1 2 2 2 8 1 6 2 2 8 4				Step 7. SUM OF CRITICAL VOLUMES 51(B2B1)+223(A1)+122(B4B3)+318(A3) = 714 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) A																										
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS V/C Ratio = .52																										

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELK GROVE-FLORIN AND CALVINE RD

Design Hour: PM

Problem Statement: EXISTING

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE-FLO		-----Approach-----				Possible Volume Adjusted			
1 1 1 ^		: -1- -2- -3- -4-				Critical Volume Critical			
R L N		e.No. of change				Prob- able Phase			
R T T T L		: 0 0 0 0				in vph			
T H H H T		: intervals/hour				to next phase			
Approach 1 << >> ^--RT		: b.LT capacity on				in vph			
2 LT--^ v v v <^--RTH 1		: change (vph)				B2B1 54(B1) 83- 54= 29(B2) 54			
LTH-^>		: c.G/C ratio				A1B2 29(B2) 293- 29= 264(A1) 29			
TH-->		: d.Opposing volume				A1A2 264(A1) OR 174(A2) 264			
1 RTH-v> ^ ^ ^ v--LT 2		: in vph				B4B3 49(B4) 108- 49= 59(B3) 49			
RT--v << >> Approach 2		: e.LT capacity on				A4B3 59(B3) 239- 59= 180(A4) 59			
L L T R R		: f.LT capacity in				A3A4 368(A3) OR 180(A4) 368			
T T H T T CALVINE ROAD		: vph (b+e)							
H H		: g.Left turn volume							
1 1 1		: in vph							
Approach 4: ELK GROVE-FLO		: h.Is volume > cap.							
		: (g>f) ?							

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		1 2 3		83(B1B2)+264(A1)+108(B4B3)+368(A3)	
3: LT= 49		0 6 6 4		= 823 vph	
TH= 636		0 8 8 9			
RT= 100 v		+ +		Step 8. INTERSECTION LEVEL OF SERVICE	
		< v v >		(compare step 7 with table 6)	
				A	
<--Approach 2					
Approach 1-->		83 ^-			
		68 ^-			
		< ^ ^ >		Step 9. RECALCULATE	
1: LT= 149 ^		162 +>		Geometric Change:	
TH= 162		131 +v		Signal Change:	
RT= 131				Volume Change:	
Approach 4					
		1 2 1			
		0 3 7 6			
		8 9 5 4			

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1		Approach 3		
--^ AND <-- A1B2 AND				
--> OR v-- /OR A2B1				See Step 6b.
--> <-- A1A2				
< B4B3		Approach 1		V/C Ratio = .6
>				
AND < ^ A3B4 AND		Approach 2		
v > OR /OR A4B3				
^ A3A4				
v				
A1 --> A3 B1 v-- B3 <		Approach 4		
v ^				
A2 <-- A4 B2 --^ B4 >				

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45
 PEAK HOUR FACTOR..... .9
 AREA POPULATION..... 20000
 NAME OF THE EAST/WEST STREET..... JACKSON HWY
 NAME OF THE NORTH/SOUTH STREET..... EXCELSIOR RD
 NAME OF THE ANALYST..... FP
 DATE OF THE ANALYSIS (mm/dd/yy)..... 02-22-1996
 TIME PERIOD ANALYZED..... AM

OTHER INFORMATION.... EXIST

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: 4-LEG
 MAJOR STREET DIRECTION: EAST/WEST
 CONTROL TYPE NORTHBOUND: STOP SIGN
 CONTROL TYPE SOUTHBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	25	52	113	3
THRU	152	453	17	5
RIGHT	22	15	47	9

NUMBER OF LANES AND LANE USAGE

	EB	WB	NB	SB
LANES	2	2	1	1

LANE USAGE

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	0	0	
WESTBOUND	0.00	0	0	
NORTHBOUND	0.00	90	100	
SOUTHBOUND	0.00	90	100	

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
NB	6.10	5.60	0.00	5.60
SB	6.10	5.60	0.00	5.60
MAJOR LEFTS				
EB	5.80	5.80	0.00	5.80
WB	5.80	5.80	0.00	5.80
MINOR THROUGHGS				
NB	7.40	7.40	0.00	7.40
SB	7.40	7.40	0.00	7.40
MINOR LEFTS				
NB	7.90	7.90	0.00	7.90
SB	7.90	7.90	0.00	7.90

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... JACKSON HWY
NAME OF THE NORTH/SOUTH STREET..... EXCELSIOR RD
DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
OTHER INFORMATION.... EXIST

CAPACITY AND LEVEL-OF-SERVICE

MOVEMENT	FLOW-RATE v (pcph)	POTEN-TIAL	ACTUAL	SHARED		RESERVE		LOS
		CAPACITY c (pcph) p	MOVEMENT CAPACITY c (pcph) M	CAPACITY c (pcph) SH	CAPACITY c (pcph)	c = c - v R	SH	

MINOR STREET

NB LEFT	138	210	190	>	190	>	52	> E
THROUGH	21	247	229	>	246	229	> 30	208 > E C
RIGHT	57	976	976	>	976	>	918	> A

MINOR STREET

SB LEFT	4	189	160	>	160	>	156	> D
THROUGH	6	246	227	>	328	227	> 307	221 > B C
RIGHT	11	813	813	>	813	>	802	> A

MAJOR STREET

EB LEFT	31	567	567		567		536	A
WB LEFT	64	840	840		840		776	A

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... JACKSON HWY
 NAME OF THE NORTH/SOUTH STREET.... EXCELSIOR RD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
 OTHER INFORMATION.... EXIST

FC = 239 → LOS C

1985 HCM: UNSIGNALIZED INTERSECTIONS

Page-1

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45

PEAK HOUR FACTOR..... .9

AREA POPULATION..... 20000

NAME OF THE EAST/WEST STREET..... JACKSON HWY

NAME OF THE NORTH/SOUTH STREET..... EXCELSIOR RD

NAME OF THE ANALYST..... FP

DATE OF THE ANALYSIS (mm/dd/yy)..... 02-22-1996

TIME PERIOD ANALYZED..... PM

OTHER INFORMATION.... EXIST

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: 4-LEG

MAJOR STREET DIRECTION: EAST/WEST

CONTROL TYPE NORTHBOUND: STOP SIGN

CONTROL TYPE SOUTHBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	10	38	19	4
THRU	428	209	2	11
RIGHT	86	11	68	9

NUMBER OF LANES AND LANE USAGE

	EB	WB	NB	SB
LANES	2	2	1	1

LANE USAGE

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	0	0	
WESTBOUND	0.00	0	0	
NORTHBOUND	0.00	90	100	
SOUTHBOUND	0.00	90	100	

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
NB	6.10	5.60	0.00	5.60
SB	6.10	5.60	0.00	5.60
MAJOR LEFTS				
EB	5.80	5.80	0.00	5.80
WB	5.80	5.80	0.00	5.80
MINOR THROUGHGS				
NB	7.40	7.40	0.00	7.40
SB	7.40	7.40	0.00	7.40
MINOR LEFTS				
NB	7.90	7.90	0.00	7.90
SB	7.90	7.90	0.00	7.90

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... JACKSON HWY
NAME OF THE NORTH/SOUTH STREET..... EXCELSIOR RD
DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; PM
OTHER INFORMATION..... EXIST

CAPACITY AND LEVEL-OF-SERVICE

Page-3

MOVEMENT	FLOW-RATE v(pcph)	POTEN-	ACTUAL	SHARED	RESERVE		LOS
		TIAL	MOVEMENT		CAPACITY		
		c (pcph)	c (pcph)	c (pcph)	c = c	- v	
		p	M	SH	R	SH	

MINOR STREET

NB LEFT	23	196	176	>	176	>	152	>	D
THROUGH	2	233	219	>	437	219	>	328	216 >B C
RIGHT	83	788	788	>	788	>	705	>	A

MINOR STREET

SB LEFT	5	161	141	>	141	>	136	>	D
THROUGH	13	217	204	>	261	204	>	232	191 >C D
RIGHT	11	952	952	>	952	>	941	>	A

MAJOR STREET

EB LEFT	12	793	793		793		780		A
WB LEFT	46	531	531		531		484		A

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... JACKSON HWY
 NAME OF THE NORTH/SOUTH STREET.... EXCELSIOR RD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; PM
 OTHER INFORMATION.... EXIST

RC = 378 ⇒ LOS B

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45

PEAK HOUR FACTOR..... .9

AREA POPULATION..... 20000

NAME OF THE EAST/WEST STREET..... ELDER CREEK RD

NAME OF THE NORTH/SOUTH STREET..... EXCELSIOR RD

NAME OF THE ANALYST..... FP

DATE OF THE ANALYSIS (mm/dd/yy)..... 02-22-1996

TIME PERIOD ANALYZED..... AM

OTHER INFORMATION.... EXIST

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: T-INTERSECTION

MAJOR STREET DIRECTION: NORTH/SOUTH

CONTROL TYPE EASTBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	32	--	66	43
THRU	197	--	153	27
RIGHT	6	--	10	67

NUMBER OF LANES

	EB	WB	NB	SB
LANES	1	--	1	2

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	0	0	
WESTBOUND	----	---	---	-
NORTHBOUND	0.00	90	100	
SOUTHBOUND	0.00	90	100	

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	---	---	---
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
EB	6.10	5.60	0.00	5.60
MAJOR LEFTS				
NB	5.80	5.80	0.00	5.80
MINOR LEFTS				
EB	7.90	7.90	0.00	7.90

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... ELDER CREEK RD
NAME OF THE NORTH/SOUTH STREET.... EXCELSIOR RD
DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
OTHER INFORMATION.... EXIST

CAPACITY AND LEVEL-OF-SERVICE

MOVEMENT	FLOW-RATE v(pcph)	POTEN-TIAL	ACTUAL	SHARED	RESERVE		LOS
		CAPACITY c (pcph) p	MOVEMENT CAPACITY c (pcph) M		CAPACITY c (pcph) SH	c = c - v	

MINOR STREET

EB LEFT	39	490	465	>	465	>	425	>	A
				>	507	>	461	>	A
RIGHT	7	987	987	>	987	>	980	>	A

MAJOR STREET

NB LEFT	81	930	930		930		850		A
---------	----	-----	-----	--	-----	--	-----	--	---

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... ELDER CREEK RD
 NAME OF THE NORTH/SOUTH STREET.... EXCELSIOR RD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
 OTHER INFORMATION.... EXIST

RC=709 => LOS A

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45

PEAK HOUR FACTOR..... .9

AREA POPULATION..... 20000

NAME OF THE EAST/WEST STREET..... ELDER CREEK RD

NAME OF THE NORTH/SOUTH STREET..... EXCELSIOR RD

NAME OF THE ANALYST..... FP

DATE OF THE ANALYSIS (mm/dd/yy)..... 02-22-1996

TIME PERIOD ANALYZED..... PM

OTHER INFORMATION.... EXIST

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: T-INTERSECTION

MAJOR STREET DIRECTION: NORTH/SOUTH

CONTROL TYPE EASTBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	55	--	13	43
THRU	197	--	32	116
RIGHT	37	--	10	23

NUMBER OF LANES

	EB	WB	NB	SB
LANES	1	--	1	2

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	0	0	
WESTBOUND	----	---	---	-
NORTHBOUND	0.00	90	100	
SOUTHBOUND	0.00	90	100	

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	---	---	---
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
EB	6.10	5.60	0.00	5.60
MAJOR LEFTS				
NB	5.80	5.80	0.00	5.80
MINOR LEFTS				
EB	7.90	7.90	0.00	7.90

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... ELDER CREEK RD
NAME OF THE NORTH/SOUTH STREET.... EXCELSIOR RD
DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; PM
OTHER INFORMATION.... EXIST

CAPACITY AND LEVEL-OF-SERVICE

MOVEMENT	FLOW-RATE	POTENTIAL CAPACITY	ACTUAL MOVEMENT CAPACITY	SHARED CAPACITY	RESERVE CAPACITY	LOS
	v(pcph)	c (pcph) p	c (pcph) M	c (pcph) SH	c = c - v R SH	

MINOR STREET

EB LEFT	67	585	579 >	579 >	512 >	A
			> 693	> 581	>	A
RIGHT	45	981	981 >	981 >	935 >	A

MAJOR STREET

NB LEFT	16	879	879	879	864	A
---------	----	-----	-----	-----	-----	---

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... ELDER CREEK RD
 NAME OF THE NORTH/SOUTH STREET.... EXCELSIOR RD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; PM
 OTHER INFORMATION..... EXIST

RC-616 → LOS A

1985 HCM: UNSIGNALIZED INTERSECTIONS

Page-1

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45
 PEAK HOUR FACTOR..... .9
 AREA POPULATION..... 20000
 NAME OF THE EAST/WEST STREET..... GERBER RD
 NAME OF THE NORTH/SOUTH STREET..... VINEYARD
 NAME OF THE ANALYST..... FP
 DATE OF THE ANALYSIS (mm/dd/yy)..... 02-22-1996
 TIME PERIOD ANALYZED..... AM
 OTHER INFORMATION.... EXIST

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: T-INTERSECTION
 MAJOR STREET DIRECTION: EAST/WEST
 CONTROL TYPE NORTHBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	55	4	63	--
THRU	113	72	32	--
RIGHT	8	24	7	--

NUMBER OF LANES

	EB	WB	NB	SB
LANES	1	1	1	--

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	0	0	
WESTBOUND	0.00	0	0	
NORTHBOUND	0.00	90	100	
SOUTHBOUND	----	---	---	

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	0	0	0
SOUTHBOUND	---	---	---

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
NB	6.10	5.60	0.00	5.60
MAJOR LEFTS				
WB	5.30	5.30	0.00	5.30
MINOR LEFTS				
NB	7.40	7.40	0.00	7.40

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... GERBER RD
NAME OF THE NORTH/SOUTH STREET.... VINEYARD
DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
OTHER INFORMATION.... EXIST

CAPACITY AND LEVEL-OF-SERVICE

Page-3

MOVEMENT	FLOW-RATE	POTENTIAL CAPACITY	ACTUAL MOVEMENT CAPACITY	SHARED CAPACITY	RESERVE CAPACITY	LOS
	v(pcph)	c (pcph) p	c (pcph) M	c (pcph) SH	c = c - v R SH	

MINOR STREET

NB LEFT	77	608	607 >	607 >	530 >	A
			> 629	>	544	>A
RIGHT	9	944	944 >	944 >	935 >	A

MAJOR STREET

WB LEFT	5	973	973	973	968	A
---------	---	-----	-----	-----	-----	---

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... GERBER RD
 NAME OF THE NORTH/SOUTH STREET.... VINEYARD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
 OTHER INFORMATION..... EXIST

RC = 567 = LOS A

1985 HCM: UNSIGNALIZED INTERSECTIONS

Page-1

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45

PEAK HOUR FACTOR..... .8

AREA POPULATION..... 150000

NAME OF THE EAST/WEST STREET..... GERBER RD

NAME OF THE NORTH/SOUTH STREET..... VINEYARD RD

NAME OF THE ANALYST..... F & P

DATE OF THE ANALYSIS (mm/dd/yy)..... 05-17-1995

TIME PERIOD ANALYZED..... PM

OTHER INFORMATION.... EXISTING

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: T-INTERSECTION

MAJOR STREET DIRECTION: EAST/WEST

CONTROL TYPE NORTHBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	55	5	29	--
THRU	81	104	32	--
RIGHT	53	40	5	--

NUMBER OF LANES

	EB	WB	NB	SB
LANES	1	1	1	--

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	90	20	N
WESTBOUND	0.00	90	20	N
NORTHBOUND	0.00	90	20	N
SOUTHBOUND	----	---	---	-

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	0	0	0
SOUTHBOUND	---	---	---

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
NB	6.10	6.10	0.00	6.10
MAJOR LEFTS				
WB	5.30	5.30	0.00	5.30
MINOR LEFTS				
NB	7.40	7.40	0.00	7.40

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... GERBER RD
NAME OF THE NORTH/SOUTH STREET.... VINEYARD RD
DATE AND TIME OF THE ANALYSIS..... 05-17-1995 ; PM
OTHER INFORMATION.... EXISTING

CAPACITY AND LEVEL-OF-SERVICE

MOVEMENT	FLOW-RATE	POTENTIAL CAPACITY	ACTUAL MOVEMENT CAPACITY	SHARED CAPACITY	RESERVE CAPACITY	LOS
	v (pcph)	c (pcph) p	c (pcph) M	c (pcph) SH	c = c - v R SH	

MINOR STREET

NB LEFT	40	564	561	> 590	561 > 544	521 > A
RIGHT	7	846	846	> 846	> 846	839 > A

MAJOR STREET

WB LEFT	7	950	950	950	944	A
---------	---	-----	-----	-----	-----	---

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... GERBER RD
 NAME OF THE NORTH/SOUTH STREET..... VINEYARD RD
 DATE AND TIME OF THE ANALYSIS..... 05-17-1995 ; PM
 OTHER INFORMATION.... EXISTING

RC: 596 ⇒ LOS A

1985 HCM: UNSIGNALIZED INTERSECTIONS

Page-1

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45

PEAK HOUR FACTOR..... .8

AREA POPULATION..... 150000

NAME OF THE EAST/WEST STREET..... GERBER RD

NAME OF THE NORTH/SOUTH STREET..... EXCELSIOR RD

NAME OF THE ANALYST..... F & P

DATE OF THE ANALYSIS (mm/dd/yy)..... 05-17-1995

TIME PERIOD ANALYZED..... AM

OTHER INFORMATION.... EXISTING

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: 4-LEG

MAJOR STREET DIRECTION: NORTH/SOUTH

CONTROL TYPE EASTBOUND: STOP SIGN

CONTROL TYPE WESTBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	111	1	25	0
THRU	0	4	222	27
RIGHT	8	0	2	44

NUMBER OF LANES AND LANE USAGE

	EB	WB	NB	SB
LANES	1	1	1	1
LANE USAGE	LTR	LTR		

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	90	20	N
WESTBOUND	0.00	90	20	N
NORTHBOUND	0.00	90	20	N
SOUTHBOUND	0.00	90	20	N

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
EB	6.10	6.10	0.00	6.10
WB	6.10	6.10	0.00	6.10
MAJOR LEFTS				
SB	5.30	5.30	0.00	5.30
NB	5.30	5.30	0.00	5.30
MINOR THROUGH				
EB	6.90	6.90	0.00	6.90
WB	6.90	6.90	0.00	6.90
MINOR LEFTS				
EB	7.40	7.40	0.00	7.40
WB	7.40	7.40	0.00	7.40

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... GERBER RD
NAME OF THE NORTH/SOUTH STREET.... EXCELSIOR RD
DATE AND TIME OF THE ANALYSIS..... 05-17-1995 ; AM
OTHER INFORMATION.... EXISTING

CAPACITY AND LEVEL-OF-SERVICE

MOVEMENT	FLOW-RATE v(pcph)	POTEN-TIAL	ACTUAL	SHARED		RESERVE		LOS
		CAPACITY c (pcph) p	MOVEMENT CAPACITY c (pcph) M	CAPACITY c (pcph) SH	CAPACITY c (pcph)	c = c - v R	v SH	

MINOR STREET

EB LEFT	153	481	468	>	468	>	315	> B
THROUGH	0	534	523	>	484	523	> 320	523 > B A
RIGHT	11	920	920	>	920	>	909	> A

MINOR STREET

WB LEFT	1	458	446	>	446	>	444	> A
THROUGH	6	513	502	>	490	502	> 483	497 > A A
RIGHT	0	713	713	>	713	>	713	> A

MAJOR STREET

SB LEFT	0	850	850		850		850	A
NB LEFT	34	997	997		997		963	A

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... GERBER RD
 NAME OF THE NORTH/SOUTH STREET.... EXCELSIOR RD
 DATE AND TIME OF THE ANALYSIS..... 05-17-1995 ; AM
 OTHER INFORMATION..... EXISTING

RC = 432 ⇒ LOS A

1985 HCM: UNSIGNALIZED INTERSECTIONS

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45
 PEAK HOUR FACTOR..... .85
 AREA POPULATION..... 150000
 NAME OF THE EAST/WEST STREET..... GERBER RD
 NAME OF THE NORTH/SOUTH STREET..... EXCELSIOR RD
 NAME OF THE ANALYST..... F & P
 DATE OF THE ANALYSIS (mm/dd/yy)..... 05-17-1995
 TIME PERIOD ANALYZED..... PM
 OTHER INFORMATION.... EXISTING

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: 4-LEG
 MAJOR STREET DIRECTION: NORTH/SOUTH
 CONTROL TYPE EASTBOUND: STOP SIGN
 CONTROL TYPE WESTBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	36	1	13	0
THRU	1	2	32	185
RIGHT	36	0	2	84

NUMBER OF LANES AND LANE USAGE

	EB	WB	NB	SB
LANES	1	1	1	1
LANE USAGE	LTR	LTR		

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	90	20	N
WESTBOUND	0.00	90	20	N
NORTHBOUND	0.00	90	20	N
SOUTHBOUND	0.00	90	20	N

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
EB	6.10	6.10	0.00	6.10
WB	6.10	6.10	0.00	6.10
MAJOR LEFTS				
SB	5.30	5.30	0.00	5.30
NB	5.30	5.30	0.00	5.30
MINOR THROUGHGS				
EB	6.90	6.90	0.00	6.90
WB	6.90	6.90	0.00	6.90
MINOR LEFTS				
EB	7.40	7.40	0.00	7.40
WB	7.40	7.40	0.00	7.40

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... GERBER RD
NAME OF THE NORTH/SOUTH STREET..... EXCELSIOR RD
DATE AND TIME OF THE ANALYSIS..... 05-17-1995 ; PM
OTHER INFORMATION..... EXISTING

CAPACITY AND LEVEL-OF-SERVICE

MOVEMENT	FLOW-RATE	POTENTIAL CAPACITY	ACTUAL MOVEMENT CAPACITY	SHARED CAPACITY	RESERVE CAPACITY	LOS
	v (pcph)	c (pcph) p	c (pcph) M	c (pcph) SH	c = c - v R SH	

MINOR STREET

EB LEFT	47	521	513 >	513 >	467 >	A
THROUGH	1	573	566 >	600 566 >	505 565 >	A A
RIGHT	47	723	723 >	723 >	676 >	A

MINOR STREET

WB LEFT	1	455	432 >	432 >	430 >	A
THROUGH	3	535	529 >	492 529 >	488 526 >	A A
RIGHT	0	944	944 >	944 >	944 >	A

MAJOR STREET

SB LEFT	0	999	999	999	999	A
NB LEFT	17	815	815	815	798	A

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... GERBER RD
 NAME OF THE NORTH/SOUTH STREET..... EXCELSIOR RD
 DATE AND TIME OF THE ANALYSIS..... 05-17-1995 ; PM
 OTHER INFORMATION.... EXISTING

RC = 547 → LOS A

1985 HCM: UNSIGNALIZED INTERSECTIONS

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45
 PEAK HOUR FACTOR..... .9
 AREA POPULATION..... 20000
 NAME OF THE EAST/WEST STREET..... CALVINE RD
 NAME OF THE NORTH/SOUTH STREET..... WATERMAN RD
 NAME OF THE ANALYST..... FP
 DATE OF THE ANALYSIS (mm/dd/yy)..... 02-22-1996
 TIME PERIOD ANALYZED..... AM

OTHER INFORMATION.... EXIST

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: 4-LEG
 MAJOR STREET DIRECTION: EAST/WEST
 CONTROL TYPE NORTHBOUND: STOP SIGN
 CONTROL TYPE SOUTHBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	18	0	7	6
THRU	241	207	2	6
RIGHT	7	5	5	42

NUMBER OF LANES AND LANE USAGE

	EB	WB	NB	SB
LANES	1	1	1	2
LANE USAGE				LT + R

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	0	0	
WESTBOUND	0.00	0	0	
NORTHBOUND	0.00	90	100	
SOUTHBOUND	0.00	90	100	

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
NB	6.10	5.60	0.00	5.60
SB	6.10	5.60	0.00	5.60
MAJOR LEFTS				
EB	5.30	5.30	0.00	5.30
WB	5.30	5.30	0.00	5.30
MINOR THROUGHGS				
NB	6.90	6.90	0.00	6.90
SB	6.90	6.90	0.00	6.90
MINOR LEFTS				
NB	7.40	7.40	0.00	7.40
SB	7.40	7.40	0.00	7.40

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... CALVINE RD
NAME OF THE NORTH/SOUTH STREET.... WATERMAN RD
DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
OTHER INFORMATION.... EXIST

CAPACITY AND LEVEL-OF-SERVICE

MOVEMENT	FLOW-RATE v(pcph)	POTEN-	ACTUAL	SHARED		RESERVE		LOS
		TIAL CAPACITY c (pcph) p	MOVEMENT CAPACITY c (pcph) M	CAPACITY c (pcph) SH	CAPACITY c (pcph) R	CAPACITY c (pcph) SH	CAPACITY c (pcph) SH	

MINOR STREET

NB LEFT	9	350	329	>	329	>	320	> B
THROUGH	2	429	422	>	434	422	>	417 420 >A A
RIGHT	6	801	801	>	801	>	795	> A

MINOR STREET

SB LEFT	7	378	370	>	394	370	>	379 362 >B B
THROUGH	7	428	422	>	422	>	414	> A
RIGHT	51	839	839		839		788	A

MAJOR STREET

EB LEFT	22	893	893		893		871	A
WB LEFT	0	854	854		854		854	A

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... CALVINE RD
 NAME OF THE NORTH/SOUTH STREET..... WATERMAN RD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
 OTHER INFORMATION..... EXIST

RC=489 ⇒ LOS A

1985 HCM: UNSIGNALIZED INTERSECTIONS

Page-1

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45
 PEAK HOUR FACTOR..... .9
 AREA POPULATION..... 20000
 NAME OF THE EAST/WEST STREET..... CALVINE RD
 NAME OF THE NORTH/SOUTH STREET..... WATERMAN RD
 NAME OF THE ANALYST..... FP
 DATE OF THE ANALYSIS (mm/dd/yy)..... 02-22-1996
 TIME PERIOD ANALYZED..... PM
 OTHER INFORMATION.... EXIST

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: 4-LEG
 MAJOR STREET DIRECTION: EAST/WEST
 CONTROL TYPE NORTHBOUND: STOP SIGN
 CONTROL TYPE SOUTHBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	36	4	4	1
THRU	120	205	9	1
RIGHT	4	6	4	18

NUMBER OF LANES AND LANE USAGE

	EB	WB	NB	SB
LANES	1	1	1	2
LANE USAGE				LT + R

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	0	0	
WESTBOUND	0.00	0	0	
NORTHBOUND	0.00	90	100	
SOUTHBOUND	0.00	90	100	

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
NB	6.10	5.60	0.00	5.60
SB	6.10	5.60	0.00	5.60
MAJOR LEFTS				
EB	5.30	5.30	0.00	5.30
WB	5.30	5.30	0.00	5.30
MINOR THROUGHS				
NB	6.90	6.90	0.00	6.90
SB	6.90	6.90	0.00	6.90
MINOR LEFTS				
NB	7.40	7.40	0.00	7.40
SB	7.40	7.40	0.00	7.40

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... CALVINE RD
NAME OF THE NORTH/SOUTH STREET..... WATERMAN RD
DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; PM
OTHER INFORMATION.... EXIST

CAPACITY AND LEVEL-OF-SERVICE

MOVEMENT	FLOW-RATE v (pcph)	POTEN-TIAL	ACTUAL	SHARED		RESERVE		LOS
		CAPACITY c (pcph) P	MOVEMENT CAPACITY c (pcph) M	CAPACITY c (pcph) SH	CAPACITY c (pcph)	CAPACITY c = c - v R SH		

MINOR STREET

NB LEFT	5	442	420	>	420	>	415	>	A
THROUGH	11	503	486	>	526	486	>	506	475 >A A
RIGHT	5	938	938	>	938	>	933	>	A

MINOR STREET

SB LEFT	1	447	425	>	454	425	>	451	424 >A A
THROUGH	1	503	487	>	487	>	486	>	A
RIGHT	22	841	841		841		819		A

MAJOR STREET

EB LEFT	44	894	894		894		850		A
WB LEFT	5	971	971		971		966		A

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... CALVINE RD
 NAME OF THE NORTH/SOUTH STREET.... WATERMAN RD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; PM
 OTHER INFORMATION.... EXIST

RC = 677 LOS A

1985 HCM: UNSIGNALIZED INTERSECTIONS

Page-1

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45
 PEAK HOUR FACTOR..... .9
 AREA POPULATION..... 20000
 NAME OF THE EAST/WEST STREET..... CALVINE RD
 NAME OF THE NORTH/SOUTH STREET..... VINEYARD
 NAME OF THE ANALYST..... FP
 DATE OF THE ANALYSIS (mm/dd/yy)..... 02-22-1996
 TIME PERIOD ANALYZED..... AM
 OTHER INFORMATION.... EXIST

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: T-INTERSECTION
 MAJOR STREET DIRECTION: EAST/WEST
 CONTROL TYPE SOUTHBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	7	4	--	2
THRU	87	146	--	1
RIGHT	4	6	--	27

NUMBER OF LANES

	EB	WB	NB	SB
LANES	1	2	--	1

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	0	0	
WESTBOUND	0.00	0	0	
NORTHBOUND	----	---	---	-
SOUTHBOUND	0.00	90	100	

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	---	---	---
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
SB	6.10	5.60	0.00	5.60
MAJOR LEFTS				
EB	5.80	5.80	0.00	5.80
MINOR LEFTS				
SB	7.90	7.90	0.00	7.90

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... CALVINE RD
NAME OF THE NORTH/SOUTH STREET.... VINEYARD
DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
OTHER INFORMATION.... EXIST

CAPACITY AND LEVEL-OF-SERVICE

Page-3

MOVEMENT	FLOW-RATE	POTENTIAL CAPACITY	ACTUAL MOVEMENT CAPACITY	SHARED CAPACITY	RESERVE CAPACITY	LOS
	v(pcph)	c (pcph) p	c (pcph) M	c (pcph) SH	c = c - v R SH	

MINOR STREET

SB LEFT	2	522	519	> 519	> 516	> A
				> 922	> 887	> A
RIGHT	33	979	979	> 979	> 946	> A

MAJOR STREET

EB LEFT	9	865	865	865	856	A
---------	---	-----	-----	-----	-----	---

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... CALVINE RD
 NAME OF THE NORTH/SOUTH STREET..... VINEYARD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
 OTHER INFORMATION..... EXIST

RC = 922 = LOS A

1985 HCM: UNSIGNALIZED INTERSECTIONS

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45
 PEAK HOUR FACTOR..... .9
 AREA POPULATION..... 20000
 NAME OF THE EAST/WEST STREET..... CALVINE RD
 NAME OF THE NORTH/SOUTH STREET..... VINEYARD
 NAME OF THE ANALYST..... FP
 DATE OF THE ANALYSIS (mm/dd/yy)..... 02-22-1996
 TIME PERIOD ANALYZED..... PM
 OTHER INFORMATION.... EXIST

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: T-INTERSECTION
 MAJOR STREET DIRECTION: EAST/WEST
 CONTROL TYPE SOUTHBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	4	4	--	8
THRU	109	133	--	1
RIGHT	4	3	--	132

NUMBER OF LANES

	EB	WB	NB	SB
LANES	1	2	--	1

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	0	0	
WESTBOUND	0.00	0	0	
NORTHBOUND	----	---	---	-
SOUTHBOUND	0.00	90	100	

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	---	---	---
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
SB	6.10	5.60	0.00	5.60
MAJOR LEFTS				
EB	5.80	5.80	0.00	5.80
MINOR LEFTS				
SB	7.90	7.90	0.00	7.90

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... CALVINE RD
NAME OF THE NORTH/SOUTH STREET.... VINEYARD
DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; PM
OTHER INFORMATION.... EXIST

CAPACITY AND LEVEL-OF-SERVICE

MOVEMENT	FLOW-RATE	POTENTIAL CAPACITY	ACTUAL MOVEMENT CAPACITY	SHARED CAPACITY	RESERVE CAPACITY	LOS
	v (pcph)	c (pcph)	c (pcph)	c (pcph)	c = c - v	
		p	M	SH	R SH	

MINOR STREET

SB LEFT	10	518	516	>	516	>	507	>	A
				>	933	>	762	>	A
RIGHT	161	981	981	>	981	>	820	>	A

MAJOR STREET

EB LEFT	5	883	883		883		878		A
---------	---	-----	-----	--	-----	--	-----	--	---

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... CALVINE RD
 NAME OF THE NORTH/SOUTH STREET.... VINEYARD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; PM
 OTHER INFORMATION..... EXIST

RL-767 => LOS A

Existing A.M. Peak
Elder Creek Rd. and S. Watt Ave.

		EB	WB	NB	SB
1	LT Volume	69	17	41	2
2	TH Volume	103	203	489	238
3	RT Volume	15	40	9	112
4	Peak Hour Factor	0.90	0.90	0.90	0.90
5	Lanes on Subject Approach	1	2	1	1
6	Lanes on Opposing Approach	2	1	1	1
7	LT Flow Rate	76.667	18.889	45.556	2.2222
8	TH Flow Rate	114.44	225.56	543.33	264.44
9	RT Flow Rate	16.667	44.444	10	124.44
10	Approach Flow Rate	207.78	288.89	598.89	391.11
11	Proportion LT	0.37	0.07	0.08	0.01
12	Proportion RT	0.08	0.15	0.02	0.32
13	Opposing Approach (Direction)	WB	EB	SB	NB
14	Conflicting Approaches (Directions)	NB, SB	NB, SB	EB, WB	EB, WB
15	Subject Approach Flow Rate	207.78	288.89	598.89	391.11
16	Opposing Approach Flow Rate	260	187	352	539
17	Conflicting Approaches Flow Rate	891	891	447	447
18	Total Intersection Flow Rate	1358.8	1366.9	1397.9	1377.1
19	Proportion, Subject Approach Flow Rate	0.15	0.21	0.43	0.28
20	Proportion, Opposing Approach Flow Rate	0.19	0.14	0.25	0.39
21	Proportion, Conflicting Approaches Flow Rate	0.66	0.65	0.32	0.32
22	LT, Opposing Approach	17	69	2	41
23	RT, Opposing Approach	40	15	112	9
24	LT, Conflicting Approaches	43	43	86	86
25	RT, Conflicting Approaches	121	121	55	55
26	Proportion LT, Opposing Approach	0.07	0.37	0.01	0.08
27	Proportion RT, Opposing Approach	0.15	0.08	0.32	0.02
28	Proportion LT, Conflicting Approaches	0.05	0.05	0.19	0.19
29	Proportion RT, Conflicting Approaches	0.14	0.14	0.12	0.12
30	Approach Capacity	324	539	746	618
31	Volume/Capacity Ratio	0.64	0.54	0.80	0.63
32	Delay	11	8	21	11
33	Level of Service	C	B	D	C
34	Average Delay (Intersection)	15			
35	Level of Service (Intersection)	C			

Existing P.M. Peak
Elder Creek Rd. and S. Watt Ave.

		EB	WB	NB	SB
1	LT Volume	98	24	18	43
2	TH Volume	197	130	290	463
3	RT Volume	38	24	10	72
4	Peak Hour Factor	0.90	0.90	0.90	0.90
5	Lanes on Subject Approach	1	2	1	1
6	Lanes on Opposing Approach	2	1	1	1
7	LT Flow Rate	108.89	26.667	20	47.778
8	TH Flow Rate	218.89	144.44	322.22	514.44
9	RT Flow Rate	42.222	26.667	11.111	80
10	Approach Flow Rate	370	197.78	353.33	642.22
11	Proportion LT	0.29	0.13	0.06	0.07
12	Proportion RT	0.11	0.13	0.03	0.12
13	Opposing Approach (Direction)	WB	EB	SB	NB
14	Conflicting Approaches (Directions)	NB, SB	NB, SB	EB, WB	EB, WB
15	Subject Approach Flow Rate	370	197.78	353.33	642.22
16	Opposing Approach Flow Rate	178	333	578	318
17	Conflicting Approaches Flow Rate	896	896	511	511
18	Total Intersection Flow Rate	1444	1426.8	1442.3	1471.2
19	Proportion, Subject Approach Flow Rate	0.26	0.14	0.24	0.44
20	Proportion, Opposing Approach Flow Rate	0.12	0.23	0.40	0.22
21	Proportion, Conflicting Approaches Flow Rate	0.62	0.63	0.35	0.35
22	LT, Opposing Approach	24	98	43	18
23	RT, Opposing Approach	24	38	72	10
24	LT, Conflicting Approaches	61	61	122	122
25	RT, Conflicting Approaches	82	82	62	62
26	Proportion LT, Opposing Approach	0.13	0.29	0.07	0.06
27	Proportion RT, Opposing Approach	0.13	0.11	0.12	0.03
28	Proportion LT, Conflicting Approaches	0.07	0.07	0.24	0.24
29	Proportion RT, Conflicting Approaches	0.09	0.09	0.12	0.12
30	Approach Capacity	336	544	593	642
31	Volume/Capacity Ratio	1.10	0.36	0.60	1.00
32	Delay	66	4	10	45
33	Level of Service	F	A	B	E
34	Average Delay (Intersection)	37			
35	Level of Service (Intersection)	E			

Existing A.M. Peak
Excelsior Rd and Florin Rd

		EB	WB	NB	SB
1	LT Volume	7	35	29	2
2	TH Volume	87	146	202	27
3	RT Volume	10	6	104	6
4	Peak Hour Factor	0.90	0.90	0.90	0.90
5	Lanes on Subject Approach	1	1	1	1
6	Lanes on Opposing Approach	1	1	1	1
7	LT Flow Rate	7.7778	38.889	32.222	2.2222
8	TH Flow Rate	96.667	162.22	224.44	30
9	RT Flow Rate	11.111	6.6667	115.56	6.6667
10	Approach Flow Rate	115.56	207.78	372.22	38.889
11	Proportion LT	0.07	0.19	0.09	0.06
12	Proportion RT	0.10	0.03	0.31	0.17
13	Opposing Approach (Direction)	WB	EB	SB	NB
14	Conflicting Approaches (Directions)	NB, SB	NB, SB	EB, WB	EB, WB
15	Subject Approach Flow Rate	115.56	207.78	372.22	38.889
16	Opposing Approach Flow Rate	187	104	35	335
17	Conflicting Approaches Flow Rate	370	370	291	291
18	Total Intersection Flow Rate	672.56	681.78	698.22	664.89
19	Proportion, Subject Approach Flow Rate	0.17	0.30	0.53	0.06
20	Proportion, Opposing Approach Flow Rate	0.28	0.15	0.05	0.50
21	Proportion, Conflicting Approaches Flow Rate	0.55	0.54	0.42	0.44
22	LT, Opposing Approach	35	7	2	29
23	RT, Opposing Approach	6	10	6	104
24	LT, Conflicting Approaches	31	31	42	42
25	RT, Conflicting Approaches	110	110	16	16
26	Proportion LT, Opposing Approach	0.19	0.07	0.06	0.09
27	Proportion RT, Opposing Approach	0.03	0.10	0.17	0.31
28	Proportion LT, Conflicting Approaches	0.08	0.08	0.14	0.14
29	Proportion RT, Conflicting Approaches	0.30	0.30	0.05	0.05
30	Approach Capacity	481	575	659	520
31	Volume/Capacity Ratio	0.24	0.36	0.57	0.07
32	Delay	2	4	9	1
33	Level of Service	A	A	B	A
34	Average Delay (Intersection)	6			
35	Level of Service (Intersection)	B			

Existing P.M. Peak
Excelsior Rd and Florin Rd

		EB	WB	NB	SB
1	LT Volume	4	93	9	8
2	TH Volume	109	133	33	132
3	RT Volume	30	3	44	14
4	Peak Hour Factor	0.90	0.90	0.90	0.90
5	Lanes on Subject Approach	1	1	1	1
6	Lanes on Opposing Approach	1	1	1	1
7	LT Flow Rate	4.4444	103.33	10	8.8889
8	TH Flow Rate	121.11	147.78	36.667	146.67
9	RT Flow Rate	33.333	3.3333	48.889	15.556
10	Approach Flow Rate	158.89	254.44	95.556	171.11
11	Proportion LT	0.03	0.41	0.10	0.05
12	Proportion RT	0.21	0.01	0.51	0.09
13	Opposing Approach (Direction)	WB	EB	SB	NB
14	Conflicting Approaches (Directions)	NB, SB	NB, SB	EB, WB	EB, WB
15	Subject Approach Flow Rate	158.89	254.44	95.556	171.11
16	Opposing Approach Flow Rate	229	143	154	86
17	Conflicting Approaches Flow Rate	240	240	372	372
18	Total Intersection Flow Rate	627.89	637.44	621.56	629.11
19	Proportion, Subject Approach Flow Rate	0.25	0.40	0.15	0.27
20	Proportion, Opposing Approach Flow Rate	0.36	0.22	0.25	0.14
21	Proportion, Conflicting Approaches Flow Rate	0.38	0.38	0.60	0.59
22	LT, Opposing Approach	93	4	8	9
23	RT, Opposing Approach	3	30	14	44
24	LT, Conflicting Approaches	17	17	97	97
25	RT, Conflicting Approaches	58	58	33	33
26	Proportion LT, Opposing Approach	0.41	0.03	0.05	0.10
27	Proportion RT, Opposing Approach	0.01	0.21	0.09	0.51
28	Proportion LT, Conflicting Approaches	0.07	0.07	0.26	0.26
29	Proportion RT, Conflicting Approaches	0.24	0.24	0.09	0.09
30	Approach Capacity	540	741	378	487
31	Volume/Capacity Ratio	0.29	0.34	0.25	0.35
32	Delay	3	4	3	4
33	Level of Service	A	A	A	A
34	Average Delay (Intersection)	3			
35	Level of Service (Intersection)	A			

Existing A.M. Peak
Elk Grove-Florin Rd and Florin Rd

		EB	WB	NB	SB
1	LT Volume	98	48	127	8
2	TH Volume	276	310	431	218
3	RT Volume	106	12	64	91
4	Peak Hour Factor	0.90	0.90	0.90	0.90
5	Lanes on Subject Approach	2	2	1	1
6	Lanes on Opposing Approach	2	2	1	1
7	LT Flow Rate	108.89	53.333	141.11	8.8889
8	TH Flow Rate	306.67	344.44	478.89	242.22
9	RT Flow Rate	117.78	13.333	71.111	101.11
10	Approach Flow Rate	533.33	411.11	691.11	352.22
11	Proportion LT	0.20	0.13	0.20	0.03
12	Proportion RT	0.22	0.03	0.10	0.29
13	Opposing Approach (Direction)	WB	EB	SB	NB
14	Conflicting Approaches (Directions)	NB, SB	NB, SB	EB, WB	EB, WB
15	Subject Approach Flow Rate	533.33	411.11	691.11	352.22
16	Opposing Approach Flow Rate	370	480	317	622
17	Conflicting Approaches Flow Rate	939	939	850	850
18	Total Intersection Flow Rate	1842.3	1830.1	1858.1	1824.2
19	Proportion, Subject Approach Flow Rate	0.29	0.22	0.37	0.19
20	Proportion, Opposing Approach Flow Rate	0.20	0.26	0.17	0.34
21	Proportion, Conflicting Approaches Flow Rate	0.51	0.51	0.46	0.47
22	LT, Opposing Approach	48	98	8	127
23	RT, Opposing Approach	12	106	91	64
24	LT, Conflicting Approaches	135	135	146	146
25	RT, Conflicting Approaches	155	155	118	118
26	Proportion LT, Opposing Approach	0.13	0.20	0.03	0.20
27	Proportion RT, Opposing Approach	0.03	0.22	0.29	0.10
28	Proportion LT, Conflicting Approaches	0.14	0.14	0.17	0.17
29	Proportion RT, Conflicting Approaches	0.17	0.17	0.14	0.14
30	Approach Capacity	604	598	631	481
31	Volume/Capacity Ratio	0.88	0.69	1.09	0.73
32	Delay	29	14	64	16
33	Level of Service	D	C	F	C
34	Average Delay (Intersection)	36			
35	Level of Service (Intersection)	E			

Existing P.M. Peak
Elk Grove-Florin Rd and Florin Rd

		EB	WB	NB	SB
1	LT Volume	84	86	125	15
2	TH Volume	279	308	262	454
3	RT Volume	204	6	41	130
4	Peak Hour Factor	0.90	0.90	0.90	0.90
5	Lanes on Subject Approach	2	2	1	1
6	Lanes on Opposing Approach	2	2	1	1
7	LT Flow Rate	93.333	95.556	138.89	16.667
8	TH Flow Rate	310	342.22	291.11	504.44
9	RT Flow Rate	226.67	6.6667	45.556	144.44
10	Approach Flow Rate	630	444.44	475.56	665.56
11	Proportion LT	0.15	0.22	0.29	0.03
12	Proportion RT	0.36	0.02	0.10	0.22
13	Opposing Approach (Direction)	WB	EB	SB	NB
14	Conflicting Approaches (Directions)	NB, SB	NB, SB	EB, WB	EB, WB
15	Subject Approach Flow Rate	630	444.44	475.56	665.56
16	Opposing Approach Flow Rate	400	567	599	428
17	Conflicting Approaches Flow Rate	1027	1027	967	967
18	Total Intersection Flow Rate	2057	2038.4	2041.6	2060.6
19	Proportion, Subject Approach Flow Rate	0.31	0.22	0.23	0.32
20	Proportion, Opposing Approach Flow Rate	0.19	0.28	0.29	0.21
21	Proportion, Conflicting Approaches Flow Rate	0.50	0.50	0.47	0.47
22	LT, Opposing Approach	86	84	15	125
23	RT, Opposing Approach	6	204	130	41
24	LT, Conflicting Approaches	140	140	170	170
25	RT, Conflicting Approaches	171	171	210	210
26	Proportion LT, Opposing Approach	0.22	0.15	0.03	0.29
27	Proportion RT, Opposing Approach	0.02	0.36	0.22	0.10
28	Proportion LT, Conflicting Approaches	0.14	0.14	0.18	0.18
29	Proportion RT, Conflicting Approaches	0.17	0.17	0.22	0.22
30	Approach Capacity	590	649	587	512
31	Volume/Capacity Ratio	1.07	0.68	0.81	1.30
32	Delay	58	13	22	139
33	Level of Service	F	C	D	F
34	Average Delay (Intersection)	66			
35	Level of Service (Intersection)	F			

Existing A.M. Peak
Bradshaw and Calvine

		EB	WB	NB	SB
1	LT Volume	113	34	5	29
2	TH Volume	116	153	401	186
3	RT Volume	12	65	16	32
4	Peak Hour Factor	0.90	0.90	0.90	0.90
5	Lanes on Subject Approach	1	1	1	1
6	Lanes on Opposing Approach	1	1	1	1
7	LT Flow Rate	125.56	37.778	5.5556	32.222
8	TH Flow Rate	128.89	170	445.56	206.67
9	RT Flow Rate	13.333	72.222	17.778	35.556
10	Approach Flow Rate	267.78	280	468.89	274.44
11	Proportion LT	0.47	0.13	0.01	0.12
12	Proportion RT	0.05	0.26	0.04	0.13
13	Opposing Approach (Direction)	WB	EB	SB	NB
14	Conflicting Approaches (Directions)	NB, SB	NB, SB	EB, WB	EB, WB
15	Subject Approach Flow Rate	267.78	280	468.89	274.44
16	Opposing Approach Flow Rate	252	241	247	422
17	Conflicting Approaches Flow Rate	669	669	493	493
18	Total Intersection Flow Rate	1188.8	1190	1208.9	1189.4
19	Proportion, Subject Approach Flow Rate	0.23	0.24	0.39	0.23
20	Proportion, Opposing Approach Flow Rate	0.21	0.20	0.20	0.35
21	Proportion, Conflicting Approaches Flow Rate	0.56	0.56	0.41	0.41
22	LT, Opposing Approach	34	113	29	5
23	RT, Opposing Approach	65	12	32	16
24	LT, Conflicting Approaches	34	34	147	147
25	RT, Conflicting Approaches	48	48	77	77
26	Proportion LT, Opposing Approach	0.13	0.47	0.12	0.01
27	Proportion RT, Opposing Approach	0.26	0.05	0.13	0.04
28	Proportion LT, Conflicting Approaches	0.05	0.05	0.30	0.30
29	Proportion RT, Conflicting Approaches	0.07	0.07	0.16	0.16
30	Approach Capacity	491	353	579	541
31	Volume/Capacity Ratio	0.55	0.79	0.81	0.51
32	Delay	8	20	22	7
33	Level of Service	B	D	D	B
34	Average Delay (Intersection)	15			
35	Level of Service (Intersection)	C			

Existing P.M. Peak
Bradshaw and Calvine

		EB	WB	NB	SB
1	LT Volume	30	17	7	51
2	TH Volume	77	119	202	453
3	RT Volume	11	23	31	79
4	Peak Hour Factor	0.90	0.90	0.90	0.90
5	Lanes on Subject Approach	1	1	1	1
6	Lanes on Opposing Approach	1	1	1	1
7	LT Flow Rate	33.333	18.889	7.7778	56.667
8	TH Flow Rate	85.556	132.22	224.44	503.33
9	RT Flow Rate	12.222	25.556	34.444	87.778
10	Approach Flow Rate	131.11	176.67	266.67	647.78
11	Proportion LT	0.25	0.11	0.03	0.09
12	Proportion RT	0.09	0.14	0.13	0.14
13	Opposing Approach (Direction)	WB	EB	SB	NB
14	Conflicting Approaches (Directions)	NB, SB	NB, SB	EB, WB	EB, WB
15	Subject Approach Flow Rate	131.11	176.67	266.67	647.78
16	Opposing Approach Flow Rate	159	118	583	240
17	Conflicting Approaches Flow Rate	823	823	277	277
18	Total Intersection Flow Rate	1113.1	1117.7	1126.7	1164.8
19	Proportion, Subject Approach Flow Rate	0.12	0.16	0.24	0.56
20	Proportion, Opposing Approach Flow Rate	0.14	0.11	0.52	0.21
21	Proportion, Conflicting Approaches Flow Rate	0.74	0.74	0.25	0.24
22	LT, Opposing Approach	17	30	51	7
23	RT, Opposing Approach	23	11	79	31
24	LT, Conflicting Approaches	58	58	47	47
25	RT, Conflicting Approaches	110	110	34	34
26	Proportion LT, Opposing Approach	0.11	0.25	0.09	0.03
27	Proportion RT, Opposing Approach	0.14	0.09	0.14	0.13
28	Proportion LT, Conflicting Approaches	0.07	0.07	0.17	0.17
29	Proportion RT, Conflicting Approaches	0.13	0.13	0.12	0.12
30	Approach Capacity	334	293	686	803
31	Volume/Capacity Ratio	0.39	0.60	0.39	0.81
32	Delay	4	10	4	21
33	Level of Service	A	B	A	D
34	Average Delay (Intersection)	14			
35	Level of Service (Intersection)	C			

Existing A.M. Peak
Excelsior and Calvine

		EB	WB	NB	SB
1	LT Volume	27	7	34	5
2	TH Volume	102	86	199	33
3	RT Volume	30	23	11	6
4	Peak Hour Factor	0.90	0.90	0.90	0.90
5	Lanes on Subject Approach	1	1	1	1
6	Lanes on Opposing Approach	1	1	1	1
7	LT Flow Rate	30	7.7778	37.778	5.5556
8	TH Flow Rate	113.33	95.556	221.11	36.667
9	RT Flow Rate	33.333	25.556	12.222	6.6667
10	Approach Flow Rate	176.67	128.89	271.11	48.889
11	Proportion LT	0.17	0.06	0.14	0.11
12	Proportion RT	0.19	0.20	0.05	0.14
13	Opposing Approach (Direction)	WB	EB	SB	NB
14	Conflicting Approaches (Directions)	NB, SB	NB, SB	EB, WB	EB, WB
15	Subject Approach Flow Rate	176.67	128.89	271.11	48.889
16	Opposing Approach Flow Rate	116	159	44	244
17	Conflicting Approaches Flow Rate	288	288	275	275
18	Total Intersection Flow Rate	580.67	575.89	590.11	567.89
19	Proportion, Subject Approach Flow Rate	0.30	0.22	0.46	0.09
20	Proportion, Opposing Approach Flow Rate	0.20	0.28	0.07	0.43
21	Proportion, Conflicting Approaches Flow Rate	0.50	0.50	0.47	0.48
22	LT, Opposing Approach	7	27	5	34
23	RT, Opposing Approach	23	30	6	11
24	LT, Conflicting Approaches	39	39	34	34
25	RT, Conflicting Approaches	17	17	53	53
26	Proportion LT, Opposing Approach	0.06	0.17	0.11	0.14
27	Proportion RT, Opposing Approach	0.20	0.19	0.14	0.05
28	Proportion LT, Conflicting Approaches	0.14	0.14	0.12	0.12
29	Proportion RT, Conflicting Approaches	0.06	0.06	0.19	0.19
30	Approach Capacity	543	481	626	475
31	Volume/Capacity Ratio	0.33	0.27	0.43	0.10
32	Delay	3	3	5	1
33	Level of Service	A	A	B	A
34	Average Delay (Intersection)	4			
35	Level of Service (Intersection)	A			

Existing P.M. Peak
Excelsior and Calvine

		EB	WB	NB	SB
1	LT Volume	14	2	26	22
2	TH Volume	34	88	43	142
3	RT Volume	37	8	4	22
4	Peak Hour Factor	0.90	0.90	0.90	0.90
5	Lanes on Subject Approach	1	1	1	1
6	Lanes on Opposing Approach	1	1	1	1
7	LT Flow Rate	15.556	2.2222	28.889	24.444
8	TH Flow Rate	37.778	97.778	47.778	157.78
9	RT Flow Rate	41.111	8.8889	4.4444	24.444
10	Approach Flow Rate	94.444	108.89	81.111	206.67
11	Proportion LT	0.16	0.02	0.36	0.12
12	Proportion RT	0.44	0.08	0.05	0.12
13	Opposing Approach (Direction)	WB	EB	SB	NB
14	Conflicting Approaches (Directions)	NB, SB	NB, SB	EB, WB	EB, WB
15	Subject Approach Flow Rate	94.444	108.89	81.111	206.67
16	Opposing Approach Flow Rate	98	85	186	73
17	Conflicting Approaches Flow Rate	259	259	183	183
18	Total Intersection Flow Rate	451.44	452.89	450.11	462.67
19	Proportion, Subject Approach Flow Rate	0.21	0.24	0.18	0.45
20	Proportion, Opposing Approach Flow Rate	0.22	0.19	0.41	0.16
21	Proportion, Conflicting Approaches Flow Rate	0.57	0.57	0.41	0.40
22	LT, Opposing Approach	2	14	22	26
23	RT, Opposing Approach	8	37	22	4
24	LT, Conflicting Approaches	48	48	16	16
25	RT, Conflicting Approaches	26	26	45	45
26	Proportion LT, Opposing Approach	0.02	0.16	0.12	0.36
27	Proportion RT, Opposing Approach	0.08	0.44	0.12	0.05
28	Proportion LT, Conflicting Approaches	0.19	0.19	0.09	0.09
29	Proportion RT, Conflicting Approaches	0.10	0.10	0.25	0.25
30	Approach Capacity	446	484	605	609
31	Volume/Capacity Ratio	0.21	0.22	0.13	0.34
32	Delay	2	2	2	4
33	Level of Service	A	A	A	A
34	Average Delay (Intersection)	3			
35	Level of Service (Intersection)	A			

APPENDIX B

Trip Generation Tables - Preferred Plan

**Table B-1
NORTH VINEYARD SPECIFIC PLAN DAILY TRIP GENERATION**

Land Use	Quantity	Units	Daily Trip Rate	Daily Vehicle Trips	Internalization	Internal Trips	External Trips	Pass-By Factor	Net External Trips	External Person Trips	Transit Trips
Low Density Residential	5,259	D.U.	9.55	50,223	14%	7,031	43,192	0%	43,192	49,671	2,484
Medium Density Residential	442	D.U.	9.45	4,177	14%	585	3,592	0%	3,592	4,131	207
High Density Residential	637	D.U.	6.47	4,121	14%	577	3,544	0%	3,544	4,076	204
Neighborhood Commercial	49	ksf GLA	varies	6,555	40%	2,622	3,933	20%	3,146	3,618	181
Community Commercial	262	ksf GLA	varies	16,603	30%	4,981	11,622	30%	8,135	9,356	468
Professional/Offices	103	ksf GFA	14.37	1,480	15%	222	1,258	0%	1,258	1,447	72
Elementary School	600	Students	1.09	654	90%	589	65	0%	65	75	4
Neighborhood Park	64	Acres	2.23	143	90%	128	14	0%	14	16	1
Community Park	27	Acres	2.99	81	50%	40	40	0%	40	46	2
			Totals:	84,037	20%	16,775	67,262	N/A	62,989	72,437	3,622

Table B-2
NORTH VINEYARD SPECIFIC PLAN A.M. PEAK HOUR TRIP GENERATION

Land Use	Quantity	Units	A.M. Peak Trip Rate	Daily Vehicle Trips	Inter-nalization	Internal Trips	External Trips	Pass-By Factor	Net External Trips	External Person Trips	Transit Trips
Low Density Residential	5,259	D.U.	0.74	3,892	14%	545	3,347	0%	3,347	3,849	192
Medium Density Residential	442	D.U.	0.74	327	14%	46	281	0%	281	323	16
High Density Residential	637	D.U.	0.56	357	14%	50	307	0%	307	353	18
Neighborhood Commercial	49	kf GLA	varies	161	40%	64	97	20%	77	89	4
Community Commercial	262	kf GLA	varies	378	30%	113	265	30%	185	213	11
Professional/Offices	103	kf GFA	1.62	167	15%	25	142	0%	142	163	8
Elementary School	600	Students	0.28	168	90%	151	17	0%	17	19	1
Neighborhood Park	64	Acres	0.11	7	90%	6	1	0%	1	1	0
Community Park	27	Acres	0.15	4	50%	2	2	0%	2	2	0
			Totals:	5,460	18%	1,003	4,457	N/A	4,359	5,013	251

**Table B-3
NORTH VINEYARD SPECIFIC PLAN P.M. PEAK HOUR TRIP GENERATION**

Land Use	Quantity	Units	P.M. Peak Trip Rate	Daily Vehicle Trips	Inter-modalization	Internal Trips	External Trips	Pass-By Factor	Net External Trips	External Person Trips	Transit Trips
Low Density Residential	5,259	D.U.	1.01	5,312	14%	744	4,568	0%	4,568	5,253	263
Medium Density Residential	442	D.U.	1.01	446	14%	62	384	0%	384	442	22
High Density Residential	637	D.U.	0.69	440	14%	62	378	0%	378	435	22
Neighborhood Commercial	49	ksf GLA	varies	595	40%	238	357	20%	286	328	16
Community Commercial	262	ksf GLA	varies	1,546	30%	464	1,082	30%	758	871	44
Professional/Offices	103	ksf GFA	1.48	152	15%	23	130	0%	130	149	7
Elementary School	600	Students	0.25	150	90%	135	15	0%	15	17	1
Neighborhood Park	64	Acres	0.22	14	90%	13	1	0%	1	2	0
Community Park	27	Acres	0.30	8	50%	4	4	0%	4	5	0
Totals:				8,663	20%	1,744	6,919	N/A	6,523	7,502	375

APPENDIX C

Existing Plus Project Level of Service Calculations

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND S. WATT
Problem Statement: EXISTING PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: S. WATT		Approach				Possible Volume Adjusted			
1 1 ^		: -1- -2- -3- -4-				Critical Volume Critical			
R L N		a.No. of change				Prob- able Phase			
R T T T L		: 0 0 0 0				Volume to next phase			
T H H H T		b.LT capacity on				in vph			
Approach 1 << >> ^-RT 1		: 0 0 0 0				change (vph)			
1 LT--^ v v v <^-RTH		: 0 0 0 0				c.G/C ratio			
LTH--> <--TH 1		: 0 0 0 0				d.Opposing volume			
TH--> <v-LTH		: 0 0 0 0				in vph			
1 RTH-v> ^ ^ ^ v--LT 1		: 0 0 0 0				e.LT capacity on			
RT--v << >> Approach 2		: 0 0 0 0				green (vph)			
L L T R R		: 0 0 0 0				f.LT capacity in			
T T H T T JACKSON HWY		: 0 0 0 0				vph (b+e)			
H H		: 0 0 0 0				g.Left turn volume			
1 1		: 0 0 0 0				in vph			
Approach 4: S. WATT		: 0 0 0 0				h.Is volume > cap.			
		: (g>f) ?				:			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		1 8 2		99(B2B1)+552(A2)+311(B4B3)+1045(A3)	
3: LT= 214		8 5 1		= 2007 vph	
TH= 856		9 6 4			
RT= 189		+ +			
v		< v >		Step 8. INTERSECTION LEVEL OF SERVICE	
				(compare step 7 with table 6)	
				F	
Approach 1-->		55 ^			
		< ^ >			
		+ +		Step 9. RECALCULATE	
1:LT= 55		213 +>		Geometric Change:	
TH= 213		156 +v		Signal Change:	
RT= 156				Volume Change:	
Approach 4					
		3 9			
		1 2 4			
		1 2 4			

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS	
--^ v-- B2B1		Approach 3			
--^ AND <-- A1B2 AND					
--> OR v-- /OR A2B1					
--> <-- A1A2					
< B4B3		Approach 1			
>		See Step 6b.			
AND < ^ A3B4 AND		Approach 2			
v > OR /OR A4B3					
^ A3A4					
v					
A1 --> A3 B1 v-- B3 <		Approach 4		Exclusive right turns reduced 30 %	
A2 <-- A4 B2 --^ B4 >				V/C Ratio = 1.46	

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND S. WATT
Problem Statement: EXISTING PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: S. WATT JACKSON HWY R L N R T T T L T H H H T Approach 1 << >> ^-RT 1 1 LT--^ v v v <^-RTH LTH-^> <--TH 1 TH--> <v-LTH 1 RTH-v> ^ ^ ^ v--LT 1 RT--v << >> Approach 2 L L T R R JACKSON HWY T T H T T H H 1 1 Approach 4: S. WATT		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 76(B1) 113- 76= 37(B2) 76 A1B2 37(B2) 1061- 37=1024(A1) 37 A1A2 1024(A1) OR 355(A2) 1024 B4B3 303(B3) 399- 303= 96(B4) 303 A3B4 96(B4) 1186- 96=1090(A3) 96 A3A4 1090(A3) OR 1041(A4) 1090			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 399 Approach 3 TH=1104 2:RT= 281 RT= 82 v TH= 355 LT= 76 <--Approach 2 Approach 1-->		Step 5. ASSIGN LANE VOLUMES, in vph 1 1 3 ----- 8 0 9 2 4 9 + + ^- 281 < v > v- 76				Step 7. SUM OF CRITICAL VOLUMES 113(B1B2)+1024(A1)+399(B3B4)+1090(A3) = 2626 vph			
Approach 4 1:LT= 113 ^ 4: RT= 66 TH= 662 TH= 975 RT= 399 LT= 303 Approach 4		113 -^ < ^ > 662 +> + + 399 +v ----- 3 9 0 7 6 3 5 6				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.91			
A1 --> A3 v ^ B1 v-- B3 < A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND BRADSHAW
Problem Statement: EXISTING PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
JACKSON HWY Approach 3: BRADSHAW 1 1 1 ^ R L N R T T T L T H H H T Approach 1 << >> ^-RT 1 1 LT--^ v v v <^-RTH LTH-^> <--TH 1 1 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T JACKSON HWY H H 1 1 1 Approach 4: BRADSHAW		-----Approach----- : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 0 0 d.Opposing volume in vph : 0 0 0 0 e.LT capacity on green (vph) : 0 0 0 0 f.LT capacity in vph (b+e) : 0 0 0 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? :				Prob- able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph B2B1 15(B1) 200- 15= 185(B2) 15 A1B2 185(B2) 180- 185= 0(A1) 185 A1A2 423(A2) OR 0(A1) 423 B4B3 74(B4) 340- 74= 266(B3) 74 A4B3 266(B3) 1149- 266= 883(A4) 266 A3A4 883(A4) OR 569(A3) 883			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
3: LT= 74 TH= 569 RT= 226 Approach 3 2: RT= 187 TH= 423 LT= 15 <--Approach 2		2 5 2 6 7 6 9 4 < v >		200(B1B2)+423(A2)+340(B4B3)+883(A4) = 1846 vph	
Approach 1--> 1: LT= 200 TH= 180 RT= 181 Approach 4 4: RT= 14 TH=1149 LT= 340		200 -^ 180 -> 181 -v < ^ > 1 3 1 4 4 1 0 9 4		Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F	
				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				Exclusive right turns reduced 30 % V/C Ratio = 1.34

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND BRADSHAW
Problem Statement: EXISTING PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW 1 1 1 ^ R L N JACKSON HWY R T T L T H H H T Approach 1 << >> ^-RT 1 1 LT--^ v v v <^-RTH LTH-^> <--TH 1 1 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T JACKSON HWY H H 1 1 1 Approach 4: BRADSHAW		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Volume Critical in vph to next Volume phase phase in vph B2B1 28(B1) 191- 28= 163(B2) 28 A1B2 163(B2) 433- 163= 270(A1) 163 A1A2 270(A1) OR 156(A2) 270 B4B3 165(B4) 317- 165= 152(B3) 165 A4B3 152(B3) 790- 152= 638(A4) 152 A3A4 1234(A3) OR 638(A4) 1234			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 165 Approach 3 2:RT= 76 TH=1234 TH= 156 RT= 182 v LT= 28 <--Approach 2 Approach 1--> 1:LT= 191 ^ 4: RT= 20 TH= 433 TH= 790 RT= 413 LT= 317 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 1 1 2 1 8 3 6 2 4 5 < v > 191 -^ < ^ > 433 -> 413 -v 3 7 1 9 2 7 0 0				Step 7. SUM OF CRITICAL VOLUMES 191(B1B2)+270(A1)+317(B4B3)+1234(A3) = 2012 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.46			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: BRADSHAW RD AND ELDER CREEK RD
Problem Statement: EXISTING PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW 1 1 1 ^ R L N R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 0 0 d.Opposing volume in vph : 0 0 0 0 e.LT capacity on green (vph) : 0 0 0 0 f.LT capacity in vph (b+e) : 0 0 0 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? :				Possible Volume Adjusted Prob- Critical Volume Critical able Volume to next Volume Phase in vph phase in vph ----- A1B2 89(A1) OR 45(B2) 89 A2B1 151(A2) OR 16(B1) 151 B4B3 6(B4) 140- 6= 134(B3) 6 A4B3 134(B3) 1473- 134=1339(A4) 134 A3A4 1339(A4) OR 663(A3) 1339			
Approach 1 << >> ^-RT 1 1 LT--^ v v v <^RTH LTH--> <--TH TH--> <v-LTH 1 1 RTH--v ^ ^ ^ v--LT RT--v << >> Approach 2 L L T R R ELDER CREEK R T T H T T H H 1 1 Approach 4:BRADSHAW									

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3 3: LT= 6 2:RT= 8 TH= 663 TH= 135 RT= 136 v LT= 17 <--Approach 2		1 6 3 6 6 3 6 < v >		89(A1)+151(A2)+140(B4B3)+1339(A4) = 1719 vph	
Approach 1--> 1:LT= 45 ^ 4: RT= 31 TH= 36 TH=1442 RT= 53 LT= 140 Approach 4		45 -^ < ^ > 36 +> + + 53 +v 1 1 4 4 4 3 0 2 1		Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F	
				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ A1B2 --> <-- A2B1 v-- < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: BRADSHAW RD AND ELDER CREEK RD
Problem Statement: EXISTING PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW 1 1 1 ^ R L N ELDER CREEK R R T T T L T H H H T Approach 1 << >> ^-RT 1 1 LT--^ v v v <^-RTH LTH-^> <--TH TH--> <v-LTH 1 1 RTH-v> ^ ^ ^ v--LT RT--v << >> Approach 2 L L T R R T T H T T ELDER CREEK R H H 1 1 Approach 4:BRADSHAW		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph A1B2 232(A1) OR 130(B2) 232 A2B1 84(A2) OR 44(B1) 84 B4B3 14(B4) 94- 14= 80(B3) 14 A4B3 80(B3) 989- 80= 909(A4) 80 A3A4 1626(A3) OR 909(A4) 1626			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 14 Approach 3 2:RT= 7 TH=1626 TH= 40 RT= 68 v LT= 44 <--Approach 2 Approach 1--> 1:LT= 130 ^ 4: RT= 35 TH= 97 TH= 954 RT= 135 LT= 94 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 1 6 ----- 6 2 1 8 6 4 < v > 130 -^ < ^ > 97 +> + + 135 +v ----- 9 9 5 3 4 4 5				Step 7. SUM OF CRITICAL VOLUMES 232(A1)+84(A2)+94(B4B3)+1626(A3) = 2036 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- F ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ A1B2 --> <-- A2B1 v-- < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.48			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: BRADSHAW RD AND FLORIN RD
Problem Statement: EXISTING PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD 1 1 1 ^ R L N R T T L T H H H T Approach 1 << >> ^-RT 1 LT--^ v v v <^-RTH 1 LTH-^> <--TH TH--> <v-LTH 1 RTH-v^ ^ ^ ^ v--LT 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 1 1 Approach 4: BRADSHAW		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 85(B1) 195- 85= 110(B2) 85 A1B2 110(B2) 392- 110= 282(A1) 110 A1A2 509(A2) OR 282(A1) 509 B4B3 55(B4) 402- 55= 347(B3) 55 A4B3 347(B3) 1452- 347=1105(A4) 347 A3A4 1105(A4) OR 582(A3) 1105			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 55 Approach 3 2:RT= 147 TH= 582 TH= 362 RT= 172 v LT= 85 <--Approach 2 Approach 1--> 1:LT= 195 ^ 4: RT= 108 TH= 178 TH=1344 RT= 214 LT= 402 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 1 5 ----- 7 8 5 ----- 2 2 5 ^+ 147 <+ 362 < v > v- 85				Step 7. SUM OF CRITICAL VOLUMES 195(B1B2)+509(A2)+402(B4B3)+1105(A4) = 2211 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- F ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.61			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: BRADSHAW RD AND FLORIN RD
Problem Statement: EXISTING PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW RD 1 1 1 ^ R L N FLORIN RD R T T T L T H H H T Approach 1 < < > > ^--RT 1 LT--^ v v v <^--RTH 1 LTH--^> <--TH TH--> <v--LTH 1 RTH-v> ^ ^ ^ v--LT 1 RT--v < < > > Approach 2 L L T R R T T H T T FLORIN RD H H 1 1 Approach 4:BRADSHAW		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- Possible Volume Adjusted able Critical Carryover Critical Phase Volume to next Volume in vph phase in vph B2B1 122(B1) 228- 122= 106(B2) 122 A1B2 106(B2) 858- 106= 752(A1) 106 A1A2 752(A1) OR 371(A2) 752 B4B3 157(B4) 335- 157= 178(B3) 157 A4B3 178(B3) 883- 178= 705(A4) 178 A3A4 1465(A3) OR 705(A4) 1465			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 157 Approach 3 2:RT= 100 TH=1465 TH= 271 RT= 272 v LT= 122 <--Approach 2 Approach 1-->		Step 5. ASSIGN LANE VOLUMES, in vph 1 2 4 1 7 6 5 2 5 7 < v > 228 -^ < ^ > 360 +> + + 498 +v 3 7 1 3 7 1 5 2 1				Step 7. SUM OF CRITICAL VOLUMES 228(B1B2)+752(A1)+335(B4B3)+1465(A3) = 2780 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 2.02			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELK GROVE-FLORIN AND GERBER RD
Problem Statement: EXISTING PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE-FLO		-----Approach-----				Possible Volume Adjusted			
1 1 ^		: -1- -2- -3- -4-				Critical Carryover Critical			
R L N		a.No. of change				Prob- able Volume to next phase			
R T T T L		: 0 0 0 0				in vph			
T H H H T		b.LT capacity on				in vph			
-----		change (vph)				-----			
Approach 1 < < > > ^--RT		: 0 0 0 0				B2B1 100(B2) 196- 100= 96(B1) 100			
1 LT--^ v v v <^--RTH 1		: 0 0 0 0				A2B1 96(B1) 453- 96= 357(A2) 96			
LTH--^ <--TH 1		: 0 0 0 0				A1A2 384(A1) OR 357(A2) 384			
1 TH--> <v--LTH		: 0 0 0 0				B4B3 234(B4) 752- 234= 518(B3) 234			
RTH--v ^ ^ ^ v--LT 1		: 0 0 0 0				A4B3 518(B3) 881- 518= 363(A4) 518			
1 RT--v < < > > Approach 2		: 0 0 0 0				A3A4 532(A3) OR 363(A4) 532			
-----		f.LT capacity in				-----			
L L T R R		vph (b+e)				-----			
T T H T T GERBER RD		: 0 0 0 0				-----			
H H		g.Left turn volume				-----			
1 1		: 0 0 0 0				-----			
Approach 4: ELK GROVE-FLO		h.Is volume > cap.				-----			
-----		: (g>f) ?				-----			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		1 3 2		196(B2B1)+384(A1)+752(B4B3)+532(A3)	
3: LT= 234		8 4 3		= 1864 vph	
TH= 345		7 5 4		-----	
RT= 187		+ +		Step 8. INTERSECTION LEVEL OF SERVICE	
v		< v >		(compare step 7 with table 6)	
-----		^+ 442		-----	
<--Approach 2		<+ 11		-----	
-----		<- 453		-----	
Approach 1-->		v- 196		-----	
-----		-----		-----	
1:LT= 100		100 -^		-----	
TH= 384		384 ->		-----	
RT= 269		269 -v		-----	
Approach 4		-----		-----	
-----		7 7 1		-----	
4: RT= 166		5 1 6		-----	
TH= 715		2 5 6		-----	
LT= 752		-----		-----	
-----		-----		-----	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1		Approach 3		
--^ AND <-- A1B2 AND		-----		
--> OR v-- /OR A2B1		-----		
--> <-- A1A2		-----		
< B4B3		Approach 1		
>		-----		
AND < ^ A3B4 AND		See Step 6b.		
v > OR /OR A4B3		-----		
^ A3A4		Approach 2		
v		-----		
-----		-----		
A1 --> A3 B1 v-- B3 <		Approach 4		Exclusive right turns reduced 30 % V/C Ratio = 1.36
A2 <-- A4 B2 --^ B4 >		-----		

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELK GROVE-FLORIN AND GERBER RD
Problem Statement: EXISTING PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP				
Approach 3: ELK GROVE-FLO 1 1 ^ R L N RT TT L TH HT Approach 1 << >> ^--RT 1 LT--^ v v v <^--RTH 1 LTH--> <--TH 1 1 TH--> <v--LTH RTH--v ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 1 1 Approach 4: ELK GROVE-FLO		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 214(B2) 277- 214= 63(B1) 214 A2B1 63(B1) 505- 63= 442(A2) 63 A1A2 585(A1) OR 442(A2) 585 B4B3 336(B3) 605- 336= 269(B4) 336 A3B4 269(B4) 924- 269= 655(A3) 269 A3A4 707(A4) OR 655(A3) 707				
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES				
3: LT= 605 Approach 3 2:RT= 489 TH= 714 TH= 520 RT= 210 v LT= 277 <--Approach 2 Approach 1-->		2 7 6 1 1 0 0 4 5 + + ^+ 489 < v > <+ 16 v- 277				277(B2B1)+585(A1)+605(B3B4)+707(A4) = 2174 vph				
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)				
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		214 ^ < ^ > 535 -> + + 835 -v 3 4 2 3 6 4 6 0 7				F				
Step 9. RECALCULATE		Step 9. RECALCULATE				Geometric Change: Signal Change: Volume Change:				
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS				
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				Exclusive right turns reduced 30 % V/C Ratio = 1.58				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: BRADSHAW AND GERBER RD
Problem Statement: EXISTING PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD 1 1 1 ^ R L N ERBER RD R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Volume Critical Volume to next Volume in vph phase in vph ----- B2B1 29(B1) 431- 29= 402(B2) 29 A1B2 402(B2) 254- 402= 0(A1) 402 A1A2 397(A2) OR 0(A1) 397 B4B3 71(B3) 92- 71= 21(B4) 71 A3B4 21(B4) 311- 21= 290(A3) 21 A3A4 819(A4) OR 290(A3) 819			
Approach 1 << >> ^-RT 1 LT--^ v v v <^-RTH 1 LTH-^> <--TH TH--> <v-LTH 1 RTH-v> ^ ^ ^ v--LT 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 1 1 Approach 4: BRADSHAW RD									
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 92 Approach 3 TH= 311 2:RT= 145 RT= 368 v TH= 252 LT= 29 <--Approach 2 Approach 1--> 1:LT= 431 ^ 4: RT= 31 TH= 187 TH= 788 RT= 67 LT= 71 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 3 3 ----- 6 1 9 ----- 8 1 2 ^+ 145 <+ 252 < v > v- 29				Step 7. SUM OF CRITICAL VOLUMES 431(B1B2)+397(A2)+92(B3B4)+819(A4) = 1739 vph ----- Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.26			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: BRADSHAW AND GERBER RD
Problem Statement: EXISTING PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD 1 1 1 ^ R L N R T T T L T H H H T Approach 1 << >> ^--RT 1 LT--^ v v v <^--RTH 1 LTH--^> <--TH TH--> <v--LTH 1 RTH--v> ^ ^ ^ v--LT 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 1 1 Approach 4: BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 37(B1) 585- 37= 548(B2) 37 A1B2 548(B2) 424- 548= 0(A1) 548 A1A2 468(A2) OR 0(A1) 468 B4B3 101(B3) 194- 101= 93(B4) 101 A3B4 93(B4) 726- 93= 633(A3) 93 A3A4 633(A3) OR 325(A4) 633			
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
3: LT= 194 Approach 3 2:RT= 180 TH= 726 TH= 288 RT= 659 v LT= 37 <--Approach 2 Approach 1-->		6 7 1 5 2 9 9 6 4 < v > 585 -^ < ^ > 321 +> + + 103 +v 4: RT= 35 TH= 290 LT= 101 Approach 4				585(B1B2)+468(A2)+194(B3B4)+633(A3) = 1880 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				Exclusive right turns reduced 30 % V/C Ratio = 1.37			
Step 9. RECALCULATE		Geometric Change: Signal Change: Volume Change:							

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELK GROVE-FLORIN AND CALVINE RD
Problem Statement: EXISTING PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE-FLO 1 1 1 ^ R L N R T T T L T H H H T Approach 1 << >> ^-RT 2 LT--^ v v v <^-RTH 1 LTH-^> <--TH TH--> <v-LTH 1 RTH-v> ^ ^ ^ v--LT 2 RT--v << >> Approach 2 L L T R R T T H T T CALVINE ROAD H H 1 1 1 Approach 4: ELK GROVE-FLO		-----Approach----- : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 0 0 d.Opposing volume in vph : 0 0 0 0 e.LT capacity on green (vph) : 0 0 0 0 f.LT capacity in vph (b+e) : 0 0 0 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? :				Prob- able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph B2B1 66(B1) 69- 66= 3(B2) 66 A1B2 3(B2) 238- 3= 235(A1) 3 A1A2 262(A2) OR 235(A1) 262 B4B3 27(B4) 122- 27= 95(B3) 27 A4B3 95(B3) 315- 95= 220(A4) 95 A3A4 433(A3) OR 220(A4) 433			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3 3: LT= 27 2:RT= 43 TH= 519 TH= 219 RT= 346 v LT= 119 <--Approach 2		3 4 4 8 3 2 6 7 3 7 + + < v v >		69(B1B2)+262(A2)+122(B4B3)+433(A3) = 886 vph	
Approach 1--> 1:LT= 125 ^ 4: RT= 79 TH= 140 TH= 551 RT= 98 LT= 122 Approach 4		69 -^ 57 -^ < ^ ^ > 140 +> + + 98 +v 1 3 2 2 1 3 7 2 5 6 9		Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) B	
				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				V/C Ratio = .64

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELK GROVE-FLORIN AND CALVINE RD
Problem Statement: EXISTING PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE-FLO CALVINE ROAD R T T T L T H H H T Approach 1 2 LT--^ LTH--> TH--> 1 RTH-v RT--v L L T R R T T H T T H H 1 1 1 Approach 4: ELK GROVE-FLO		Approach : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 0 0 d.Opposing volume in vph : 0 0 0 0 e.LT capacity on green (vph) : 0 0 0 0 f.LT capacity in vph (b+e) : 0 0 0 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? :				Possible Critical Volume in vph Probable Phase B2B1 69(B1) A1B2 107(B2) A1A2 223(A1) OR 202(A2) B4B3 49(B4) A4B3 59(B3) A3A4 491(A3) OR 262(A4)			
		Volume Adjustments: 176- 69= 107(B2) 330- 107= 223(A1) 108- 49= 59(B3) 321- 59= 262(A4)				Adjusted Critical Volume in vph 107 223 49 59 491			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
3: LT= 49 TH= 741 RT= 241 Approach 3 2: RT= 27 TH= 175 LT= 125 Approach 2 Approach 1--> 1: LT= 320 TH= 199 RT= 131 Approach 4		2 2 4 4 5 9 4 1 0 1 9 + + + < v v > ^+ 27 <+ 175 v- 57 v- 69		176(B1B2)+223(A1)+108(B4B3)+491(A3) = 998 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) C	
		176 -^ 144 -^ 199 +> 131 +v 1 3 2 1 0 2 2 0 8 1 0 1		Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND WATERMAN RD
Problem Statement: EXISTING PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: WATERMAN RD LORIN RD R L ^ R T T T L N T H H H T Approach 1 << >> ^--RT LT--^ v v v <^--RTH LTH-^> <--TH 1 1 TH--> <v--LTH RTH-v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 1 1 Approach 4: WATERMAN RD		-----Approach----- : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 1 0 d.Opposing volume in vph : 703 0 154 0 e.LT capacity on green (vph) : 0 0 1046 0 f.LT capacity in vph (b+e) : 0 0 1046 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? : NO NO				Prob- able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph A2B1 86(B1) 703- 86= 617(A2) 86 A1A2 617(A2) OR 274(A1) 617 A4B3 371(B3) OR 108(A4) 371			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 0 TH= 0 RT= 0 Approach 3 2:RT= 0 TH= 703 LT= 86 v <--Approach 2 Approach 1--> 1:LT= 0 TH= 274 RT= 225 Approach 4 4: RT= 154 TH= 0 LT= 371		Step 5. ASSIGN LANE VOLUMES, in vph <- 703 v- 86 <> 3 1 7 5 1 4				Step 7. SUM OF CRITICAL VOLUMES 703(B1A2)+371(B3)+0()+0() = 1074 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) C Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING <-- A2B1 v-- --> <-- A1A2 < ^ A4B3 		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .75			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND WATERMAN RD
Problem Statement: EXISTING PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY	Step 4. LEFT TURN CHECK	Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP																																																																																																					
<p>FLORIN RD</p> <p>Approach 3: WATERMAN RD</p> <table style="width:100%; border-collapse: collapse;"> <tr> <td style="width:50%; border-right: 1px solid black;"> <table style="width:100%; border-collapse: collapse;"> <tr> <td style="width:50%; text-align: center;">R L</td> <td style="width:50%; text-align: center;">N</td> </tr> <tr> <td style="text-align: center;">R T T T L</td> <td style="text-align: center;">^</td> </tr> <tr> <td style="text-align: center;">T H H H T</td> <td style="text-align: center;">^</td> </tr> <tr> <td style="text-align: center;"><< >></td> <td style="text-align: center;">^--RT</td> </tr> <tr> <td style="text-align: center;">LT--^</td> <td style="text-align: center;">v v v <^--RTH</td> </tr> <tr> <td style="text-align: center;">LTH--^</td> <td style="text-align: center;"><--TH 1</td> </tr> <tr> <td style="text-align: center;">1 TH--></td> <td style="text-align: center;"><v-LTH</td> </tr> <tr> <td style="text-align: center;">RTH--v</td> <td style="text-align: center;">^ ^ ^ v--LT 1</td> </tr> <tr> <td style="text-align: center;">1 RT--v</td> <td style="text-align: center;"><< >> Approach 2</td> </tr> <tr> <td style="border-right: 1px solid black;"> <table style="width:100%; border-collapse: collapse;"> <tr> <td style="width:50%; text-align: center;">L L T R R</td> <td style="width:50%; text-align: center;">FLORIN RD</td> </tr> <tr> <td style="text-align: center;">T T H T T</td> <td style="text-align: center;">H H</td> </tr> <tr> <td style="text-align: center;">1 1</td> <td style="text-align: center;">1 1</td> </tr> </table> </td> <td style="border-right: 1px solid black;"> <p>Approach 4: WATERMAN RD</p> </td> </tr> </table> </td> <td> <table style="width:100%; border-collapse: collapse;"> <tr> <td style="width:50%;"></td> <td style="width:50%; text-align: center;">-----Approach-----</td> </tr> <tr> <td style="width:50%;"></td> <td style="width:50%; text-align: center;">: -1- -2- -3- -4-</td> </tr> <tr> <td style="width:50%;">a.No. of change intervals/hour</td> <td style="width:50%;">: 0 0 0 0</td> </tr> <tr> <td>b.LT capacity on change (vph)</td> <td style="width:50%;">: 0 0 0 0</td> </tr> <tr> <td>c.G/C ratio</td> <td style="width:50%;">: 0 0 1 0</td> </tr> <tr> <td>d.Opposing volume in vph</td> <td style="width:50%;">: 658 0 123 0</td> </tr> <tr> <td>e.LT capacity on green (vph)</td> <td style="width:50%;">: 0 0 1077 0</td> </tr> <tr> <td>f.LT capacity in vph (b+e)</td> <td style="width:50%;">: 0 0 1077 0</td> </tr> <tr> <td>g.Left turn volume in vph</td> <td style="width:50%;">: 0 0 0 0</td> </tr> <tr> <td>h.Is volume > cap. (g>f) ?</td> <td style="width:50%;">: NO NO</td> </tr> </table> </td> <td> <table style="width:100%; border-collapse: collapse;"> <tr> <td style="width:25%;"></td> <td style="width:25%; text-align: center;">Possible Critical Volume in vph</td> <td style="width:25%;"></td> <td style="width:25%; text-align: center;">Volume Adjusted Critical Volume in vph</td> </tr> <tr> <td>Prob-able Phase</td> <td style="width:25%;">A2B1</td> <td style="width:25%;">148(B1) 658- 148= 510(A2)</td> <td style="width:25%;">148</td> </tr> <tr> <td></td> <td>A1A2</td> <td>740(A1) OR 510(A2)</td> <td>740</td> </tr> <tr> <td></td> <td>A4B3</td> <td>425(B3) OR 86(A4)</td> <td>425</td> </tr> </table> </td> </tr> <tr> <td> <p>Step 2. IDENTIFY VOLUMES, in vph</p> <table style="width:100%; border-collapse: collapse;"> <tr> <td style="width:50%; border-right: 1px solid black;"> <p>3: LT= 0</p> <p>TH= 0</p> <p>RT= 0</p> </td> <td style="width:50%;"> <p>Approach 3</p> <p>2:RT= 0</p> <p>TH= 658</p> <p>LT= 148</p> </td> </tr> <tr> <td style="border-right: 1px solid black;"></td> <td style="text-align: center;"><--Approach 2</td> </tr> <tr> <td style="border-right: 1px solid black;"> <p>Approach 1--></p> <p>1:LT= 0</p> <p>TH= 740</p> <p>RT= 515</p> </td> <td style="border-right: 1px solid black;"> <p>Approach 4</p> <p>4: RT= 123</p> <p>TH= 0</p> <p>LT= 425</p> </td> </tr> </table> </td> <td> <p>Step 5. ASSIGN LANE VOLUMES, in vph</p> <table style="width:100%; border-collapse: collapse;"> <tr> <td style="width:50%; border-right: 1px solid black;"></td> <td style="width:50%;"></td> </tr> <tr> <td style="border-right: 1px solid black;"></td> <td style="text-align: center;"><- 658</td> </tr> <tr> <td style="border-right: 1px solid black;"></td> <td style="text-align: center;">v- 148</td> </tr> <tr> <td style="border-right: 1px solid black;"></td> <td style="text-align: center;">< ></td> </tr> <tr> <td style="border-right: 1px solid black;"></td> <td style="text-align: center;"> </td> </tr> <tr> <td style="border-right: 1px solid black;"></td> <td style="text-align: center;">4 1</td> </tr> <tr> <td style="border-right: 1px solid black;"></td> <td style="text-align: center;">2 2</td> </tr> <tr> <td style="border-right: 1px solid black;"></td> <td style="text-align: center;">5 3</td> </tr> </table> </td> <td> <p>Step 7. SUM OF CRITICAL VOLUMES</p> <p>888(B1A1)+425(B3)+0()+0()</p> <p>= 1313 vph</p> <p>Step 8. INTERSECTION LEVEL OF SERVICE</p> <p>(compare step 7 with table 6)</p> <p style="text-align: center;"> E </p> <p>Step 9. RECALCULATE</p> <p>Geometric Change:</p> <p>Signal Change:</p> <p>Volume Change:</p> </td> </tr> <tr> <td> <p>Step 3. IDENTIFY PHASING</p> <p><-- A2B1</p> <p>v--</p> <p>--> <-- A1A2</p> <p>< ^ A4B3</p> <p> </p> </td> <td> <p>Step 6a. 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<p>Step 3. IDENTIFY PHASING</p> <p><-- A2B1</p> <p>v--</p> <p>--> <-- A1A2</p> <p>< ^ A4B3</p> <p> </p>	<p>Step 6a. CRITICAL VOLUMES, in vph (two phase signal)</p> <table style="width:100%; border-collapse: collapse;"> <tr> <td style="width:50%; border-right: 1px solid black;"></td> <td style="width:50%; text-align: center;">Approach 3</td> </tr> <tr> <td style="border-right: 1px solid black;"></td> <td style="text-align: center;">Approach 1</td> </tr> <tr> <td style="border-right: 1px solid black;"></td> <td style="text-align: center;">See Step 6b.</td> </tr> <tr> <td style="border-right: 1px solid black;"></td> <td style="text-align: center;">Approach 2</td> </tr> <tr> <td style="border-right: 1px solid black;"></td> <td style="text-align: center;">Approach 4</td> </tr> </table>		Approach 3		Approach 1		See Step 6b.		Approach 2		Approach 4	<p>COMMENTS</p> <p>Exclusive right turns reduced 30 %</p> <p>V/C Ratio = .92</p>																																																																																											
	Approach 3																																																																																																						
	Approach 1																																																																																																						
	See Step 6b.																																																																																																						
	Approach 2																																																																																																						
	Approach 4																																																																																																						

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND WATERMAN RD
Problem Statement: EXISTING PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: WATERMAN RD 1 1 ^ R L N R T T T L T H H H T Approach 1 << >> ^-RT 1 1 LT--^ v v v <^-RTH LTH-^> <--TH 1 1 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H Approach 4: WATERMAN RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : D 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 323 0 259 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : NO NO (g>f) ? :				Prob- Possible Volume Adjusted able Critical Carryover Critical Phase Volume to next Volume in vph phase in vph ----- A1B2 207(B2) 323- 207= 116(A1) 207 A1A2 553(A2) OR 116(A1) 553 A3B4 181(A3) OR 113(B4) 181			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 113 TH= 0 RT= 259 Approach 3 2:RT= 99 TH= 553 LT= 0 v <--Approach 2 Approach 1-->		Step 5. ASSIGN LANE VOLUMES, in vph 2 1 5 1 9 3 <>				Step 7. SUM OF CRITICAL VOLUMES 760(B2A2)+181(A3)+0()+0() = 941 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) B			
1:LT= 207 TH= 323 RT= 0 Approach 4		4: RT= 0 TH= 0 LT= 0 207 -^ 323 ->				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ A1B2 --> --> <-- A1A2 A3B4 v >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .66			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

1985 HCM: UNSIGNALIZED INTERSECTIONS

Page-1

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45
 PEAK HOUR FACTOR..... .9
 AREA POPULATION..... 20000
 NAME OF THE EAST/WEST STREET..... JACKSON HWY
 NAME OF THE NORTH/SOUTH STREET..... EXCELSIOR RD
 NAME OF THE ANALYST..... FP
 DATE OF THE ANALYSIS (mm/dd/yy)..... 02-22-1996
 TIME PERIOD ANALYZED..... AM
 OTHER INFORMATION.... EXISTING PLUS PROJECT

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: 4-LEG
 MAJOR STREET DIRECTION: EAST/WEST
 CONTROL TYPE NORTHBOUND: STOP SIGN
 CONTROL TYPE SOUTHBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	25	71	113	3
THRU	152	453	74	44
RIGHT	22	15	75	9

NUMBER OF LANES AND LANE USAGE

	EB	WB	NB	SB
LANES	2	2	1	1

LANE USAGE

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	0	0	
WESTBOUND	0.00	0	0	
NORTHBOUND	0.00	90	100	
SOUTHBOUND	0.00	90	100	

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
NB	6.10	5.60	0.00	5.60
SB	6.10	5.60	0.00	5.60
MAJOR LEFTS				
EB	5.80	5.80	0.00	5.80
WB	5.80	5.80	0.00	5.80
MINOR THROUGHGS				
NB	7.40	7.40	0.00	7.40
SB	7.40	7.40	0.00	7.40
MINOR LEFTS				
NB	7.90	7.90	0.00	7.90
SB	7.90	7.90	0.00	7.90

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... JACKSON HWY
NAME OF THE NORTH/SOUTH STREET..... EXCELSIOR RD
DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
OTHER INFORMATION.... EXISTING PLUS PROJECT

CAPACITY AND LEVEL-OF-SERVICE

MOVEMENT	FLOW-RATE	POTENTIAL CAPACITY	ACTUAL MOVEMENT CAPACITY	SHARED CAPACITY	RESERVE CAPACITY	LOS
	v (pcph)	c (pcph) p	c (pcph) M	c (pcph) SH	c = c - v R SH	

MINOR STREET

NB LEFT	138	187	141 >	141 >	3 >	E
THROUGH	90	238	216 >	215 >	-105 >	F D
RIGHT	92	976	976 >	976 >	884 >	A

MINOR STREET

SB LEFT	4	148	87 >	87 >	84 >	E
THROUGH	54	237	215 >	224 >	155 >	D D
RIGHT	11	813	813 >	813 >	802 >	A

MAJOR STREET

EB LEFT	31	567	567	567	536	A
WB LEFT	87	840	840	840	753	A

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... JACKSON HWY
 NAME OF THE NORTH/SOUTH STREET.... EXCELSIOR RD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
 OTHER INFORMATION.... EXISTING PLUS PROJECT

RC = 117 ⇒ LOS D

1985 HCM: UNSIGNALIZED INTERSECTIONS

Page-1

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45
 PEAK HOUR FACTOR..... .9
 AREA POPULATION..... 20000
 NAME OF THE EAST/WEST STREET..... JACKSON HWY
 NAME OF THE NORTH/SOUTH STREET..... EXCELSIOR RD
 NAME OF THE ANALYST..... FP
 DATE OF THE ANALYSIS (mm/dd/yy)..... 02-22-1996
 TIME PERIOD ANALYZED..... PM
 OTHER INFORMATION.... EXISTING PLUS PROJECT

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: 4-LEG
 MAJOR STREET DIRECTION: EAST/WEST
 CONTROL TYPE NORTHBOUND: STOP SIGN
 CONTROL TYPE SOUTHBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	10	72	19	4
THRU	428	209	58	80
RIGHT	86	11	96	9

NUMBER OF LANES AND LANE USAGE

	EB	WB	NB	SB
LANES	2	2	1	1

LANE USAGE

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	0	0	
WESTBOUND	0.00	0	0	
NORTHBOUND	0.00	90	100	
SOUTHBOUND	0.00	90	100	

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
NB	6.10	5.60	0.00	5.60
SB	6.10	5.60	0.00	5.60
MAJOR LEFTS				
EB	5.80	5.80	0.00	5.80
WB	5.80	5.80	0.00	5.80
MINOR THROUGHGS				
NB	7.40	7.40	0.00	7.40
SB	7.40	7.40	0.00	7.40
MINOR LEFTS				
NB	7.90	7.90	0.00	7.90
SB	7.90	7.90	0.00	7.90

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... JACKSON HWY
NAME OF THE NORTH/SOUTH STREET.... EXCELSIOR RD
DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; PM
OTHER INFORMATION.... EXISTING PLUS PROJECT

CAPACITY AND LEVEL-OF-SERVICE

MOVEMENT	FLOW-RATE	POTENTIAL CAPACITY	ACTUAL MOVEMENT CAPACITY	SHARED CAPACITY	RESERVE CAPACITY		LOS
	v(pcph)	c (pcph) p	c (pcph) M	c (pcph) SH	c = c - v R	- v SH	

MINOR STREET

NB LEFT	23	155	81 >	81 >	58 >	E
THROUGH	71	219	192 >	263 192 >	52 122 >	E D
RIGHT	117	788	788 >	788 >	671 >	A

MINOR STREET

SB LEFT	5	124	73 >	73 >	68 >	E
THROUGH	98	203	179 >	182 179 >	68 81 >	E E
RIGHT	11	952	952 >	952 >	941 >	A

MAJOR STREET

EB LEFT	12	793	793	793	780	A
WB LEFT	88	531	531	531	443	A

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... JACKSON HWY
 NAME OF THE NORTH/SOUTH STREET.... EXCELSIOR RD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; PM
 OTHER INFORMATION.... EXISTING PLUS PROJECT

Rc = 158 => LOS D

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45

PEAK HOUR FACTOR..... .9

AREA POPULATION..... 20000

NAME OF THE EAST/WEST STREET..... ELDER CREEK RD

NAME OF THE NORTH/SOUTH STREET..... EXCELSIOR RD

NAME OF THE ANALYST..... FP

DATE OF THE ANALYSIS (mm/dd/yy)..... 02-22-1996

TIME PERIOD ANALYZED..... AM

OTHER INFORMATION.... EXISTING PLUS PROJECT

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: T-INTERSECTION

MAJOR STREET DIRECTION: NORTH/SOUTH

CONTROL TYPE EASTBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	32	--	66	43
THRU	197	--	238	85
RIGHT	6	--	10	67

NUMBER OF LANES

	EB	WB	NB	SB
LANES	1	--	1	2

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	0	0	
WESTBOUND	----	---	---	-
NORTHBOUND	0.00	90	100	
SOUTHBOUND	0.00	90	100	

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	---	---	---
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
EB	6.10	5.60	0.00	5.60
MAJOR LEFTS				
NB	5.80	5.80	0.00	5.80
MINOR LEFTS				
EB	7.90	7.90	0.00	7.90

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... ELDER CREEK RD
NAME OF THE NORTH/SOUTH STREET.... EXCELSIOR RD
DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
OTHER INFORMATION.... EXISTING PLUS PROJECT

CAPACITY AND LEVEL-OF-SERVICE

Page-3

MOVEMENT	FLOW-RATE	POTENTIAL CAPACITY	ACTUAL MOVEMENT CAPACITY	SHARED CAPACITY	RESERVE CAPACITY	LOS
	v(pcph)	c (pcph) p	c (pcph) M	c (pcph) SH	c = c - v R SH	

MINOR STREET

EB LEFT	39	381	360	> 360	> 321	B
				> 400	> 353	>B
RIGHT	7	979	979	> 979	> 972	A

MAJOR STREET

NB LEFT	81	865	865	865	784	A
---------	----	-----	-----	-----	-----	---

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... ELDER CREEK RD
 NAME OF THE NORTH/SOUTH STREET..... EXCELSIOR RD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
 OTHER INFORMATION..... EXISTING PLUS PROJECT

RC=628 → LOS A

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45
 PEAK HOUR FACTOR..... .9
 AREA POPULATION..... 20000
 NAME OF THE EAST/WEST STREET..... ELDER CREEK RD
 NAME OF THE NORTH/SOUTH STREET..... EXCELSIOR RD
 NAME OF THE ANALYST..... FP
 DATE OF THE ANALYSIS (mm/dd/yy)..... 02-22-1996
 TIME PERIOD ANALYZED..... PM
 OTHER INFORMATION.... EXISTING PLUS PROJECT

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: T-INTERSECTION
 MAJOR STREET DIRECTION: NORTH/SOUTH
 CONTROL TYPE EASTBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	55	--	13	43
THRU	197	--	116	219
RIGHT	37	--	10	23

NUMBER OF LANES

	EB	WB	NB	SB
LANES	1	--	1	2

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	0	0	
WESTBOUND	----	---	---	-
NORTHBOUND	0.00	90	100	
SOUTHBOUND	0.00	90	100	

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	---	---	---
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
EB	6.10	5.60	0.00	5.60
MAJOR LEFTS				
NB	5.80	5.80	0.00	5.80
MINOR LEFTS				
EB	7.90	7.90	0.00	7.90

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... ELDER CREEK RD
NAME OF THE NORTH/SOUTH STREET.... EXCELSIOR RD
DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; PM
OTHER INFORMATION.... EXISTING PLUS PROJECT

CAPACITY AND LEVEL-OF-SERVICE

MOVEMENT	FLOW-RATE v(pcph)	POTEN-TIAL	ACTUAL	SHARED		RESERVE		LOS
		CAPACITY c (pcph) p	MOVEMENT CAPACITY c (pcph) M	CAPACITY c (pcph) SH	CAPACITY c (pcph)	c = c - v R SH		

MINOR STREET

EB LEFT	67	423	418	>	418	>	351	> B
				>	538	>	426	>A
RIGHT	45	939	939	>	939	>	894	> A

MAJOR STREET

NB LEFT	16	770	770		770		754	A
---------	----	-----	-----	--	-----	--	-----	---

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... ELDER CREEK RD
 NAME OF THE NORTH/SOUTH STREET.... EXCELSIOR RD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; PM
 OTHER INFORMATION.... EXISTING PLUS PROJECT

RC = 467 ⇒ LOS A

1985 HCM: UNSIGNALIZED INTERSECTIONS

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45
 PEAK HOUR FACTOR..... .9
 AREA POPULATION..... 20000
 NAME OF THE EAST/WEST STREET..... GERBER RD
 NAME OF THE NORTH/SOUTH STREET..... VINEYARD
 NAME OF THE ANALYST..... FP
 DATE OF THE ANALYSIS (mm/dd/yy)..... 02-22-1996
 TIME PERIOD ANALYZED..... AM
 OTHER INFORMATION.... EXISTING PLUS PROJECT

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: 4-LEG
 MAJOR STREET DIRECTION: EAST/WEST
 CONTROL TYPE NORTHBOUND: STOP SIGN
 CONTROL TYPE SOUTHBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	25	2	64	21
THRU	178	108	33	17
RIGHT	51	9	3	43

NUMBER OF LANES AND LANE USAGE

	EB	WB	NB	SB
LANES	2	2	2	2
LANE USAGE			L + TR	L + TR

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	0	0	
WESTBOUND	0.00	0	0	
NORTHBOUND	0.00	90	100	
SOUTHBOUND	0.00	90	100	

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
NB	6.10	5.60	0.00	5.60
SB	6.10	5.60	0.00	5.60
MAJOR LEFTS				
EB	5.80	5.80	0.00	5.80
WB	5.80	5.80	0.00	5.80
MINOR THROUGHGS				
NB	7.40	7.40	0.00	7.40
SB	7.40	7.40	0.00	7.40
MINOR LEFTS				
NB	7.90	7.90	0.00	7.90
SB	7.90	7.90	0.00	7.90

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... GERBER RD
NAME OF THE NORTH/SOUTH STREET.... VINEYARD
DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
OTHER INFORMATION.... EXISTING PLUS PROJECT

CAPACITY AND LEVEL-OF-SERVICE

MOVEMENT	FLOW-RATE	POTENTIAL CAPACITY	ACTUAL MOVEMENT CAPACITY	SHARED CAPACITY	RESERVE CAPACITY	LOS
	v (pcph)	c (pcph)	c (pcph)	c (pcph)	c = c - v	
	p	p	m	sh	r sh	

MINOR STREET

NB LEFT	78	391	360	360	282	C
THROUGH	40	475	464	> 464	> 424	A
RIGHT	4	947	947	> 485	> 947	>A A

MINOR STREET

SB LEFT	26	393	364	364	338	B
THROUGH	21	458	448	> 448	> 427	A
RIGHT	53	984	984	> 735	> 984	>A A

MAJOR STREET

EB LEFT	31	904	904	904	874	A
WB LEFT	2	783	783	783	781	A

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... GERBER RD
 NAME OF THE NORTH/SOUTH STREET..... VINEYARD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
 OTHER INFORMATION.... EXISTING PLUS PROJECT

RC = 501 => LOS A

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45
 PEAK HOUR FACTOR..... .9
 AREA POPULATION..... 20000
 NAME OF THE EAST/WEST STREET..... GERBER RD
 NAME OF THE NORTH/SOUTH STREET..... VINEYARD
 NAME OF THE ANALYST..... FP
 DATE OF THE ANALYSIS (mm/dd/yy)..... 02-22-1996
 TIME PERIOD ANALYZED..... PM
 OTHER INFORMATION.... EXISTING PLUS PROJECT

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: 4-LEG
 MAJOR STREET DIRECTION: EAST/WEST
 CONTROL TYPE NORTHBOUND: STOP SIGN
 CONTROL TYPE SOUTHBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	57	2	82	15
THRU	150	193	23	30
RIGHT	82	23	3	41

NUMBER OF LANES AND LANE USAGE

	EB	WB	NB	SB
LANES	2	2	2	2
LANE USAGE			L + TR	L + TR

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	0	0	
WESTBOUND	0.00	0	0	
NORTHBOUND	0.00	90	100	
SOUTHBOUND	0.00	90	100	

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
NB	6.10	5.60	0.00	5.60
SB	6.10	5.60	0.00	5.60
MAJOR LEFTS				
EB	5.80	5.80	0.00	5.80
WB	5.80	5.80	0.00	5.80
MINOR THROUGHGS				
NB	7.40	7.40	0.00	7.40
SB	7.40	7.40	0.00	7.40
MINOR LEFTS				
NB	7.90	7.90	0.00	7.90
SB	7.90	7.90	0.00	7.90

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... GERBER RD
NAME OF THE NORTH/SOUTH STREET.... VINEYARD
DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; PM
OTHER INFORMATION.... EXISTING PLUS PROJECT

CAPACITY AND LEVEL-OF-SERVICE

MOVEMENT	FLOW-RATE	POTENTIAL CAPACITY	ACTUAL MOVEMENT CAPACITY	SHARED CAPACITY	RESERVE CAPACITY	LOS
	v (pcph)	c (pcph) p	c (pcph) M	c (pcph) SH	c = c - v R SH	

MINOR STREET

NB LEFT	100	304	262	262	162	D
THROUGH	28	390	369	> 369	> 341	> B
RIGHT	4	945	945	> 397 945	> 365 941	>B A

MINOR STREET

SB LEFT	18	314	284	284	266	C
THROUGH	37	369	349	> 349	> 312	> B
RIGHT	50	954	954	> 551 954	> 464 904	>A A

MAJOR STREET

EB LEFT	70	797	797	797	727	A
WB LEFT	2	780	780	780	778	A

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... GERBER RD
 NAME OF THE NORTH/SOUTH STREET.... VINEYARD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; PM
 OTHER INFORMATION.... EXISTING PLUS PROJECT

RC=406 ⇒ LOS A

1985 HCM: UNSIGNALIZED INTERSECTIONS

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45
 PEAK HOUR FACTOR..... .9
 AREA POPULATION..... 20000
 NAME OF THE EAST/WEST STREET..... FLORIN RD
 NAME OF THE NORTH/SOUTH STREET..... VINEYARD RD
 NAME OF THE ANALYST..... FP
 DATE OF THE ANALYSIS (mm/dd/yy)..... 02-22-1996
 TIME PERIOD ANALYZED..... AM
 OTHER INFORMATION.... EXISTING PLUS PROJECT

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: T-INTERSECTION
 MAJOR STREET DIRECTION: EAST/WEST
 CONTROL TYPE NORTHBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	36	12	124	--
THRU	292	470	9	--
RIGHT	49	6	27	--

NUMBER OF LANES

	EB	WB	NB	SB
LANES	2	2	2	--

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	0	0	
WESTBOUND	0.00	0	0	
NORTHBOUND	0.00	90	100	
SOUTHBOUND	----	---	---	-

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	0	0	0
SOUTHBOUND	---	---	---

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
NB	6.10	5.60	0.00	5.60
MAJOR LEFTS				
WB	5.80	5.80	0.00	5.80
MINOR LEFTS				
NB	7.90	7.90	0.00	7.90

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... FLORIN RD
 NAME OF THE NORTH/SOUTH STREET.... VINEYARD RD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
 OTHER INFORMATION.... EXISTING PLUS PROJECT

CAPACITY AND LEVEL-OF-SERVICE

MOVEMENT	FLOW-RATE	POTENTIAL CAPACITY	ACTUAL MOVEMENT CAPACITY	SHARED CAPACITY	RESERVE CAPACITY		LOS
	v (pcph)	c (pcph)	c (pcph)	c (pcph)	c = c - v	R SH	

MINOR STREET

NB LEFT	152	180	178	178	26	E
RIGHT	33	882	882	882	849	A

MAJOR STREET

WB LEFT	15	671	671	671	656	A
---------	----	-----	-----	-----	-----	---

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... FLORIN RD
 NAME OF THE NORTH/SOUTH STREET..... VINEYARD RD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
 OTHER INFORMATION..... EXISTING PLUS PROJECT

RC = 209 ⇒ LOS C

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45
 PEAK HOUR FACTOR..... .9
 AREA POPULATION..... 20000
 NAME OF THE EAST/WEST STREET..... FLORIN RD
 NAME OF THE NORTH/SOUTH STREET..... VINEYARD RD
 NAME OF THE ANALYST..... FP
 DATE OF THE ANALYSIS (mm/dd/yy)..... 02-22-1996
 TIME PERIOD ANALYZED..... PM
 OTHER INFORMATION.... EXISTING PLUS PROJECT

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: T-INTERSECTION
 MAJOR STREET DIRECTION: EAST/WEST
 CONTROL TYPE NORTHBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	36	34	85	--
THRU	489	408	9	--
RIGHT	139	6	23	--

NUMBER OF LANES

	EB	WB	NB	SB
LANES	2	2	2	--

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	0	0	
WESTBOUND	0.00	0	0	
NORTHBOUND	0.00	90	100	
SOUTHBOUND	----	---	---	-

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	0	0	0
SOUTHBOUND	---	---	---

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
NB	6.10	5.60	0.00	5.60
MAJOR LEFTS				
WB	5.80	5.80	0.00	5.80
MINOR LEFTS				
NB	7.90	7.90	0.00	7.90

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... FLORIN RD
NAME OF THE NORTH/SOUTH STREET.... VINEYARD RD
DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; PM
OTHER INFORMATION.... EXISTING PLUS PROJECT

CAPACITY AND LEVEL-OF-SERVICE

MOVEMENT	FLOW-RATE v(pcph)	POTEN-	ACTUAL	SHARED	RESERVE	LOS
		TIAL	MOVEMENT	CAPACITY	CAPACITY	
		c (pcph)	c (pcph)	c (pcph)	c = c - v	
		p	M	SH	R SH	

MINOR STREET

NB LEFT	104	115	109	109	5	E
RIGHT	28	730	730	730	702	A

MAJOR STREET

WB LEFT	42	451	451	451	409	A
---------	----	-----	-----	-----	-----	---

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... FLORIN RD
 NAME OF THE NORTH/SOUTH STREET..... VINEYARD RD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; PM
 OTHER INFORMATION.... EXISTING PLUS PROJECT

RC = 215 ⇒ LOS C

1985 HCM: UNSIGNALIZED INTERSECTIONS

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45
 PEAK HOUR FACTOR..... .8
 AREA POPULATION..... 150000
 NAME OF THE EAST/WEST STREET..... GERBER RD
 NAME OF THE NORTH/SOUTH STREET..... EXCELSIOR RD
 NAME OF THE ANALYST..... F & P
 DATE OF THE ANALYSIS (mm/dd/yy)..... 05-17-1995
 TIME PERIOD ANALYZED..... AM

OTHER INFORMATION.... EXISTING PLUS PROJECT

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: 4-LEG
 MAJOR STREET DIRECTION: NORTH/SOUTH
 CONTROL TYPE EASTBOUND: STOP SIGN
 CONTROL TYPE WESTBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	164	1	40	0
THRU	0	4	222	27
RIGHT	36	0	2	72

NUMBER OF LANES AND LANE USAGE

	EB	WB	NB	SB
LANES	1	1	1	1
LANE USAGE	LTR	LTR		

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	90	20	N
WESTBOUND	0.00	90	20	N
NORTHBOUND	0.00	90	20	N
SOUTHBOUND	0.00	90	20	N

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
EB	6.10	6.10	0.00	6.10
WB	6.10	6.10	0.00	6.10
MAJOR LEFTS				
SB	5.30	5.30	0.00	5.30
NB	5.30	5.30	0.00	5.30
MINOR THROUGHGS				
EB	6.90	6.90	0.00	6.90
WB	6.90	6.90	0.00	6.90
MINOR LEFTS				
EB	7.40	7.40	0.00	7.40
WB	7.40	7.40	0.00	7.40

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... GERBER RD
NAME OF THE NORTH/SOUTH STREET.... EXCELSIOR RD
DATE AND TIME OF THE ANALYSIS..... 05-17-1995 ; AM
OTHER INFORMATION.... EXISTING PLUS PROJECT

CAPACITY AND LEVEL-OF-SERVICE

MOVEMENT	FLOW-RATE v(pcph)	POTEN-	ACTUAL	SHARED		RESERVE		LOS
		TIAL CAPACITY c (pcph) p	MOVEMENT CAPACITY c (pcph) M	CAPACITY c (pcph) SH	CAPACITY c (pcph) SH	c = c - v R	v SH	

MINOR STREET

EB LEFT	226	455	437	>	437	>	211	>	C
THROUGH	0	506	489	>	482	489	>	207	489 > C A
RIGHT	50	902	902	>	902	>	853	>	A

MINOR STREET

WB LEFT	1	403	376	>	376	>	375	>	B
THROUGH	6	478	462	>	442	462	>	435	456 > A A
RIGHT	0	713	713	>	713	>	713	>	A

MAJOR STREET

SB LEFT	0	850	850		850		850		A
NB LEFT	55	981	981		981		926		A

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... GERBER RD
 NAME OF THE NORTH/SOUTH STREET.... EXCELSIOR RD
 DATE AND TIME OF THE ANALYSIS..... 05-17-1995 ; AM
 OTHER INFORMATION.... EXISTING PLUS PROJECT

EC = 329 ⇒ LOS B

1985 HCM: UNSIGNALIZED INTERSECTIONS

Page-1

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45
 PEAK HOUR FACTOR..... .85
 AREA POPULATION..... 150000
 NAME OF THE EAST/WEST STREET..... GERBER RD
 NAME OF THE NORTH/SOUTH STREET..... EXCELSIOR RD
 NAME OF THE ANALYST..... F & P
 DATE OF THE ANALYSIS (mm/dd/yy)..... 05-17-1995
 TIME PERIOD ANALYZED..... PM
 OTHER INFORMATION.... EXISTING PLUS PROJECT

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: 4-LEG
 MAJOR STREET DIRECTION: NORTH/SOUTH
 CONTROL TYPE EASTBOUND: STOP SIGN
 CONTROL TYPE WESTBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	90	1	50	0
THRU	1	2	32	185
RIGHT	64	0	2	157

NUMBER OF LANES AND LANE USAGE

	EB	WB	NB	SB
LANES	1	1	1	1
LANE USAGE	LTR	LTR		

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	90	20	N
WESTBOUND	0.00	90	20	N
NORTHBOUND	0.00	90	20	N
SOUTHBOUND	0.00	90	20	N

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
EB	6.10	6.10	0.00	6.10
WB	6.10	6.10	0.00	6.10
MAJOR LEFTS				
SB	5.30	5.30	0.00	5.30
NB	5.30	5.30	0.00	5.30
MINOR THROUGHGS				
EB	6.90	6.90	0.00	6.90
WB	6.90	6.90	0.00	6.90
MINOR LEFTS				
EB	7.40	7.40	0.00	7.40
WB	7.40	7.40	0.00	7.40

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... GERBER RD
NAME OF THE NORTH/SOUTH STREET.... EXCELSIOR RD
DATE AND TIME OF THE ANALYSIS..... 05-17-1995 ; PM
OTHER INFORMATION.... EXISTING PLUS PROJECT

CAPACITY AND LEVEL-OF-SERVICE

Page-3

MOVEMENT	FLOW-RATE v(pcph)	POTEN-TIAL	ACTUAL	SHARED		RESERVE		LOS
		CAPACITY c (pcph) p	MOVEMENT CAPACITY c (pcph) M	CAPACITY c (pcph) SH	CAPACITY c (pcph) R	CAPACITY c (pcph) SH	- v	

MINOR STREET

EB LEFT	116	457	431	>	431	>	315	> B
THROUGH	1	506	480	>	510	480	>	309 478 >B A
RIGHT	83	686	686	>	686	>	603	> A

MINOR STREET

WB LEFT	1	353	308	>	308	>	307	> B
THROUGH	3	447	423	>	376	423	>	372 420 >B A
RIGHT	0	944	944	>	944	>	944	> A

MAJOR STREET

SB LEFT	0	999	999		999		999	A
NB LEFT	65	736	736		736		671	A

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... GERBER RD
 NAME OF THE NORTH/SOUTH STREET.... EXCELSIOR RD
 DATE AND TIME OF THE ANALYSIS..... 05-17-1995 ; PM
 OTHER INFORMATION.... EXISTING PLUS PROJECT

RC = 397 ⇒ LOS B

1985 HCM: UNSIGNALIZED INTERSECTIONS

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45
 PEAK HOUR FACTOR..... .9
 AREA POPULATION..... 20000
 NAME OF THE EAST/WEST STREET..... CALVINE RD
 NAME OF THE NORTH/SOUTH STREET..... WATERMAN RD
 NAME OF THE ANALYST..... FP
 DATE OF THE ANALYSIS (mm/dd/yy)..... 02-22-1996
 TIME PERIOD ANALYZED..... AM

OTHER INFORMATION.... EXISTING PLUS PROJECT

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: 4-LEG
 MAJOR STREET DIRECTION: EAST/WEST
 CONTROL TYPE NORTHBOUND: STOP SIGN
 CONTROL TYPE SOUTHBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	18	29	7	6
THRU	241	264	2	6
RIGHT	37	5	20	42

NUMBER OF LANES AND LANE USAGE

	EB	WB	NB	SB
LANES	1	1	1	2
LANE USAGE				LT + R

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	0	0	
WESTBOUND	0.00	0	0	
NORTHBOUND	0.00	90	100	
SOUTHBOUND	0.00	90	100	

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
NB	6.10	5.60	0.00	5.60
SB	6.10	5.60	0.00	5.60
MAJOR LEFTS				
EB	5.30	5.30	0.00	5.30
WB	5.30	5.30	0.00	5.30
MINOR THROUGHGS				
NB	6.90	6.90	0.00	6.90
SB	6.90	6.90	0.00	6.90
MINOR LEFTS				
NB	7.40	7.40	0.00	7.40
SB	7.40	7.40	0.00	7.40

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... CALVINE RD
NAME OF THE NORTH/SOUTH STREET.... WATERMAN RD
DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
OTHER INFORMATION.... EXISTING PLUS PROJECT

CAPACITY AND LEVEL-OF-SERVICE

Page-3

MOVEMENT	FLOW-RATE	POTENTIAL CAPACITY	ACTUAL MOVEMENT CAPACITY	SHARED CAPACITY	RESERVE CAPACITY		LOS
	v(pcph)	c (pcph) p	c (pcph) M	c (pcph) SH	c = c - v R	v SH	

MINOR STREET

NB LEFT	9	293	266	>	266	>	258	>	C
THROUGH	2	360	345	>	504	345	>	469	343 >A B
RIGHT	24	785	785	>	785	>	761	>	A

MINOR STREET

SB LEFT	7	298	280	>	305	280	>	290	272 >C C
THROUGH	7	350	336	>	336	>	329	>	B
RIGHT	51	778	778		778		726		A

MAJOR STREET

EB LEFT	22	831	831		831		809		A
WB LEFT	35	822	822		822		786		A

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... CALVINE RD
 NAME OF THE NORTH/SOUTH STREET.... WATERMAN RD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
 OTHER INFORMATION.... EXISTING PLUS PROJECT

RC=671 ⇒ LOS A

1985 HCM: UNSIGNALIZED INTERSECTIONS

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45

PEAK HOUR FACTOR..... .9

AREA POPULATION..... 20000

NAME OF THE EAST/WEST STREET..... CALVINE RD

NAME OF THE NORTH/SOUTH STREET..... WATERMAN RD

NAME OF THE ANALYST..... FP

DATE OF THE ANALYSIS (mm/dd/yy)..... 02-22-1996

TIME PERIOD ANALYZED..... PM

OTHER INFORMATION.... EXISTING PLUS PROJECT

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: 4-LEG

MAJOR STREET DIRECTION: EAST/WEST

CONTROL TYPE NORTHBOUND: STOP SIGN

CONTROL TYPE SOUTHBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	36	32	4	1
THRU	120	261	9	1
RIGHT	78	6	41	18

NUMBER OF LANES AND LANE USAGE

	EB	WB	NB	SB
LANES	1	1	1	2
LANE USAGE				LT + R

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	0	0	
WESTBOUND	0.00	0	0	
NORTHBOUND	0.00	90	100	
SOUTHBOUND	0.00	90	100	

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
NB	6.10	5.60	0.00	5.60
SB	6.10	5.60	0.00	5.60
MAJOR LEFTS				
EB	5.30	5.30	0.00	5.30
WB	5.30	5.30	0.00	5.30
MINOR THROUGHGS				
NB	6.90	6.90	0.00	6.90
SB	6.90	6.90	0.00	6.90
MINOR LEFTS				
NB	7.40	7.40	0.00	7.40
SB	7.40	7.40	0.00	7.40

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... CALVINE RD
NAME OF THE NORTH/SOUTH STREET..... WATERMAN RD
DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; PM
OTHER INFORMATION.... EXISTING PLUS PROJECT

CAPACITY AND LEVEL-OF-SERVICE

MOVEMENT	FLOW-RATE v(pcph)	POTEN-	ACTUAL	SHARED		RESERVE		LOS
		TIAL CAPACITY c (pcph) p	MOVEMENT CAPACITY c (pcph) M	CAPACITY c (pcph) SH	CAPACITY c (pcph)	CAPACITY c = c - v R	CAPACITY SH	

MINOR STREET

NB LEFT	5	357	330	>	330	>	325	> B
THROUGH	11	414	391	>	667	391	>	601 380 >A B
RIGHT	50	895	895	>	895	>	845	> A

MINOR STREET

SB LEFT	1	316	284	>	320	284	>	317 283 >B C
THROUGH	1	388	366	>	366	>	365	> B
RIGHT	22	780	780		780		758	A

MAJOR STREET

EB LEFT	44	833	833		833		789	A
WB LEFT	39	908	908		908		869	A

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... CALVINE RD
 NAME OF THE NORTH/SOUTH STREET.... WATERMAN RD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; PM
 OTHER INFORMATION.... EXISTING PLUS PROJECT

RC = 737 = LOS A

1985 HCM: UNSIGNALIZED INTERSECTIONS

Page-1

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45

PEAK HOUR FACTOR..... .9

AREA POPULATION..... 20000

NAME OF THE EAST/WEST STREET..... CALVINE RD

NAME OF THE NORTH/SOUTH STREET..... VINEYARD

NAME OF THE ANALYST..... FP

DATE OF THE ANALYSIS (mm/dd/yy)..... 02-22-1996

TIME PERIOD ANALYZED..... AM

OTHER INFORMATION.... EXISTING PLUS PROJECT

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: T-INTERSECTION

MAJOR STREET DIRECTION: EAST/WEST

CONTROL TYPE SOUTHBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	22	4	--	31
THRU	87	146	--	1
RIGHT	4	21	--	56

NUMBER OF LANES

	EB	WB	NB	SB
LANES	1	2	--	1

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	0	0	
WESTBOUND	0.00	0	0	
NORTHBOUND	----	---	---	
SOUTHBOUND	0.00	90	100	

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	---	---	---
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
SB	6.10	5.60	0.00	5.60
MAJOR LEFTS				
EB	5.80	5.80	0.00	5.80
MINOR LEFTS				
SB	7.90	7.90	0.00	7.90

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... CALVINE RD
NAME OF THE NORTH/SOUTH STREET.... VINEYARD
DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
OTHER INFORMATION.... EXISTING PLUS PROJECT

CAPACITY AND LEVEL-OF-SERVICE

MOVEMENT	FLOW-RATE	POTENTIAL CAPACITY	ACTUAL MOVEMENT CAPACITY	SHARED CAPACITY	RESERVE CAPACITY	LOS
	v (pcph)	c (pcph) p	c (pcph) M	c (pcph) SH	c = c - v R SH	

MINOR STREET

SB LEFT	38	502	492	> 492	> 455	> A
				> 723	> 617	> A
RIGHT	68	977	977	> 977	> 908	> A

MAJOR STREET

EB LEFT	27	848	848	848	821	A
---------	----	-----	-----	-----	-----	---

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... CALVINE RD
 NAME OF THE NORTH/SOUTH STREET..... VINEYARD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; AM
 OTHER INFORMATION.... EXISTING PLUS PROJECT

RC = 6:58 → LOS A

IDENTIFYING INFORMATION

AVERAGE RUNNING SPEED, MAJOR STREET.. 45

PEAK HOUR FACTOR..... .9

AREA POPULATION..... 20000

NAME OF THE EAST/WEST STREET..... CALVINE RD

NAME OF THE NORTH/SOUTH STREET..... VINEYARD

NAME OF THE ANALYST..... FP

DATE OF THE ANALYSIS (mm/dd/yy)..... 02-22-1996

TIME PERIOD ANALYZED..... PM

OTHER INFORMATION.... EXISTING PLUS PROJECT

INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: T-INTERSECTION

MAJOR STREET DIRECTION: EAST/WEST

CONTROL TYPE SOUTHBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	41	4	--	36
THRU	109	133	--	1
RIGHT	4	40	--	160

NUMBER OF LANES

	EB	WB	NB	SB
LANES	1	2	--	1

ADJUSTMENT FACTORS

Page-2

	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	0	0	
WESTBOUND	0.00	0	0	
NORTHBOUND	----	---	---	-
SOUTHBOUND	0.00	90	100	

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND	0	0	0
NORTHBOUND	---	---	---
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR VALUES (Table 10-2)	ADJUSTED VALUE	SIGHT DIST. ADJUSTMENT	FINAL CRITICAL GAP
MINOR RIGHTS				
SB	6.10	5.60	0.00	5.60
MAJOR LEFTS				
EB	5.80	5.80	0.00	5.80
MINOR LEFTS				
SB	7.90	7.90	0.00	7.90

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... CALVINE RD
NAME OF THE NORTH/SOUTH STREET.... VINEYARD
DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; PM
OTHER INFORMATION.... EXISTING PLUS PROJECT

CAPACITY AND LEVEL-OF-SERVICE

MOVEMENT	FLOW-RATE	POTENTIAL CAPACITY	ACTUAL MOVEMENT CAPACITY	SHARED CAPACITY	RESERVE CAPACITY	LOS
	v(pcph)	c (pcph) p	c (pcph) M	c (pcph) SH	c = c - v R SH	

MINOR STREET

SB LEFT	44	470	454	> 806	454 > 566	410 > A
RIGHT	196	976	976	> 976	> 780	> A

MAJOR STREET

EB LEFT	50	841	841	841	791	A
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IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... CALVINE RD
 NAME OF THE NORTH/SOUTH STREET.... VINEYARD
 DATE AND TIME OF THE ANALYSIS..... 02-22-1996 ; PM
 OTHER INFORMATION.... EXISTING PLUS PROJECT

RC = 605 → LOS A

Existing Plus Project A.M. Peak
Elder Creek Rd. and S. Watt Ave.

		EB	WB	NB	SB
1	LT Volume	69	47	127	2
2	TH Volume	103	203	1063	536
3	RT Volume	60	40	66	112
4	Peak Hour Factor	0.90	0.90	0.90	0.90
5	Lanes on Subject Approach	1	2	1	1
6	Lanes on Opposing Approach	2	1	1	1
7	LT Flow Rate	76.667	52.222	141.11	2.2222
8	TH Flow Rate	114.44	225.56	1181.1	595.56
9	RT Flow Rate	66.667	44.444	73.333	124.44
10	Approach Flow Rate	257.78	322.22	1395.6	722.22
11	Proportion LT	0.30	0.16	0.10	0.00
12	Proportion RT	0.26	0.14	0.05	0.17
13	Opposing Approach (Direction)	WB	EB	SB	NB
14	Conflicting Approaches (Directions)	NB, SB	NB, SB	EB, WB	EB, WB
15	Subject Approach Flow Rate	257.78	322.22	1395.6	722.22
16	Opposing Approach Flow Rate	290	232	650	1256
17	Conflicting Approaches Flow Rate	1906	1906	522	522
18	Total Intersection Flow Rate	2453.8	2460.2	2567.6	2500.2
19	Proportion, Subject Approach Flow Rate	0.11	0.13	0.54	0.29
20	Proportion, Opposing Approach Flow Rate	0.12	0.09	0.25	0.50
21	Proportion, Conflicting Approaches Flow Rate	0.78	0.77	0.20	0.21
22	LT, Opposing Approach	47	69	2	127
23	RT, Opposing Approach	40	60	112	66
24	LT, Conflicting Approaches	129	129	116	116
25	RT, Conflicting Approaches	178	178	100	100
26	Proportion LT, Opposing Approach	0.16	0.30	0.00	0.10
27	Proportion RT, Opposing Approach	0.14	0.26	0.17	0.05
28	Proportion LT, Conflicting Approaches	0.07	0.07	0.22	0.22
29	Proportion RT, Conflicting Approaches	0.09	0.09	0.19	0.19
30	Approach Capacity	174	467	845	711
31	Volume/Capacity Ratio	1.48	0.69	1.65	1.02
32	Delay	274	14	531	47
33	Level of Service	F	C	F	F
34	Average Delay (Intersection)	315			
35	Level of Service (Intersection)	F			

Existing Plus Project P.M. Peak
Elder Creek Rd. and S. Watt Ave.

		EB	WB	NB	SB
1	LT Volume	98	99	105	43
2	TH Volume	197	130	873	1209
3	RT Volume	150	24	68	72
4	Peak Hour Factor	0.90	0.90	0.90	0.90
5	Lanes on Subject Approach	1	2	1	1
6	Lanes on Opposing Approach	2	1	1	1
7	LT Flow Rate	108.89	110	116.67	47.778
8	TH Flow Rate	218.89	144.44	970	1343.3
9	RT Flow Rate	166.67	26.667	75.556	80
10	Approach Flow Rate	494.44	281.11	1162.2	1471.1
11	Proportion LT	0.22	0.39	0.10	0.03
12	Proportion RT	0.34	0.09	0.07	0.05
13	Opposing Approach (Direction)	WB	EB	SB	NB
14	Conflicting Approaches (Directions)	NB, SB	NB, SB	EB, WB	EB, WB
15	Subject Approach Flow Rate	494.44	281.11	1162.2	1471.1
16	Opposing Approach Flow Rate	253	445	1324	1046
17	Conflicting Approaches Flow Rate	2370	2370	698	698
18	Total Intersection Flow Rate	3117.4	3096.1	3184.2	3215.1
19	Proportion, Subject Approach Flow Rate	0.16	0.09	0.36	0.46
20	Proportion, Opposing Approach Flow Rate	0.08	0.14	0.42	0.33
21	Proportion, Conflicting Approaches Flow Rate	0.76	0.77	0.22	0.22
22	LT, Opposing Approach	99	98	43	105
23	RT, Opposing Approach	24	150	72	68
24	LT, Conflicting Approaches	148	148	197	197
25	RT, Conflicting Approaches	140	140	174	174
26	Proportion LT, Opposing Approach	0.39	0.22	0.03	0.10
27	Proportion RT, Opposing Approach	0.09	0.34	0.05	0.07
28	Proportion LT, Conflicting Approaches	0.06	0.06	0.28	0.28
29	Proportion RT, Conflicting Approaches	0.06	0.06	0.25	0.25
30	Approach Capacity	116	492	747	758
31	Volume/Capacity Ratio	4.26	0.57	1.56	1.94
32	Delay	#####	9	369	1591
33	Level of Service	F	B	F	F
34	Average Delay (Intersection)	#####			
35	Level of Service (Intersection)	F			

Existing Plus Project A.M. Peak
Excelsior Rd and Florin Rd

		EB	WB	NB	SB
1	LT Volume	69	50	29	2
2	TH Volume	116	161	225	54
3	RT Volume	10	6	132	37
4	Peak Hour Factor	0.90	0.90	0.90	0.90
5	Lanes on Subject Approach	1	1	1	1
6	Lanes on Opposing Approach	1	1	1	1
7	LT Flow Rate	76.667	55.556	32.222	2.2222
8	TH Flow Rate	128.89	178.89	250	60
9	RT Flow Rate	11.111	6.6667	146.67	41.111
10	Approach Flow Rate	216.67	241.11	428.89	103.33
11	Proportion LT	0.35	0.23	0.08	0.02
12	Proportion RT	0.05	0.03	0.34	0.40
13	Opposing Approach (Direction)	WB	EB	SB	NB
14	Conflicting Approaches (Directions)	NB, SB	NB, SB	EB, WB	EB, WB
15	Subject Approach Flow Rate	216.67	241.11	428.89	103.33
16	Opposing Approach Flow Rate	217	195	93	386
17	Conflicting Approaches Flow Rate	479	479	412	412
18	Total Intersection Flow Rate	912.67	915.11	933.89	901.33
19	Proportion, Subject Approach Flow Rate	0.24	0.26	0.46	0.11
20	Proportion, Opposing Approach Flow Rate	0.24	0.21	0.10	0.43
21	Proportion, Conflicting Approaches Flow Rate	0.52	0.52	0.44	0.46
22	LT, Opposing Approach	50	69	2	29
23	RT, Opposing Approach	6	10	37	132
24	LT, Conflicting Approaches	31	31	119	119
25	RT, Conflicting Approaches	169	169	16	16
26	Proportion LT, Opposing Approach	0.23	0.35	0.02	0.08
27	Proportion RT, Opposing Approach	0.03	0.05	0.40	0.34
28	Proportion LT, Conflicting Approaches	0.06	0.06	0.29	0.29
29	Proportion RT, Conflicting Approaches	0.35	0.35	0.04	0.04
30	Approach Capacity	527	503	627	485
31	Volume/Capacity Ratio	0.41	0.48	0.68	0.21
32	Delay	5	6	13	2
33	Level of Service	A	B	C	A
34	Average Delay (Intersection)	9			
35	Level of Service (Intersection)	B			

Existing Plus Project P.M. Peak
Excelsior Rd and Florin Rd

		EB	WB	NB	SB
1	LT Volume	62	130	9	8
2	TH Volume	137	170	59	157
3	RT Volume	30	3	72	92
4	Peak Hour Factor	0.90	0.90	0.90	0.90
5	Lanes on Subject Approach	1	1	1	1
6	Lanes on Opposing Approach	1	1	1	1
7	LT Flow Rate	68.889	144.44	10	8.8889
8	TH Flow Rate	152.22	188.89	65.556	174.44
9	RT Flow Rate	33.333	3.3333	80	102.22
10	Approach Flow Rate	254.44	336.67	155.56	285.56
11	Proportion LT	0.27	0.43	0.06	0.03
12	Proportion RT	0.13	0.01	0.51	0.36
13	Opposing Approach (Direction)	WB	EB	SB	NB
14	Conflicting Approaches (Directions)	NB, SB	NB, SB	EB, WB	EB, WB
15	Subject Approach Flow Rate	254.44	336.67	155.56	285.56
16	Opposing Approach Flow Rate	303	229	257	140
17	Conflicting Approaches Flow Rate	397	397	532	532
18	Total Intersection Flow Rate	954.44	962.67	944.56	957.56
19	Proportion, Subject Approach Flow Rate	0.27	0.35	0.16	0.30
20	Proportion, Opposing Approach Flow Rate	0.32	0.24	0.27	0.15
21	Proportion, Conflicting Approaches Flow Rate	0.42	0.41	0.56	0.56
22	LT, Opposing Approach	130	62	8	9
23	RT, Opposing Approach	3	30	92	72
24	LT, Conflicting Approaches	17	17	192	192
25	RT, Conflicting Approaches	164	164	33	33
26	Proportion LT, Opposing Approach	0.43	0.27	0.03	0.06
27	Proportion RT, Opposing Approach	0.01	0.13	0.36	0.51
28	Proportion LT, Conflicting Approaches	0.04	0.04	0.36	0.36
29	Proportion RT, Conflicting Approaches	0.41	0.41	0.06	0.06
30	Approach Capacity	573	672	428	494
31	Volume/Capacity Ratio	0.44	0.50	0.36	0.58
32	Delay	5	7	4	9
33	Level of Service	B	B	A	B
34	Average Delay (Intersection)	7			
35	Level of Service (Intersection)	B			

Existing Plus Project A.M. Peak
Elk Grove-Florin Rd and Florin Rd

		EB	WB	NB	SB
1	LT Volume	98	192	328	261
2	TH Volume	469	683	661	337
3	RT Volume	210	500	138	91
4	Peak Hour Factor	0.90	0.90	0.90	0.90
5	Lanes on Subject Approach	2	2	1	1
6	Lanes on Opposing Approach	2	2	1	1
7	LT Flow Rate	108.89	213.33	364.44	290
8	TH Flow Rate	521.11	758.89	734.44	374.44
9	RT Flow Rate	233.33	555.56	153.33	101.11
10	Approach Flow Rate	863.33	1527.8	1252.2	765.56
11	Proportion LT	0.13	0.14	0.29	0.38
12	Proportion RT	0.27	0.36	0.12	0.13
13	Opposing Approach (Direction)	WB	EB	SB	NB
14	Conflicting Approaches (Directions)	NB, SB	NB, SB	EB, WB	EB, WB
15	Subject Approach Flow Rate	863.33	1527.8	1252.2	765.56
16	Opposing Approach Flow Rate	1375	777	689	1127
17	Conflicting Approaches Flow Rate	1816	1816	2152	2152
18	Total Intersection Flow Rate	4054.3	4120.8	4093.2	4044.6
19	Proportion, Subject Approach Flow Rate	0.21	0.37	0.31	0.19
20	Proportion, Opposing Approach Flow Rate	0.34	0.19	0.17	0.28
21	Proportion, Conflicting Approaches Flow Rate	0.45	0.44	0.53	0.53
22	LT, Opposing Approach	192	98	261	328
23	RT, Opposing Approach	500	210	91	138
24	LT, Conflicting Approaches	589	589	290	290
25	RT, Conflicting Approaches	229	229	710	710
26	Proportion LT, Opposing Approach	0.14	0.13	0.38	0.29
27	Proportion RT, Opposing Approach	0.36	0.27	0.13	0.12
28	Proportion LT, Conflicting Approaches	0.32	0.32	0.13	0.13
29	Proportion RT, Conflicting Approaches	0.13	0.13	0.33	0.33
30	Approach Capacity	622	659	495	480
31	Volume/Capacity Ratio	1.39	2.32	2.53	1.59
32	Delay	196	6655	14936	428
33	Level of Service	F	F	F	F
34	Average Delay (Intersection)	6661			
35	Level of Service (Intersection)	F			

Existing Plus Project P.M. Peak
Elk Grove-Florin Rd and Florin Rd

		EB	WB	NB	SB
1	LT Volume	84	226	346	649
2	TH Volume	764	672	515	752
3	RT Volume	465	482	228	130
4	Peak Hour Factor	0.90	0.90	0.90	0.90
5	Lanes on Subject Approach	2	2	1	1
6	Lanes on Opposing Approach	2	2	1	1
7	LT Flow Rate	93.333	251.11	384.44	721.11
8	TH Flow Rate	848.89	746.67	572.22	835.56
9	RT Flow Rate	516.67	535.56	253.33	144.44
10	Approach Flow Rate	1458.9	1533.3	1210	1701.1
11	Proportion LT	0.06	0.16	0.32	0.42
12	Proportion RT	0.35	0.35	0.21	0.08
13	Opposing Approach (Direction)	WB	EB	SB	NB
14	Conflicting Approaches (Directions)	NB, SB	NB, SB	EB, WB	EB, WB
15	Subject Approach Flow Rate	1458.9	1533.3	1210	1701.1
16	Opposing Approach Flow Rate	1380	1313	1531	1089
17	Conflicting Approaches Flow Rate	2620	2620	2693	2693
18	Total Intersection Flow Rate	5458.9	5466.3	5434	5483.1
19	Proportion, Subject Approach Flow Rate	0.27	0.28	0.22	0.31
20	Proportion, Opposing Approach Flow Rate	0.25	0.24	0.28	0.20
21	Proportion, Conflicting Approaches Flow Rate	0.48	0.48	0.50	0.49
22	LT, Opposing Approach	226	84	649	346
23	RT, Opposing Approach	482	465	130	228
24	LT, Conflicting Approaches	995	995	310	310
25	RT, Conflicting Approaches	358	358	947	947
26	Proportion LT, Opposing Approach	0.16	0.06	0.42	0.32
27	Proportion RT, Opposing Approach	0.35	0.35	0.08	0.21
28	Proportion LT, Conflicting Approaches	0.38	0.38	0.12	0.12
29	Proportion RT, Conflicting Approaches	0.14	0.14	0.35	0.35
30	Approach Capacity	592	627	481	567
31	Volume/Capacity Ratio	2.46	2.44	2.52	3.00
32	Delay	11668	10806	14270	89765
33	Level of Service	F	F	F	F
34	Average Delay (Intersection)	34482			
35	Level of Service (Intersection)	F			

Existing Plus Project A.M. Peak
Bradshaw and Calvin

		EB	WB	NB	SB
1	LT Volume	143	63	5	29
2	TH Volume	131	182	446	272
3	RT Volume	12	65	31	89
4	Peak Hour Factor	0.90	0.90	0.90	0.90
5	Lanes on Subject Approach	1	1	1	1
6	Lanes on Opposing Approach	1	1	1	1
7	LT Flow Rate	158.89	70	5.5556	32.222
8	TH Flow Rate	145.56	202.22	495.56	302.22
9	RT Flow Rate	13.333	72.222	34.444	98.889
10	Approach Flow Rate	317.78	344.44	535.56	433.33
11	Proportion LT	0.50	0.20	0.01	0.07
12	Proportion RT	0.04	0.21	0.06	0.23
13	Opposing Approach (Direction)	WB	EB	SB	NB
14	Conflicting Approaches (Directions)	NB, SB	NB, SB	EB, WB	EB, WB
15	Subject Approach Flow Rate	317.78	344.44	535.56	433.33
16	Opposing Approach Flow Rate	310	286	390	482
17	Conflicting Approaches Flow Rate	872	872	596	596
18	Total Intersection Flow Rate	1499.8	1502.4	1521.6	1511.3
19	Proportion, Subject Approach Flow Rate	0.21	0.23	0.35	0.29
20	Proportion, Opposing Approach Flow Rate	0.21	0.19	0.26	0.32
21	Proportion, Conflicting Approaches Flow Rate	0.58	0.58	0.39	0.39
22	LT, Opposing Approach	63	143	29	5
23	RT, Opposing Approach	65	12	89	31
24	LT, Conflicting Approaches	34	34	206	206
25	RT, Conflicting Approaches	120	120	77	77
26	Proportion LT, Opposing Approach	0.20	0.50	0.07	0.01
27	Proportion RT, Opposing Approach	0.21	0.04	0.23	0.06
28	Proportion LT, Conflicting Approaches	0.04	0.04	0.35	0.35
29	Proportion RT, Conflicting Approaches	0.14	0.14	0.13	0.13
30	Approach Capacity	467	350	590	555
31	Volume/Capacity Ratio	0.68	0.98	0.91	0.78
32	Delay	13	42	32	19
33	Level of Service	C	E	E	C
34	Average Delay (Intersection)	27			
35	Level of Service (Intersection)	D			

Existing Plus Project P.M. Peak
Bradshaw and Calvin

		EB	WB	NB	SB
1	LT Volume	105	45	7	51
2	TH Volume	114	147	314	537
3	RT Volume	11	23	68	135
4	Peak Hour Factor	0.90	0.90	0.90	0.90
5	Lanes on Subject Approach	1	1	1	1
6	Lanes on Opposing Approach	1	1	1	1
7	LT Flow Rate	116.67	50	7.7778	56.667
8	TH Flow Rate	126.67	163.33	348.89	596.67
9	RT Flow Rate	12.222	25.556	75.556	150
10	Approach Flow Rate	255.56	238.89	432.22	803.33
11	Proportion LT	0.46	0.21	0.02	0.07
12	Proportion RT	0.05	0.11	0.17	0.19
13	Opposing Approach (Direction)	WB	EB	SB	NB
14	Conflicting Approaches (Directions)	NB, SB	NB, SB	EB, WB	EB, WB
15	Subject Approach Flow Rate	255.56	238.89	432.22	803.33
16	Opposing Approach Flow Rate	215	230	723	389
17	Conflicting Approaches Flow Rate	1112	1112	445	445
18	Total Intersection Flow Rate	1582.6	1580.9	1600.2	1637.3
19	Proportion, Subject Approach Flow Rate	0.16	0.15	0.27	0.49
20	Proportion, Opposing Approach Flow Rate	0.14	0.15	0.45	0.24
21	Proportion, Conflicting Approaches Flow Rate	0.70	0.70	0.28	0.27
22	LT, Opposing Approach	45	105	51	7
23	RT, Opposing Approach	23	11	135	68
24	LT, Conflicting Approaches	58	58	150	150
25	RT, Conflicting Approaches	203	203	34	34
26	Proportion LT, Opposing Approach	0.21	0.46	0.07	0.02
27	Proportion RT, Opposing Approach	0.11	0.05	0.19	0.17
28	Proportion LT, Conflicting Approaches	0.05	0.05	0.34	0.34
29	Proportion RT, Conflicting Approaches	0.18	0.18	0.08	0.08
30	Approach Capacity	354	265	624	708
31	Volume/Capacity Ratio	0.72	0.90	0.69	1.13
32	Delay	16	31	14	74
33	Level of Service	C	E	C	F
34	Average Delay (Intersection)	45			
35	Level of Service (Intersection)	E			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: BRADSHAW RD AND FLORIN RD
Problem Statement: EXISTING PLUS PROJECT (M)

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW RD		-----Approach-----				Possible Volume Adjusted			
1 2 2 ^		: -1- -2- -3- -4-				Critical Volume Critical			
R L N		a.No. of change				Prob- able Phase			
R T T T L		: 0 0 0 0				Critical Volume in vph			
T H H H T		: 0 0 0 0				Volume Carryover to next phase			
-----		: 0 0 0 0				Adjusted Critical Volume in vph			
Approach 1 << >> ^--RT 1		b.LT capacity on change (vph)				B2B1 85(B1) 195- 85= 110(B2) 85			
1 LT--^ v v v <^--RTH		: 0 0 0 0				A1B2 110(B2) 150- 110= 40(A1) 110			
LTH-->		: 0 0 0 0				A1A2 181(A2) OR 40(A1) 181			
2 TH-->		: 0 0 0 0				B4B3 31(B4) 221- 31= 190(B3) 31			
RTH--v>		: 0 0 0 0				A4B3 190(B3) 672- 190= 482(A4) 190			
1 RT--v << >> v--LT 1		e.LT capacity on green (vph)				A3A4 482(A4) OR 291(A3) 482			
L L T R R		: 0 0 0 0							
T T H T T FLORIN RD		f.LT capacity in vph (b+e)							
H H		: 0 0 0 0							
2 2 1		g.Left turn volume in vph							
Approach 4:BRADSHAW		: 0 0 0 0							
		h.Is volume > cap. (g>f) ?							

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		1 2 2		195(B1B2)+181(A2)+221(B4B3)+482(A4)	
3: LT= 55 2:RT= 147		7 9 9 2 3		= 1079 vph	
TH= 582 TH= 362		2 1 1 5 1			
RT= 172 v LT= 85		< v v >		Step 8. INTERSECTION LEVEL OF SERVICE	
				(compare step 7 with table 6)	
<--Approach 2				C	
Approach 1-->		195 -^		Step 9. RECALCULATE	
		89 ->		Geometric Change:	
1:LT= 195 ^ 4: RT= 108		< < ^ ^ >		Signal Change:	
TH= 178 TH=1344				Volume Change:	
RT= 214 LT= 402		214 -v			
Approach 4					
		2 1 6 6 1			
		2 8 7 7 0			
		1 1 2 2 8			

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS	
--^ v-- B2B1		Approach 3			
--^ AND <-- A1B2 AND					
--> OR v-- /OR A2B1					
--> <-- A1A2		Approach 1			
< B4B3		See Step 6b.			
>		Approach 2			
AND < ^ A3B4 AND					
v > OR /OR A4B3					
^ A3A4					
v					
-----		Approach 4		Exclusive right turns reduced 30 %	
A1 --> A3 B1 v-- B3 <				V/C Ratio = .78	
v ^					
A2 <-- A4 B2 --^ B4 >					

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: BRADSHAW RD AND FLORIN RD
Problem Statement: EXISTING PLUS PROJECT (M)

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW RD 1 2 2 ^ R L N FLORIN RD R T T T L T H H H T Approach 1 << >> ^--RT 1 1 LT--^ v v v <^--RTH LTH--> <--TH 2 2 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 2 2 1 Approach 4:BRADSHAW		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph B2B1 122(B1) 228- 122= 106(B2) 122 A1B2 106(B2) 299- 106= 193(A1) 106 A1A2 193(A1) OR 136(A2) 193 B4B3 87(B4) 185- 87= 98(B3) 87 A4B3 98(B3) 386- 98= 288(A4) 98 A3A4 733(A3) OR 288(A4) 733			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 157 Approach 3 2:RT= 100 TH=1465 TH= 271 RT= 272 v LT= 122 <--Approach 2 Approach 1--> 1:LT= 228 ^ 4: RT= 111 TH= 360 TH= 772 RT= 498 LT= 335 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 2 7 7 7 3 3 7 8 2 3 3 1 7 < v v >> ^- 100 <- 136 <- 136 v- 122 228 -^ 180 -> 180 -> 498 -v << ^ ^ ^> 1 1 3 3 1 8 5 8 8 1 5 1 6 6 1				Step 7. SUM OF CRITICAL VOLUMES 228(B1B2)+193(A1)+185(B4B3)+733(A3) = 1339 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) E			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 40 % V/C Ratio = .971			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: BRADSHAW RD AND ELDER CREEK RD
Problem Statement: EXISTING PLUS PROJECT (M)

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW 1 2 1 ^ R L N ELDER CREEK R R T T L T H H H T Approach 1 << >> ^--RT 1 1 LT--^ v v v <^--RTH LTH--> <--TH TH--> <v--LTH 1 1 RTH--v ^ ^ ^ v--LT RT--v << >> Approach 2 L L T R R T T H T T ELDER CREEK R H H 1 1 1 Approach 4: BRADSHAW		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- Possible Volume Adjusted able Critical Carryover Critical Phase in vph phase in vph ----- A1B2 89(A1) OR 45(B2) 89 A2B1 151(A2) OR 16(B1) 151 B4B3 6(B4) 140- 6= 134(B3) 6 A4B3 134(B3) 737- 134= 603(A4) 134 A3A4 603(A4) OR 332(A3) 603			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 6 TH= 663 RT= 136 Approach 3 2: RT= 8 TH= 135 LT= 17 <--Approach 2 Approach 1-->		Step 5. ASSIGN LANE VOLUMES, in vph 1 3 3 3 3 3 6 2 2 6 < v v > 45 -^ < ^ ^ > 36 +> + + 53 +v 1 7 7 4 3 0 3 0 7 6 1				Step 7. SUM OF CRITICAL VOLUMES 89(A1)+151(A2)+140(B4B3)+603(A4) = 983 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) C Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ A1B2 --> <-- A2B1 v-- < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .71			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: BRADSHAW RD AND ELDER CREEK RD
Problem Statement: EXISTING PLUS PROJECT (M)

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK		Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP	
Approach 3:BRADSHAW 1 2 1 ^ R L N ELDER CREEK R R T T T L T H H H T Approach 1 << >> ^--RT 1 1 LT--^ v v v <^--RTH LTH-^> <--TH TH--> <v-LTH 1 1 RTH-v> ^ ^ ^ v--LT RT--v << >> Approach 2 L L T R R T T H T T ELDER CREEK R H H 1 1 1 Approach 4:BRADSHAW		Approach : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :		Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph A1B2 232(A1) OR 130(B2) 232 A2B1 84(A2) OR 44(B1) 84 B4B3 14(B4) 94- 14= 80(B3) 14 A4B3 80(B3) 495- 80= 415(A4) 80 A3A4 813(A3) OR 415(A4) 813	
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 14 Approach 3 2:RT= 7 TH=1626 TH= 40 RT= 68 v LT= 44 <--Approach 2 Approach 1--> 1:LT= 130 ^ 4: RT= 35 TH= 97 TH= 954 RT= 135 LT= 94 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 8 8 6 1 1 1 8 3 3 4 < v v > 130 -^ < ^ ^ > 97 +> + + 135 +v 4 4 9 9 6 3 4 5 0 5		Step 7. SUM OF CRITICAL VOLUMES 232(A1)+84(A2)+94(B4B3)+813(A3) = 1223 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) D Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	
Step 3. IDENTIFY PHASING --^ A1B2 --> <-- A2B1 v-- < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		COMMENTS Exclusive right turns reduced / % V/C Ratio = .89	
A1 --> A3 81 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >					

Existing Plus Project A.M. Peak
Excelsior and Calvin

		EB	WB	NB	SB
1	LT Volume	27	7	49	5
2	TH Volume	102	86	229	61
3	RT Volume	59	23	11	6
4	Peak Hour Factor	0.90	0.90	0.90	0.90
5	Lanes on Subject Approach	1	1	1	1
6	Lanes on Opposing Approach	1	1	1	1
7	LT Flow Rate	30	7.7778	54.444	5.5556
8	TH Flow Rate	113.33	95.556	254.44	67.778
9	RT Flow Rate	65.556	25.556	12.222	6.6667
10	Approach Flow Rate	208.89	128.89	321.11	80
11	Proportion LT	0.14	0.06	0.17	0.07
12	Proportion RT	0.31	0.20	0.04	0.08
13	Opposing Approach (Direction)	WB	EB	SB	NB
14	Conflicting Approaches (Directions)	NB, SB	NB, SB	EB, WB	EB, WB
15	Subject Approach Flow Rate	208.89	128.89	321.11	80
16	Opposing Approach Flow Rate	116	188	72	289
17	Conflicting Approaches Flow Rate	361	361	304	304
18	Total Intersection Flow Rate	685.89	677.89	697.11	673
19	Proportion, Subject Approach Flow Rate	0.30	0.19	0.46	0.12
20	Proportion, Opposing Approach Flow Rate	0.17	0.28	0.10	0.43
21	Proportion, Conflicting Approaches Flow Rate	0.53	0.53	0.44	0.45
22	LT, Opposing Approach	7	27	5	49
23	RT, Opposing Approach	23	59	6	11
24	LT, Conflicting Approaches	54	54	34	34
25	RT, Conflicting Approaches	17	17	82	82
26	Proportion LT, Opposing Approach	0.06	0.14	0.07	0.17
27	Proportion RT, Opposing Approach	0.20	0.31	0.08	0.04
28	Proportion LT, Conflicting Approaches	0.15	0.15	0.11	0.11
29	Proportion RT, Conflicting Approaches	0.05	0.05	0.27	0.27
30	Approach Capacity	514	473	676	524
31	Volume/Capacity Ratio	0.41	0.27	0.47	0.15
32	Delay	5	3	6	2
33	Level of Service	A	A	B	A
34	Average Delay (Intersection)	5			
35	Level of Service (Intersection)	A			

Existing Plus Project P.M. Peak
Excelsior and Calvin

		EB	WB	NB	SB
1	LT Volume	14	2	63	22
2	TH Volume	34	88	117	170
3	RT Volume	65	8	4	22
4	Peak Hour Factor	0.90	0.90	0.90	0.90
5	Lanes on Subject Approach	1	1	1	1
6	Lanes on Opposing Approach	1	1	1	1
7	LT Flow Rate	15.556	2.2222	70	24.444
8	TH Flow Rate	37.778	97.778	130	188.89
9	RT Flow Rate	72.222	8.8889	4.4444	24.444
10	Approach Flow Rate	125.56	108.89	204.44	237.78
11	Proportion LT	0.12	0.02	0.34	0.10
12	Proportion RT	0.58	0.08	0.02	0.10
13	Opposing Approach (Direction)	WB	EB	SB	NB
14	Conflicting Approaches (Directions)	NB, SB	NB, SB	EB, WB	EB, WB
15	Subject Approach Flow Rate	125.56	108.89	204.44	237.78
16	Opposing Approach Flow Rate	98	113	214	184
17	Conflicting Approaches Flow Rate	398	398	211	211
18	Total Intersection Flow Rate	621.56	619.89	629.44	632.78
19	Proportion, Subject Approach Flow Rate	0.20	0.18	0.32	0.38
20	Proportion, Opposing Approach Flow Rate	0.16	0.18	0.34	0.29
21	Proportion, Conflicting Approaches Flow Rate	0.64	0.64	0.34	0.33
22	LT, Opposing Approach	2	14	22	63
23	RT, Opposing Approach	8	65	22	4
24	LT, Conflicting Approaches	85	85	16	16
25	RT, Conflicting Approaches	26	26	73	73
26	Proportion LT, Opposing Approach	0.02	0.12	0.10	0.34
27	Proportion RT, Opposing Approach	0.08	0.58	0.10	0.02
28	Proportion LT, Conflicting Approaches	0.21	0.21	0.08	0.08
29	Proportion RT, Conflicting Approaches	0.07	0.07	0.35	0.35
30	Approach Capacity	378	437	734	662
31	Volume/Capacity Ratio	0.33	0.25	0.28	0.36
32	Delay	4	3	3	4
33	Level of Service	A	A	A	A
34	Average Delay (Intersection)	3			
35	Level of Service (Intersection)	A			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND BRADSHAW
Problem Statement: EXISTING PLUS PROJECT (M)

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW		-----Approach-----				Possible Volume Adjusted			
1 2 2 ^		: -1- -2- -3- -4-				Critical Volume Critical			
R L N		a.No. of change : 0 0 0 0				Prob- able Phase			
R T T T L		intervals/hour : 0 0 0 0				Critical Volume in vph			
T H H H T		b.LT capacity on change (vph) : 0 0 0 0				Volume to next phase			
Approach 1 << >> ^--RT 1		c.G/C ratio : 0 0 0 0				B2B1 15(B1) 200- 15= 185(B2) 15			
1 LT--^ v v v <^--RTH		d.Opposing volume : 0 0 0 0				A1B2 185(B2) 180- 185= 0(A1) 185			
LTH--^ <--TH 1		in vph : 0 0 0 0				A1A2 423(A2) OR 0(A1) 423			
1 TH--> <v--LTH		e.LT capacity on green (vph) : 0 0 0 0				B4B3 41(B4) 187- 41= 146(B3) 41			
RTH--v ^ ^ ^ v--LT 1		f.LT capacity in vph (b+e) : 0 0 0 0				A4B3 146(B3) 575- 146= 429(A4) 146			
1 RT--v << >> Approach 2		g.Left turn volume : 0 0 0 0				A3A4 429(A4) OR 285(A3) 429			
L L T R R		h.Is volume > cap. (g>f) ? :							
T T H T T JACKSON HWY									
H H									
2 2 1									
Approach 4: BRADSHAW									

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
3: LT= 74 TH= 569 RT= 226		2 2 2 2 8 8 3 4 6 5 5 3 1 < v v >>		200(B1B2)+423(A2)+187(B4B3)+429(A4)	
Approach 3 2:RT= 187 TH= 423 LT= 15		^-- 187 <- 423 v- 15		= 1239 vph	
<--Approach 2				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)	
Approach 1-->				E	
1:LT= 200 TH= 180 RT= 181		200 --^ << ^ ^ ^ > 180 --> 181 --v		Step 9. RECALCULATE	
Approach 4 4: RT= 14 TH=1149 LT= 340		1 1 5 5 8 5 7 7 1 7 3 5 5 4		Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1		Approach 3		
--^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		Approach 1		
< B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		See Step 6b.		
		Approach 2		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Approach 4		Exclusive right turns reduced 30 % V/C Ratio = .9

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND BRADSHAW
Problem Statement: EXISTING PLUS PROJECT (M)

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW 1 2 2 ^ R L N JACKSON HWY R T T T L T H H H T Approach 1 << >> ^--RT 1 1 LT--^ v v v <^--RTH LTH--^> <--TH 1 1 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T JACKSON HWY H H 2 2 1 Approach 4:BRADSHAW		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Prob- Critical Carryover Critical able Volume to next Volume Phase in vph phase in vph ----- B2B1 28(B1) 191- 28= 163(B2) 28 A1B2 163(B2) 433- 163= 270(A1) 163 A1A2 270(A1) OR 156(A2) 270 B4B3 91(B4) 175- 91= 84(B3) 91 A4B3 84(B3) 395- 84= 311(A4) 84 A3A4 617(A3) OR 311(A4) 617			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 165 Approach 3 2:RT= 76 TH=1234 TH= 156 RT= 182 v LT= 28 ----- <--Approach 2 ----- Approach 1--> ----- 1:LT= 191 ^ 4: RT= 20 TH= 433 TH= 790 RT= 413 LT= 317 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph ----- 1 6 6 ----- 8 1 1 7 9 ----- 2 7 7 5 1 ----- < v v >> ----- 191 ^- << ^ ^ ^ 433 -> 413 -v ----- 1 1 3 3 ----- 7 4 9 9 2 ----- 5 3 5 5 0				Step 7. SUM OF CRITICAL VOLUMES ----- 191(B1B2)+270(A1)+175(B4B3)+617(A3) ----- = 1253 vph ----- Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- E ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING ----- --^ v-- B2B1 ----- --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 ----- < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v ----- A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS ----- Exclusive right turns reduced 30 % V/C Ratio = .91			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: BRADSHAW AND GERBER RD

Design Hour: PM

Problem Statement: EXISTING PLUS PROJECT (M)

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW RD 1 2 1 ^ R L N R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph			
Approach 1 << >> ^-RT 1 2 LT--^ v v v <^RTH LTH-^> <--TH 1 1 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R GERBER RD T T H T T H H 1 2 1 Approach 4:BRADSHAW RD						B2B1 21(B1) 322- 21= 301(B2) 21 A1B2 301(B2) 321- 301= 20(A1) 301 A1A2 288(A2) OR 20(A1) 288 B4B3 101(B3) 194- 101= 93(B4) 101 A3B4 93(B4) 461- 93= 368(A3) 93 A3A4 368(A3) OR 145(A4) 368			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
3: LT= 194 Approach 3 TH= 726 2:RT= 180 RT= 659 v TH= 288 LT= 37 <--Approach 2		6 3 3 1 ----- 5 6 6 9 9 3 3 4 < v v >		^- 180 <- 288 v- 17 v- 21 Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- D -----	
Approach 1--> ----- 1:LT= 585 ^ 4: RT= 35 TH= 321 TH= 290 RT= 103 LT= 101 Approach 4		322 -^ 264 -^ < ^ ^ > 321 -> 103 -v ----- 1 1 1 0 4 4 3 1 5 5 5		Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				Exclusive right turns reduced 30 % V/C Ratio = .854

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: BRADSHAW AND GERBER RD

Design Hour: AM

Problem Statement: EXISTING PLUS PROJECT (M)

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD		Approach				Possible Volume Adjusted			
1 2 1		-1- -2- -3- -4-				Critical Volume to next phase			
R L N		a.No. of change intervals/hour				Prob- Critical Volume in vph			
GERBER RD R T T T L		: 0 0 0 0				Phase			
T H H H T		b.LT capacity on change (vph)				B2B1 17(B1) 238- 17= 221(B2) 17			
Approach 1 << >> ^--RT 1		: 0 0 0 0				A1B2 221(B2) 187- 221= 0(A1) 221			
2 LT--^ v v v <^--RTH		: 0 0 0 0				A1A2 252(A2) OR 0(A1) 252			
LTH-^>		c.G/C ratio				B4B3 71(B3) 92- 71= 21(B4) 71			
1 TH-->		: 0 0 0 0				A3B4 21(B4) 258- 21= 237(A3) 21			
RTH-v>		d.Opposing volume in vph				A3A4 394(A4) OR 237(A3) 394			
1 RT--v << >> v--LT 2		: 0 0 0 0							
Approach 2		e.LT capacity on green (vph)							
L L T R R		: 0 0 0 0							
T T H T T GERBER RD		f.LT capacity in vph (b+e)							
H H		: 0 0 0 0							
1 2 1		g.Left turn volume in vph							
Approach 4: BRADSHAW RD		: 0 0 0 0							
		h.Is volume > cap. (g>f) ?							

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		3 1 1		238(B1B2)+252(A2)+92(B3B4)+394(A4)	
3: LT= 92		6 5 5 9		= 976 vph	
TH= 311		8 6 6 2			
RT= 368		< v v >			
v		^-- 145		Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)	
		<-- 252		C	
		v-- 14			
		v-- 17			
Approach 2		238 ^--		Step 9. RECALCULATE	
Approach 1-->		194 ^--		Geometric Change:	
		< ^ ^ >		Signal Change:	
				Volume Change:	
		187 ^--			
		67 -v			
Approach 4		3 3			
		7 9 9 3			
		1 4 4 1			

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1		Approach 3		
--^ AND <-- A1B2 AND				
--> OR v-- /OR A2B1				Approach 1
--> <-- A1A2				
< B4B3		See Step 6b.		Approach 2
>				
AND < ^ A3B4 AND				Approach 4
v > OR /OR A4B3				
^ A3A4				Exclusive right turns reduced 30 % V/C Ratio = .716
v				
A1 --> A3 B1 v-- B3 <				
A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELK GROVE-FLORIN AND FLORIN
Problem Statement: EXISTING PLUS PROJECT MITIGATED

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE-FLO 1 2 2 ^ R L N R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 D 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph			
Approach 1 << >> ^-RT 1 1 LT--^ v v v <^-RTH LTH-^> <--TH 2 2 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2						B4B3 144(B4) 180- 144= 36(B3) 144 A4B3 36(B3) 331- 36= 295(A4) 36 A3A4 295(A4) OR 169(A3) 295 B2B1 98(B2) 192- 98= 94(B1) 98 A2B1 94(B1) 350- 94= 256(A2) 94 A1A2 256(A2) OR 235(A1) 256			
L L T R R T T H T T FLORIN RD H H 2 2 1 Approach 4: ELK GROVE-FLO									

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3 3: LT= 261 2:RT= 500 TH= 337 TH= 683 RT= 91 v LT= 192		1 1 1 1 9 6 6 1 4 1 9 9 8 4 < v v v >		180(B4B3)+295(A4)+192(B2B1)+256(A2) = 923 vph	
Approach 2 Approach 1-->		98 -^ 235 -> << ^ ^ ^ > 235 -> 210 -v		Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) B	
Approach 4 1:LT= 98 ^ 4: RT= 138 TH= 469 TH= 661 RT= 210 LT= 328		1 1 3 3 1 8 4 3 3 3 0 8 1 1 8		Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING	Step 6a. CRITICAL VOLUMES, in vph (two phase signal)	COMMENTS
< B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2	Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4	Exclusive right turns reduced 30 % V/C Ratio = .67
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELK GROVE-FLORIN AND FLORIN
Problem Statement: EXISTING PLUS PROJECT MITIGATED

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE-FLO 1 2 2 R L M R T T T L T H H H T Approach 1 << >> ^--RT 1 1 LT--^ v v v <^--RTH LTH--^> <--TH 2 2 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 2 2 1 Approach 4: ELK GROVE-FLO		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- able Phase Critical Volume in vph Volume to next phase Adjusted Critical Volume in vph B4B3 190(B3) 358- 190= 168(B4) 190 A3B4 168(B4) 376- 168= 208(A3) 168 A3A4 258(A4) OR 208(A3) 258 B2B1 84(B2) 226- 84= 142(B1) 84 A2B1 142(B1) 337- 142= 195(A2) 142 A1A2 382(A1) OR 195(A2) 382			
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
3: LT= 649 TH= 752 RT= 130 Approach 3 2:RT= 482 TH= 672 v LT= 226 <--Approach 2 Approach 1-->		1 3 3 2 3 3 7 7 9 5 0 6 6 3 8 < v v >> 84 -^ 382 -> 382 -> 465 -v << ^ ^ ^> 1 1 2 2 2 9 5 5 5 2 0 6 8 8 8				358(B3B4)+258(A4)+226(B2B1)+382(A1) = 1224 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) D Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
< B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				Exclusive right turns reduced 30 % V/C Ratio = .89			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELK GROVE-FLORIN AND GERBER RD
Problem Statement: EXISTING PLUS PROJECT (M)

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE-FLO 1 2 2 ^ R L N R T T T L T H H H T Approach 1 << >> ^--RT 1 LT--^ v v v <^RTH 1 LTH-^> <--TH 1 1 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 2 2 1 Approach 4: ELK GROVE-FLO		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph ----- B2B1 100(B2) 196- 100= 96(B1) 100 A2B1 96(B1) 453- 96= 357(A2) 96 A1A2 384(A1) OR 357(A2) 384 B4B3 129(B4) 414- 129= 285(B3) 129 A4B3 285(B3) 358- 285= 73(A4) 285 A3A4 173(A3) OR 73(A4) 173			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 234 TH= 345 RT= 187 Approach 3 2: RT= 442 TH= 464 LT= 196 v <--Approach 2 Approach 1--> 1: LT= 100 TH= 384 RT= 269 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 1 1 1 1 1 8 7 7 0 2 7 3 3 5 9 < v v > 100 -^ << ^ ^ > 384 -> 269 -v 4 3 3 3 1 1 3 5 5 6 4 8 8 8 6				Step 7. SUM OF CRITICAL VOLUMES 196(B2B1)+384(A1)+414(B4B3)+173(A3) = 1167 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) D Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .85			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELK GROVE-FLORIN AND GERBER RD
Problem Statement: EXISTING PLUS PROJECT (M)

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE-FLO		-----Approach-----				Possible Volume Adjusted			
1 2 2 ^		: -1- -2- -3- -4-				Critical Volume Critical			
R L M		a.No. of change : 0 0 0 0				Prob- able Phase			
R T T T L		intervals/hour : 0 0 0 0				Critical Volume in vph			
T H H H T		b.LT capacity on change (vph) : 0 0 0 0				Volume to next phase			
Approach 1 << >> ^--RT		c.G/C ratio : 0 0 0 0				B2B1 214(B2) 277- 214= 63(B1) 214			
1 LT--^ v v v <^--RTH 1		d.Opposing volume : 0 0 0 0				A2B1 63(B1) 505- 63= 442(A2) 63			
LTH--^ <--TH 1		in vph : 0 0 0 0				A1A2 535(A1) OR 442(A2) 535			
1 TH--> <v--LTH		e.LT capacity on : 0 0 0 0				B4B3 185(B3) 333- 185= 148(B4) 185			
RTH--v ^ ^ ^ v--LT 1		green (vph) : 0 0 0 0				A3B4 148(B4) 357- 148= 209(A3) 148			
1 RT--v << >> Approach 2		f.LT capacity in : 0 0 0 0				A3A4 230(A4) OR 209(A3) 230			
L L T R R		vph (b+e) :							
T T H T T GERBER RD		g.Left turn volume : 0 0 0 0							
H H		in vph :							
2 2 1		h.Is volume > cap. :							
Approach 4: ELK GROVE-FLO		(g>f) ? :							

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		2 3 3 2 3		277(B2B1)+535(A1)+333(B3B4)+230(A4)	
3: LT= 605		1 5 5 7 3		= 1375 vph	
TH= 714		0 7 7 3 3			
RT= 210				Step 8. INTERSECTION LEVEL OF SERVICE	
v		< v v >		(compare step 7 with table 6)	
<--Approach 2				E	
Approach 1-->		214 ^		Step 9. RECALCULATE	
1: LT= 214		535 ->		Geometric Change:	
TH= 535		835 -v		Signal Change:	
RT= 835		1 1 2 2 2		Volume Change:	
Approach 4		8 5 3 3 4			
		5 1 0 0 7			

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1		Approach 3		
--^ AND <-- A1B2 AND		Approach 1		
--> OR v-- /OR A2B1		See Step 6b.		
--> <-- A1A2		Approach 2		
< B4B3				
>				
AND < ^ A3B4 AND				Exclusive right turns reduced 40 % V/C Ratio = 1.04
v > OR /OR A4B3				
^ A3A4				
v				
A1 --> A3 B1 v-- B3 <		Approach 4		
v ^				
A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELDER CREEK AND S. WATT

Design Hour: AM

Problem Statement: EXISTING PLUS PROJECT MITIGATED

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: S. WATT 1 2 1 ^ R L N ELDER CREEK R T T T L T H H H T Approach 1 << >> ^-RT 1 LT--^ v v v <^-RTH 1 LTH--> <--TH TH--> <v-LTH 1 RTH--v ^ ^ ^ v--LT 1 RT--v << >> Approach 2 L L T R R T T H T T ELDER CREEK H H 1 2 1 Approach 4: S. WATT		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 47(B1) 69- 47= 22(B2) 47 A1B2 22(B2) 163- 22= 141(A1) 22 A1A2 243(A2) OR 141(A1) 243 B4B3 2(B4) 127- 2= 125(B3) 2 A4B3 125(B3) 532- 125= 407(A4) 125 A3A4 407(A4) OR 268(A3) 407			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
3: LT= 2 Approach 3 2:RT= 40 TH= 536 TH= 203 RT= 112 v LT= 47 <--Approach 2 Approach 1-->		1 2 2 1 6 6 2 8 8 2 < v v > 69 -^ < ^ ^ > 103 +> 60 +v 1 5 5 2 3 3 6 7 2 2 6		69(B1B2)+243(A2)+127(B4B3)+407(A4) = 846 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) B Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	
Approach 4 4: RT= 66 TH=1063 LT= 127					

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				Exclusive right turns reduced 30 % V/C Ratio = .62

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELDER CREEK AND S. WATT
Problem Statement: EXISTING PLUS PROJECT MITIGATED

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: S. WATT 1 2 1 ^ R L N ELDER CREEK R T T T L T H H H T Approach 1 << >> ^--RT 1 LT--^ v v v <^--RTH 1 LTH--^> <--TH TH--> <v-LTH 1 RTH--v> ^ ^ ^ v--LT 1 RT--v << >> Approach 2 L L T R R T T H T T ELDER CREEK H H 1 2 1 Approach 4: S. WATT		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- Possible Volume Adjusted able Critical Carryover Critical Phase Volume to next Volume in vph phase in vph B2B1 98(B2) 99- 98= 1(B1) 98 A2B1 1(B1) 154- 1= 153(A2) 1 A1A2 347(A1) OR 153(A2) 347 B4B3 43(B4) 105- 43= 62(B3) 43 A4B3 62(B3) 437- 62= 375(A4) 62 A3A4 605(A3) OR 375(A4) 605			
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
3: LT= 43 Approach 3 2:RT= 24 TH=1209 TH= 130 RT= 72 v LT= 99 <--Approach 2 Approach 1-->		6 6 7 0 0 4 2 5 5 3 < v v > 98 -^ < ^ ^ > 197 +> 150 +v 1 4 4 0 3 3 6 5 7 7 8				99(B2B1)+347(A1)+105(B4B3)+605(A3) = 1156 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) D			
1:LT= 98 ^ 4: RT= 68 TH= 197 TH= 873 RT= 150 LT= 105 Approach 4		Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:				Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >			
		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .84			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND S. WATT

Design Hour: AM

Problem Statement: EXISTING PLUS PROJECT (M)

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: S. WATT 1 2 1 ^ R L N JACKSON HWY R T T T L T H H H T Approach 1 << >> ^--RT 1 1 LT--^ v v v <^--RTH LTH-^> <--TH 2 2 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T JACKSON HWY H H 1 2 1 Approach 4: S. WATT		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 55(B2) 99- 55= 44(B1) 55 A2B1 44(B1) 327- 44= 283(A2) 44 A1A2 283(A2) OR 109(A1) 283 B4B3 214(B4) 311- 214= 97(B3) 214 A4B3 97(B3) 461- 97= 364(A4) 97 A3A4 428(A3) OR 364(A4) 428			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3 3: LT= 214 2:RT= 467 TH= 856 TH= 596 RT= 189 v LT= 99 <--Approach 2		1 4 4 2 ----- 8 2 2 1 9 8 8 4 < v v >		99(B2B1)+283(A2)+311(B4B3)+428(A3) = 1121 vph	
Approach 1--> 1:LT= 55 4: RT= 44 TH= 213 TH= 922 RT= 156 LT= 311 Approach 4		55 -^ 107 -> < ^ ^ ^ > 107 -> 156 -v ----- 3 4 4 1 6 6 4 1 1 1 4		Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) D	
				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND S. WATT
Problem Statement: EXISTING PLUS PROJECT (M)

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: S. WATT 1 2 1 ^ R L N JACKSON HWY R T T T L T H H H T Approach 1 << >> ^--RT 1 1 LT--^ v v v <^--RTH LTH--^> <--TH 2 2 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R JACKSON HWY T T H T T H H 1 2 1 Approach 4: S. WATT		Approach : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 0 0 d.Opposing volume in vph : 0 0 0 0 e.LT capacity on green (vph) : 0 0 0 0 f.LT capacity in vph (b+e) : 0 0 0 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? :				Possible Critical Volume to next phase Prob-Phase B2B1 76(B1) 113- 76= 37(B2) 76 A1B2 37(B2) 331- 37= 294(A1) 37 A1A2 294(A1) OR 197(A2) 294 B4B3 303(B3) 399- 303= 96(B4) 303 A3B4 96(B4) 552- 96= 456(A3) 96 A3A4 488(A4) OR 456(A3) 488			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 399 Approach 3 2:RT= 281 TH=1104 TH= 355 RT= 82 v LT= 76 <--Approach 2 Approach 1--> 1:LT= 113 ^ 4: RT= 66 TH= 662 TH= 975 RT= 399 LT= 303 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 5 5 3 8 5 5 9 2 2 2 9 < v v > ^- 281 <- 178 <- 178 v- 76				Step 7. SUM OF CRITICAL VOLUMES 113(B1B2)+294(A1)+399(B3B4)+488(A4) = 1294 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) E			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .94			

APPENDIX D

Cumulative No Project Level of Service Calculations

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND S. WATT AVE.
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: S. WATT AVE. 1 3 2 ^ R L N R T T T L T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--^> <--TH 3 3 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T JACKSON HWY H H 2 3 1 Approach 4: S. WATT AVE.		Approach : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 0 0 d.Opposing volume in vph : 0 0 0 0 e.LT capacity on green (vph) : 0 0 0 0 f.LT capacity in vph (b+e) : 0 0 0 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? :				Possible Critical Volume Adjusted Prob- able Critical Volume to next phase in vph Phase in vph B2B1 44(B2) 77- 44= 33(B1) 44 A2B1 33(B1) 609- 33= 576(A2) 33 A1A2 576(A2) OR 126(A1) 576 B4B3 121(B4) 380- 121= 259(B3) 121 A4B3 259(B3) 453- 259= 194(A4) 259 A3A4 693(A3) OR 194(A4) 693			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 220 Approach 3 TH=2080 2:RT= 870 RT= 540 v TH=1110 LT= 140 <--Approach 2 Approach 1--> 1:LT= 80 ^ 4:RT= 60 TH= 200 TH=1360 RT= 180 LT= 690 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 5 6 6 6 1 4 9 9 9 2 0 3 3 3 9 1 < v v v > 44 ^- 36 ^- 67 -> 67 -> << ^ ^ ^ ^ > 67 -> 180 -v 3 3 4 4 4 8 1 5 5 5 6 0 1 3 3 3 0				Step 7. SUM OF CRITICAL VOLUMES 77(B2B1)+576(A2)+380(B4B3)+693(A3) = 1726 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4 COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.26							

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND S. WATT AVE.
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: S. WATT AVE. 1 3 2 ^ R L M R T T T L T H H H T Approach 1 << >> ^-RT 1 2 LT--^ v v v <^-RTH LTH-^> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T JACKSON HWY H H 2 3 1 Approach 4: S. WATT AVE.		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 61(B1) 253- 61= 192(B2) 61 A1B2 192(B2) 399- 192= 207(A1) 192 A1A2 210(A2) OR 207(A1) 210 B4B3 193(B3) 523- 193= 330(B4) 193 A3B4 330(B4) 517- 330= 187(A3) 330 A3A4 680(A4) OR 187(A3) 680			
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
3: LT= 950 Approach 3 TH=1550 2:RT= 300 RT= 130 v TH= 310 LT= 110 <--Approach 2 Approach 1--> 1:LT= 460 ^ 4: RT= 90 TH=1090 TH=2040 RT= 570 LT= 350 Approach 4		1 5 5 5 4 5 3 1 1 1 2 2 0 7 7 7 8 3 < v v v > 253 -^ 207 -^ 363 -> 363 -> << ^ ^ ^ > 363 -> 570 -v ----- 1 1 6 6 6 9 5 8 8 8 9 3 8 0 0 0 0				253(B1B2)+210(A2)+523(B3B4)+680(A4) = 1666 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- F ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				Exclusive right turns reduced 30 % V/C Ratio = 1.21			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND BRADSHAW RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD 1 3 2 ^ R L N JACKSON HWY R T T T L T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--> <--TH 3 3 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T JACKSON HWY H H 2 3 1 Approach 4: BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 105(B1) 154- 105= 49(B2) 105 A1B2 49(B2) 119- 49= 70(A1) 49 A1A2 363(A2) OR 70(A1) 363 B4B3 11(B4) 457- 11= 446(B3) 11 A4B3 446(B3) 600- 446= 154(A4) 446 A3A4 417(A3) OR 154(A4) 417			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 20 Approach 3 2:RT= 260 TH=1250 TH=1090 RT= 320 v LT= 190 <--Approach 2 Approach 1--> 1:LT= 280 ^ 4: RT= 40 TH= 250 TH=1800 RT= 170 LT= 830 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 3 4 4 4 2 1 1 1 1 0 7 7 7 9 1 < v v v v > 154 ^- 126 ^- 83 -> 83 -> << ^ ^ ^ > 83 -> 170 -v 4 3 6 6 6 5 7 0 0 0 4 7 4 0 0 0 0				Step 7. SUM OF CRITICAL VOLUMES 154(B1B2)+363(A2)+457(B4B3)+417(A3) = 1391 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.01			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND BRADSHAW RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW RD 1 3 2 ^ R L N R T T T L T H H H T ----- Approach 1 << >> ^-RT 1 2 LT--^ v v v <^-RTH LTH-^> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T JACKSON HWY H H 2 3 1 Approach 4:BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- able Phase Critical Volume in vph Possible Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph B2B1 55(B1) 149- 55= 94(B2) 55 A1B2 94(B2) 637- 94= 543(A1) 94 A1A2 543(A1) OR 100(A2) 543 B4B3 83(B4) 209- 83= 126(B3) 83 A4B3 126(B3) 467- 126= 341(A4) 126 A3A4 610(A3) OR 341(A4) 610			
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
3: LT= 150 Approach 3 TH=1830 2:RT= 110 RT= 260 v TH= 300 LT= 100 <--Approach 2		2 6 6 6 6 1 1 1 6 8 0 0 0 0 8 3 < v v v > 149 -^ 122 -^ 320 -> 320 -> 320 -> 320 -> 910 -v << ^ ^ ^ > 2 1 4 4 4 2 0 7 6 6 6 0 9 1 7 7 7 0				149(B1B2)+543(A1)+209(B4B3)+610(A3) = 1511 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F			
Approach 1--> 1:LT= 270 ^ 4: RT= 200 TH= 960 TH=1400 RT= 910 LT= 380 Approach 4		Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:				Step 7. SUM OF CRITICAL VOLUMES 149(B1B2)+543(A1)+209(B4B3)+610(A3) = 1511 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				Exclusive right turns reduced 30 % V/C Ratio = 1.1			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND EXCELSIOR RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: EXCELSIOR RD 1 1 1 ^ R L N JACKSON HWY R T T T L T H H H T Approach 1 < < > > ^--RT 1 1 LT--^ v v v <^--RTH LTH--> <--TH 2 2 TH--> <v-LTH RTH--v> ^ ^ ^ v--LT 1 1 RT--v < < > > Approach 2 L L T R R JACKSON HWY T T H T T H H 1 1 1 Approach 4: EXCELSIOR RD		-----Approach----- : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 0 0 d.Opposing volume in vph : 0 0 0 0 e.LT capacity on green (vph) : 0 0 0 0 f.LT capacity in vph (b+e) : 0 0 0 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? :				Possible Critical Volume to next phase Prob-able Phase B2B1 40(B2) 200- 40= 160(B1) 40 A2B1 160(B1) 590- 160= 430(A2) 160 A1A2 430(A2) OR 110(A1) 430 B4B3 10(B4) 120- 10= 110(B3) 10 A4B3 110(B3) 1150- 110=1040(A4) 110 A3A4 1040(A4) OR 240(A3) 1040			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 10 Approach 3 TH= 240 2:RT= 80 RT= 60 v TH=1180 LT= 200 <--Approach 2 Approach 1--> 1:LT= 40 ^ 4: RT= 80 TH= 220 TH=1150 RT= 20 LT= 120 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 2 6 4 1 0 0 0 < v > 40 -^ 110 -> 110 -> 20 -v < ^ > 1 2 5 8 0 0 0				Step 7. SUM OF CRITICAL VOLUMES 200(B2B1)+430(A2)+120(B4B3)+1040(A4) = 1790 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.3			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND EXCELSIOR RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: EXCELSIOR RD 1 1 1 ^ R L M R T T T L T H H H T Approach 1 << >> ^--RT 1 1 LT--^ v v v <^--RTH LTH--> <--TH 2 2 TH--> <v--LTH RTH--v ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R JACKSON HWY T T H T T H H 1 1 1 Approach 4: EXCELSIOR RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 60(B2) 110- 60= 50(B1) 60 A2B1 50(B1) 180- 50= 130(A2) 50 A1A2 600(A1) OR 130(A2) 600 B4B3 30(B4) 30- 30= 0(B3) 30 A4B3 0(B3) 360- 0= 360(A4) 0 A3A4 960(A3) OR 360(A4) 960			
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
3: LT= 30 Approach 3 2:RT= 30 TH= 960 TH= 360 RT= 90 v LT= 110 <--Approach 2 Approach 1--> 1:LT= 60 ^ 4: RT= 170 TH=1200 TH= 360 RT= 120 LT= 30 Approach 4		9 9 6 3 0 0 0 < v > 60 -^ 600 -> < ^ > 600 -> 120 -v 3 1 3 6 7 0 0 0				110(B2B1)+600(A1)+30(B4B3)+960(A3) = 1700 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				Exclusive right turns reduced 30 % V/C Ratio = 1.24			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELDER CREEK RD AND S. WATT AVE.
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: S. WATT AVE. 1 3 2 ^ R L N ELDER CREEK R R T T T L T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--> <--TH 2 2 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T ELDER CREEK R H H 2 3 1 Approach 4: S. WATT AVE.		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- able Phase Critical Volume in vph Possible Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph B2B1 61(B1) 83- 61= 22(B2) 61 A1B2 22(B2) 125- 22= 103(A1) 22 A1A2 392(A2) OR 103(A1) 392 B4B3 94(B4) 253- 94= 159(B3) 94 A4B3 159(B3) 1037- 159= 878(A4) 159 A3A4 878(A4) OR 377(A3) 878			
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
3: LT= 170 Approach 3 TH=1130 2:RT= 560 RT= 270 v TH= 750 LT= 110 <--Approach 2 Approach 1--> 1:LT= 150 ^ 4: RT= 140 TH= 250 TH=3110 RT= 120 LT= 460 Approach 4		2 3 3 3 7 7 7 7 9 0 7 7 7 7 4 < v v v > ^- 560 <- 375 <- 375 v- 50 v- 61 83 ^- 68 ^- 125 -> 125 -> << ^ ^ ^ > 120 -v 2 2 0 0 1 5 0 3 3 4 3 7 7 7 0				83(B1B2)+392(A2)+253(B4B3)+878(A4) = 1606 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				Exclusive right turns reduced 30 % V/C Ratio = 1.17			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELDER CREEK RD AND S. WATT AVE.
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: S. WATT AVE. 1 3 2 ^ R L M ELDER CREEK R R T T T L T H H H T		Approach : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 0 0 d.Opposing volume in vph : 0 0 0 0 e.LT capacity on green (vph) : 0 0 0 0 f.LT capacity in vph (b+e) : 0 0 0 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? :				Prob-able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph B2B1 99(B1) 121- 99= 22(B2) 99 A1B2 22(B2) 325- 22= 303(A1) 22 A1A2 303(A1) OR 220(A2) 303 B4B3 121(B3) 325- 121= 204(B4) 121 A3B4 204(B4) 817- 204= 613(A3) 204 A3A4 613(A3) OR 543(A4) 613			
Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--^> <--TH 2 2 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R		ELDER CREEK R T T H T T H H 2 3 1 Approach 4: S. WATT AVE.							
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
3: LT= 590 TH=2450 RT= 160 Approach 3 2:RT= 250 TH= 440 LT= 180 <--Approach 2		1 8 8 8 2 3 6 1 1 1 6 2 0 7 7 7 6 5 < v v v >>				121(B1B2)+303(A1)+325(B3B4)+613(A3) = 1362 vph			
Approach 1--> 1:LT= 220 TH= 650 RT= 450 Approach 4		121 ^- 99 ^- 325 -> 325 -> 450 -v << ^ ^ ^ ^ >> 1 5 5 5 1 2 9 4 4 5 1 9 3 3 0				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) E			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				Exclusive right turns reduced 30 % V/C Ratio = .99			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELDER CREEK RD AND BRADSHAW RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK		Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP	
Approach 3:BRADSHAW RD 1 3 2 ^ R L N ELDER CREEK R R T T L T H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^~RTH LTH-^> <--TH 2 2 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T ELDER CREEK R H H 2 3 1 Approach 4:BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :		Possible Volume Adjusted Prob- Critical Carryover Critical able Volume to next Volume Phase in vph phase in vph B2B1 11(B1) 138- 11= 127(B2) 11 A1B2 127(B2) 105- 127= 0(A1) 127 A1A2 185(A2) OR 0(A1) 185 B4B3 6(B4) 418- 6= 412(B3) 6 A4B3 412(B3) 833- 412= 421(A4) 412 A3A4 421(A4) OR 303(A3) 421	
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 10 Approach 3 2:RT= 10 TH= 910 TH= 370 RT= 330 v LT= 20 <--Approach 2 Approach 1--> 1:LT= 250 ^ 4: RT= 120 TH= 210 TH=2500 RT= 20 LT= 760 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 3 3 3 3 3 0 0 0 0 3 3 3 5 6 < v v v > 138 ^ 113 ^ 105 -> << ^ ^ ^ > 105 -> 20 -v 4 3 8 8 8 1 1 4 3 3 3 2 8 2 3 3 3 0		Step 7. SUM OF CRITICAL VOLUMES 138(B1B2)+185(A2)+418(B4B3)+421(A4) = 1162 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) D Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .85	

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELDER CREEK RD AND BRADSHAW RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW RD		-----Approach-----				Possible Volume Adjusted			
1 3 2 ^		: -1- -2- -3- -4-				Critical Volume Critical			
R L N		a.No. of change				Prob- able Phase			
R T T T L		: 0 0 0 0				Volume to next phase			
T H H H T		: 0 0 0 0				in vph			
-----		: 0 0 0 0				-----			
Approach 1 << >> ^--RT 1		: 0 0 0 0				B2B1 44(B1) 187- 44= 143(B2) 44			
2 LT--^ v v v <^--RTH		: 0 0 0 0				A1B2 143(B2) 455- 143= 312(A1) 143			
LTH--^>		: 0 0 0 0				A1A2 312(A1) OR 135(A2) 312			
2 TH-->		: 0 0 0 0				B4B3 11(B4) 50- 11= 39(B3) 11			
RTH--v>		: 0 0 0 0				A4B3 39(B3) 430- 39= 391(A4) 39			
1 RT--v << >> Approach 2		: 0 0 0 0				A3A4 847(A3) OR 391(A4) 847			
L L T R R		: 0 0 0 0				-----			
T T H T T ELDER CREEK R		: 0 0 0 0				-----			
H H		: 0 0 0 0				-----			
2 3 1		: 0 0 0 0				-----			
Approach 4:BRADSHAW RD		: 0 0 0 0				-----			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		2 8 8 8		187(B1B2)+312(A1)+50(B4B3)+847(A3)	
3: LT= 20		8 4 4 4 1		= 1396 vph	
TH=2540		0 7 7 7 9 1		-----	
RT= 280		< v v v v >		Step 8. INTERSECTION LEVEL OF SERVICE	
v		-----		(compare step 7 with table 6)	
2:RT= 10		-----		-----	
TH= 270		-----		-----	
LT= 80		-----		-----	
<--Approach 2		-----		-----	
-----		187 -^		-----	
Approach 1-->		153 -^		-----	
-----		180 ->		-----	
1:LT= 340		180 ->		-----	
TH= 360		650 -v		-----	
RT= 650		-----		-----	
Approach 4		-----		-----	
-----		4 4 4		-----	
4: RT= 30		5 4 3 3 3 3		-----	
TH=1290		0 1 0 0 0 0		-----	
LT= 90		-----		-----	
-----		-----		-----	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1		Approach 3		
--^ AND <-- A1B2 AND		-----		
--> OR v-- /OR A2B1		-----		
--> <-- A1A2		-----		
< B4B3		-----		
>		-----		
AND < ^ A3B4 AND		-----		
v > OR /OR A4B3		-----		
^ A3A4		-----		
v		-----		
-----		-----		
A1 --> A3 B1 v-- B3 <		-----		Exclusive right turns reduced 30 % V/C Ratio = 1.02
A2 <-- A4 B2 --^ B4 >		-----		

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELDER CREEK RD AND EXCELSIOR RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY	Step 4. LEFT TURN CHECK	Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP																																																																																																																																																																																																																																																																																																																																					
<p>Approach 3: EXCELSIOR RD</p> <table style="width:100%; border-collapse: collapse;"> <tr> <td style="width:10%;"></td> <td style="width:10%; text-align: center;">1</td> <td style="width:10%; text-align: center;">1</td> <td style="width:10%;"></td> <td style="width:10%; text-align: center;">^</td> </tr> <tr> <td></td> <td style="text-align: center;">R</td> <td style="text-align: center;">L</td> <td></td> <td style="text-align: center;">N</td> </tr> <tr> <td>ELDER CREEK R</td> <td style="text-align: center;">R</td> <td style="text-align: center;">T</td> <td style="text-align: center;">T</td> <td style="text-align: center;">L</td> </tr> <tr> <td></td> <td style="text-align: center;">T</td> <td style="text-align: center;">H</td> <td style="text-align: center;">H</td> <td style="text-align: center;">T</td> </tr> <tr> <td>Approach 1</td> <td style="text-align: center;"><</td> <td style="text-align: center;">></td> <td style="text-align: center;">></td> <td style="text-align: center;">^--RT</td> </tr> <tr> <td>1 LT--^</td> <td style="text-align: center;">v</td> <td style="text-align: center;">v</td> <td style="text-align: center;">v</td> <td style="text-align: center;"><^--RTH</td> </tr> <tr> <td>LTH--^></td> <td></td> <td></td> <td></td> <td style="text-align: center;"><--TH</td> </tr> <tr> <td>TH--></td> <td></td> <td></td> <td></td> <td style="text-align: center;"><v--LTH</td> </tr> <tr> <td>RTH--v></td> <td style="text-align: center;">^</td> <td style="text-align: center;">^</td> <td style="text-align: center;">^</td> <td style="text-align: center;">v--LT</td> </tr> <tr> <td>1 RT--v</td> <td style="text-align: center;"><</td> <td style="text-align: center;">></td> <td style="text-align: center;">></td> <td style="text-align: center;">Approach 2</td> </tr> <tr> <td></td> <td style="text-align: center;">L</td> <td style="text-align: center;">L</td> <td style="text-align: center;">T</td> <td style="text-align: center;">R</td> </tr> <tr> <td></td> <td style="text-align: center;">T</td> <td style="text-align: center;">T</td> <td style="text-align: center;">H</td> <td style="text-align: center;">T</td> </tr> <tr> <td></td> <td style="text-align: center;">H</td> <td style="text-align: center;">H</td> <td></td> <td style="text-align: center;">ELDER CREEK R</td> </tr> <tr> <td></td> <td style="text-align: center;">1</td> <td style="text-align: center;">1</td> <td></td> <td></td> </tr> <tr> <td></td> <td colspan="4" style="text-align: center;">Approach 4:</td> </tr> </table>		1	1		^		R	L		N	ELDER CREEK R	R	T	T	L		T	H	H	T	Approach 1	<	>	>	^--RT	1 LT--^	v	v	v	<^--RTH	LTH--^>				<--TH	TH-->				<v--LTH	RTH--v>	^	^	^	v--LT	1 RT--v	<	>	>	Approach 2		L	L	T	R		T	T	H	T		H	H		ELDER CREEK R		1	1				Approach 4:				<table style="width:100%; 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<p>Step 2. IDENTIFY VOLUMES, in vph</p> <table style="width:100%; border-collapse: collapse;"> <tr> <td style="width:10%;"></td> <td style="width:10%;"></td> <td style="width:10%;"></td> <td style="width:10%;"></td> <td style="width:10%;"></td> <td style="width:10%;"></td> <td style="width:10%;"></td> <td style="width:10%;"></td> <td style="width:10%;"></td> <td style="width:10%;"></td> </tr> <tr> <td></td> <td style="text-align: center;">3</td> <td style="text-align: center;">1</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>3: LT= 0</td> <td style="text-align: center;">5</td> <td style="text-align: center;">0</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>TH= 100</td> <td style="text-align: center;">0</td> <td style="text-align: center;">0</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>RT= 350</td> <td style="text-align: center;"><</td> <td style="text-align: center;">v</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td></td> <td style="text-align: center;">v</td> <td style="text-align: center;"><</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td></td> <td colspan="4" style="text-align: center;"><--Approach 2</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td></td> <td colspan="4" style="text-align: center;">Approach 1--></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>1:LT= 330</td> <td style="text-align: center;">^</td> <td style="text-align: center;">4: RT= 0</td> <td style="text-align: center;">330</td> <td style="text-align: center;">-^</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>TH= 0</td> <td style="text-align: center;"> </td> <td style="text-align: center;">TH=1020</td> <td style="text-align: center;">10</td> <td style="text-align: center;">-v</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td>RT= 10</td> <td style="text-align: center;"> </td> <td style="text-align: center;">LT= 90</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> <tr> <td></td> <td colspan="4" style="text-align: center;">Approach 4</td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> </table>												3	1								3: LT= 0	5	0								TH= 100	0	0								RT= 350	<	v									v	<									<--Approach 2										Approach 1-->									1:LT= 330	^	4: RT= 0	330	-^						TH= 0		TH=1020	10	-v						RT= 10		LT= 90									Approach 4									<p>Step 5. 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SUM OF CRITICAL VOLUMES</p> <p>1020(B3A4)+330(B2)+0(+)+0(+)</p> <p>= 1350 vph</p> <p>Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)</p> <p style="text-align: center;">E</p> <p>Step 9. RECALCULATE</p> <p>Geometric Change:</p> <p>Signal Change:</p> <p>Volume Change:</p>																																																																											
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Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELDER CREEK RD AND EXCELSIOR RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY Approach 3: EXCELSIOR RD 1 1 ^ R L N ELDER CREEK R R T T T L T H H H T Approach 1 << >> ^--RT 1 LT--^ v v v <^--RTH LTH--> <--TH TH--> <v--LTH RTH--v ^ ^ ^ v--LT 1 RT--v << >> Approach 2 L L T R R T T H T T ELDER CREEK R H H 1 1 Approach 4:		Step 4. LEFT TURN CHECK -----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 1 0 d.Opposing volume : 0 70 250 0 in vph : e.LT capacity on : 0 0 950 0 green (vph) : f.LT capacity in : 0 0 950 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : NO NO (g>f) ? :		Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph ----- A4B3 20(B3) 250- 20= 230(A4) 20 A3A4 780(A3) OR 230(A4) 780 A1B2 310(B2) OR 49(A1) 310	
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 0 TH= 780 RT= 340 Approach 3 2:RT= 0 TH= 0 LT= 0 v <--Approach 2 Approach 1--> 1:LT= 310 TH= 0 RT= 70 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 3 7 ----- 4 8 0 0 < v < ^ 2 ----- 2 5 0 0		Step 7. SUM OF CRITICAL VOLUMES 800(B3A3)+310(B2)+0()+0() = 1110 vph ----- Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- C ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	
Step 3. IDENTIFY PHASING < ^ A4B3 ^ A3A4 v --^ A1B2 -->		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4		COMMENTS ----- Exclusive right turns reduced 30 % V/C Ratio = .78	
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >					

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND ELK GROVE-FLORIN RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:ELK GROVE-FLO 1 3 2 ^ R L N FLORIN RD R T T T L T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--^> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 2 3 1 Approach 4:ELK GROVE-FLO		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Volume Critical to next Volume in vph B2B1 39(B1) 308- 39= 269(B2) 39 A1B2 269(B2) 167- 269= 0(A1) 269 A1A2 487(A2) OR 0(A1) 487 B4B3 28(B4) 275- 28= 247(B3) 28 A4B3 247(B3) 943- 247= 696(A4) 247 A3A4 696(A4) OR 320(A3) 696			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 50 Approach 3 2:RT= 460 TH= 960 TH=1460 RT= 210 v LT= 70 <--Approach 2 Approach 1--> 1:LT= 560 ^ 4: RT= 80 TH= 500 TH=2830 RT= 220 LT= 500 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 2 3 3 3 1 2 2 2 2 0 0 0 3 8 < v v v > 308 -^ 252 -^ 167 -> 167 -> << ^ ^ ^ ^ > 167 -> 220 -v 2 2 9 9 9 7 2 4 4 4 8 5 5 3 3 3 0				Step 7. SUM OF CRITICAL VOLUMES 308(B1B2)+487(A2)+275(B4B3)+696(A4) = 1766 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4 COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.28							

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND ELK GROVE-FLORIN RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE-FLO 1 3 2 ^ R L N RT TT L TH HHT Approach 1 << >> ^-RT 1 2 LT--^ v v v <^-RTH LTH-^> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 2 3 1 Approach 4: ELK GROVE-FLO		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 61(B1) 165- 61= 104(B2) 61 A1B2 104(B2) 443- 104= 339(A1) 104 A1A2 339(A1) OR 227(A2) 339 B4B3 171(B3) 204- 171= 33(B4) 171 A3B4 33(B4) 757- 33= 724(A3) 33 A3A4 724(A3) OR 507(A4) 724			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 370 Approach 3 TH=2270 2:RT= 80 RT= 510 v TH= 680 LT= 110 <--Approach 2 Approach 1--> 1:LT= 300 ^ 4: RT= 120 TH=1330 TH=1520 RT= 530 LT= 310 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 5 7 7 7 1 2 ----- 1 5 5 5 6 0 0 7 7 7 7 4 < v v v > 165 -^ 135 -^ 443 -> 443 -> < < ^ ^ ^ > 443 -> 530 -v ----- 1 1 5 5 5 1 7 4 0 0 0 2 1 0 7 7 7 0				Step 7. SUM OF CRITICAL VOLUMES 165(B1B2)+339(A1)+204(B3B4)+724(A3) = 1432 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v ----- A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.04			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND WATERMAN RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: WATERMAN RD R L N R T T T L T H H H T Approach 1 << >> ^--RT LT--^ v v v <^--RTH LTH--> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 2 1 Approach 4: WATERMAN RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 1 0 d.Opposing volume : 490 0 670 0 in vph : e.LT capacity on : 0 0 530 0 green (vph) : f.LT capacity in : 0 0 530 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : NO NO : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph A2B1 165(B1) 163- 165= 0(A2) 165 A1A2 93(A1) OR 0(A2) 93 A4B3 622(B3) OR 469(A4) 622			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 0 Approach 3 2:RT= 0 TH= 0 TH= 490 RT= 0 v LT= 300 <--Approach 2 Approach 1--> 1:LT= 0 ^ 4: RT= 670 TH= 280 TH= 0 RT= 90 LT=1130 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph <- 163 <- 163 <- 163 v- 135 v- 165 93 -> 93 -> <<> 93 -> 90 -v 6 5 6 ----- 2 0 7 2 9 0				Step 7. SUM OF CRITICAL VOLUMES 258(B1A1)+622(B3)+0()+0() = 880 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) B Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING <-- A2B1 v-- --> <-- A1A2 < ^ A4B3 		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .62			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND WATERMAN RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: WATERMAN RD R L N R T T T L T H H H T Approach 1 << >> ^--RT LT--^ v v v <^--RTH LTH--> <--TH 3 3 TH--> <v--LTH RTH--v ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 2 1 Approach 4: WATERMAN RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 1 0 d.Opposing volume : 1620 0 390 0 in vph : e.LT capacity on : 0 0 810 0 green (vph) : f.LT capacity in : 0 0 810 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : NO NO (g>f) ? :				Prob- Possible Volume Adjusted able Critical Carryover Critical Phase Volume to next Volume in vph phase in vph ----- A2B1 253(B1) 540- 253= 287(A2) 253 A1A2 749(A1) OR 287(A2) 749 A4B3 273(A4) OR 160(B3) 273			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 0 Approach 3 2:RT= 0 TH= 0 TH=1620 RT= 0 v LT= 460 ----- <--Approach 2 ----- Approach 1--> 1:LT= 0 ^ 4: RT= 390 TH=1380 TH= 0 RT=1070 LT= 290 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph ----- <- 540 <- 540 <- 540 v- 207 v- 253 ----- 460 -> 460 -> << > 460 -> 1070 -v ----- 1 1 3 6 3 9 0 1 0				Step 7. SUM OF CRITICAL VOLUMES 1002(B1A1)+273(A4)+0()+0() = 1275 vph ----- Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- D ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING <-- A2B1 v-- --> <-- A1A2 < ^ A4B3 		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS ----- Exclusive right turns reduced 30 % V/C Ratio = .89			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND BRADSHAW RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW RD 1 3 2 ^ R L N FLORIN RD R T T T L T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH-^> <--TH 3 3 TH--> <v--LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 2 3 1 Approach 4:BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- Possible Volume Adjusted able Critical Carryover Critical Phase in vph to next Volume in vph phase in vph B2B1 55(B1) 407- 55= 352(B2) 55 A1B2 352(B2) 42- 352= 0(A1) 352 A1A2 525(A2) OR 0(A1) 525 B4B3 6(B4) 451- 6= 445(B3) 6 A4B3 445(B3) 630- 445= 185(A4) 445 A3A4 223(A3) OR 185(A4) 223			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 10 Approach 3 2:RT= 750 TH= 670 TH= 750 RT= 280 v LT= 100 <--Approach 2 407 -^ 333 -^ 37 -> 37 -> 37 -> 60 -v Approach 1--> 1:LT= 740 ^ 4: RT= 70 TH= 110 TH=1890 RT= 60 LT= 820 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 2 2 2 2 8 2 2 2 0 3 3 3 5 6 < v v v v > ^- 750 <- 250 <- 250 v- 45 v- 55				Step 7. SUM OF CRITICAL VOLUMES 407(B1B2)+525(A2)+451(B4B3)+223(A3) = 1606 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.17			
Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND BRADSHAW RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD 1 3 2 ^ R L N RT TT L TH HHT Approach 1 << >> ^-RT 1 2 LT--^ v v v <^-RTH LTH-^> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 2 3 1 Approach 4: BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph B2B1 198(B2) 297- 198= 99(B1) 198 A2B1 99(B1) 63- 99= 0(A2) 99 A1A2 371(A1) OR 0(A2) 371 B4B3 77(B3) 220- 77= 143(B4) 77 A3B4 143(B4) 763- 143= 620(A3) 143 A3A4 620(A3) OR 330(A4) 620			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
3: LT= 400 Approach 3 TH=2290 2:RT= 60 RT= 590 v TH= 190 LT= 540 <--Approach 2 Approach 1-->		5 7 7 7 1 2 9 6 6 6 8 2 0 3 3 3 0 0 < v v v > 198 -^ 162 -^ 273 -> 273 -> 273 -> 530 -v 3 3 3 7 6 3 3 3 6 7 3 0 0 0 0		297(B2B1)+371(A1)+220(B3B4)+620(A3) = 1508 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	
1:LT= 360 ^ 4: RT= 60 TH= 820 TH= 990 RT= 530 LT= 140 Approach 4					

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND VINEYARD RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: VINEYARD RD R L ^ N FLORIN RD R T T T L T H H H T Approach 1 << >> ^--RT LT--^ v v v <^--RTH LTH-^> <--TH 3 3 TH--> <v--LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 2 1 Approach 4: VINEYARD RD		-----Approach----- : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 1 0 d.Opposing volume in vph : 630 0 910 0 e.LT capacity on green (vph) : 0 0 290 0 f.LT capacity in vph (b+e) : 0 0 290 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? : NO NO :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph A2B1 17(B1) 210- 17= 193(A2) 17 A1A2 193(A2) OR 37(A1) 193 A4B3 637(A4) OR 534(B3) 637			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 0 Approach 3 2:RT= 0 TH= 0 TH= 630 RT= 0 v LT= 30 <--Approach 2 Approach 1--> 1:LT= 0 ^ 4: RT= 910 TH= 110 TH= 0 RT= 10 LT= 970 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph <- 210 <- 210 <- 210 v- 14 v- 17 37 -> 37 -> 37 -> << >> 10 -v 5 4 9 3 3 1 4 7 0				Step 7. SUM OF CRITICAL VOLUMES 210(B1A2)+637(A4)+0()+0() = 847 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) A Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING <-- A2B1 v-- --> <-- A1A2 < ^ A4B3 		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .59			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND VINEYARD RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: VINEYARD RD FLORIN RD: R L N R T T T L T H H H T Approach 1: << >> ^--RT LT--^ v v v <^--RTH LTH-^> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 2 1 Approach 4: VINEYARD RD		-----Approach----- : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 1 0 d.Opposing volume in vph : 670 0 190 0 e.LT capacity on green (vph) : 0 0 1010 0 f.LT capacity in vph (b+e) : 0 0 1010 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? : NO NO				Prob-able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph A2B1 110(B1) 223- 110= 113(A2) 110 A1A2 518(A1) OR 113(A2) 518 A4B3 133(A4) OR 44(B3) 133			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 0 TH= 0 RT= 0 Approach 3 2: RT= 0 TH= 670 LT= 200 v <--Approach 2 Approach 1--> 1: LT= 0 TH= 490 RT= 740 Approach 4 4: RT= 190 TH= 0 LT= 80		Step 5. ASSIGN LANE VOLUMES, in vph <- 223 <- 223 <- 223 v- 90 v- 110 163 -> 163 -> 163 -> 740 -v <<> 1 4 3 9 4 6 0				Step 7. SUM OF CRITICAL VOLUMES 628(B1A1)+133(A4)+0()+0() = 761 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) A Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING <-- A2B1 v-- --> <-- A1A2 < ^ A4B3 		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .53			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND EXCELSIOR RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: EXCELSIOR RD 1 1 1 ^ R L N R T T T L T H H H T Approach 1 < < > > ^--RT 1 1 LT--^ v v v <^--RTH LTH--> <--TH 2 2 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 1 1 RT--v < < > > Approach 2 L L T R R T T H T T FLORIN RD H H 1 1 1 Approach 4: EXCELSIOR RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- Possible Volume Adjusted able Critical Carryover Critical Phase in vph to next Volume in vph in vph B2B1 50(B1) 340- 50= 290(B2) 50 A1B2 290(B2) 350- 290= 60(A1) 290 A1A2 285(A2) OR 60(A1) 285 B4B3 0(B4) 40- 0= 40(B3) 0 A4B3 40(B3) 710- 40= 670(A4) 40 A3A4 670(A4) OR 42(A3) 670			
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
3: LT= 0 Approach 3 2:RT= 10 TH= 30 TH= 570 RT= 60 v LT= 50 <--Approach 2 Approach 1--> 1:LT= 340 ^ 4: RT= 140 TH= 700 TH= 710 RT= 10 LT= 40 Approach 4		6 3 0 0 0 < v > 340 -^ 350 -> 350 -> < ^ > 10 -v 7 1 4 1 4 0 0 0				340(B1B2)+285(A2)+40(B4B3)+670(A4) = 1335 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) E			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				Exclusive right turns reduced 30 % V/C Ratio = .97			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND ELK GROVE-FLORIN RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP				
Approach 3: ELK GROVE-FLO 1 3 2 ^ R L N R T T L T H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH-^> <--TH 2 2 TH--> <v--LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R GERBER RD H H 2 3 1 Approach 4: ELK GROVE-FLO		a.No. of change intervals/hour	: -1-	-2-	-3-	-4-	Prob-able Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph
		b.LT capacity on change (vph)	: 0	0	0	0	B2B1	28(B1) 154-	28= 126(B2)	28
		c.G/C ratio	: 0	0	0	0	A1B2	126(B2) 215-	126= 89(A1)	126
		d.Opposing volume in vph	: 0	0	0	0	A1A2	500(A2) OR	89(A1)	500
		e.LT capacity on green (vph)	: 0	0	0	0	B4B3	127(B4) 440-	127= 313(B3)	127
		f.LT capacity in vph (b+e)	: 0	0	0	0	A4B3	313(B3) 1027-	313= 714(A4)	313
		g.Left turn volume in vph	: 0	0	0	0	A3A4	714(A4) OR	323(A3)	714
		h.Is volume > cap. (g>f) ?	:	:	:	:				
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES				
3: LT= 230 Approach 3 2:RT= 320 TH= 970 TH=1000 RT= 140 v LT= 50 <--Approach 2 Approach 1--> 1:LT= 280 ^ 4: RT= 110 TH= 430 TH=3080 RT= 300 LT= 800 Approach 4		1 3 3 3 1 1 4 2 2 2 0 2 0 3 3 3 4 7 < v v v >				154(B1B2)+500(A2)+440(B4B3)+714(A4) = 1808 vph				
		154 ^ 126 ^ 215 -> << ^ ^ ^ > 215 -> 300 -v 4 3 0 0 0 1 4 6 2 2 2 1 0 0 7 7 7 0				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F				
						Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:				
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS				
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				Exclusive right turns reduced 30 % V/C Ratio = 1.31				
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >										

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND ELK GROVE-FLORIN RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE-FLO 1 3 2 ^ R L N R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph			
GERBER RD Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--> <--TH 2 2 TH--> <v--LTH RTH--v ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 2 3 1 Approach 4: ELK GROVE-FLO						B2B1 66(B1) 77- 66= 11(B2) 66 A1B2 11(B2) 595- 11= 584(A1) 11 A1A2 584(A1) OR 285(A2) 584 B4B3 193(B3) 237- 193= 44(B4) 193 A3B4 44(B4) 800- 44= 756(A3) 44 A3A4 756(A3) OR 507(A4) 756			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES			
3: LT= 430 TH=2400 RT= 270 Approach 3 2:RT= 360 TH= 570 LT= 120 <--Approach 2		2 8 8 8 1 2 7 0 0 0 9 3 0 0 0 0 4 7 < v v v >		77 ^ 63 ^ 460 -> 460 -> 850 -v << ^ ^ ^ ^ > 1 1 5 5 5 9 5 0 0 0 6 3 8 7 7 7 0		77(B1B2)+584(A1)+237(B3B4)+756(A3) = 1654 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F	
Approach 1--> 1:LT= 140 TH= 920 RT= 850 Approach 4 4: RT= 60 TH=1520 LT= 350				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			

Step 3. IDENTIFY PHASING	Step 6a. CRITICAL VOLUMES, in vph (two phase signal)	COMMENTS
--^ v-- B2B1 --- AND <-- A1B2 AND --> OR v-- /OR A2B1 ---> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v	Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4	Exclusive right turns reduced 30 % V/C Ratio = 1.2
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND WATERMAN RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: WATERMAN RD 1 2 2 ^ R L N R T T T L T H H H T		Approach : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Volume Critical Volume to next Volume in vph phase in vph B2B1 11(B1) 105- 11= 94(B2) 11 A1B2 94(B2) 195- 94= 101(A1) 94 A1A2 375(A2) OR 101(A1) 375 B4B3 77(B4) 99- 77= 22(B3) 77 A4B3 22(B3) 635- 22= 613(A4) 22 A3A4 613(A4) OR 110(A3) 613			
GERBER RD Approach 1 < < > > ^--RT 1 2 LT--^ v v v <^--RTH LTH--> <--TH 2 2 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 2 1 RT--v < < > > Approach 2 L L T R R T T H T T GERBER RD H H 2 2 1 Approach 4: WATERMAN RD									
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
3: LT= 140 Approach 3 2:RT= 340 TH= 220 TH= 750 RT= 40 v LT= 20 <--Approach 2		1 1 4 1 1 6 7 0 0 0 3 7 < v v > >				105(B1B2)+375(A2)+99(B4B3)+613(A4) = 1192 vph			
Approach 1--> 1:LT= 190 ^ 4: RT= 400 TH= 390 TH=1270 RT= 70 LT= 180 Approach 4		105 -^ 86 -^ 195 -> 195 -> < < ^ ^ > 70 -v 6 6 4 9 8 3 3 0 9 1 5 5 0				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) D			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				Exclusive right turns reduced 30 % V/C Ratio = .87			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND WATERMAN RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: WATERMAN RD 1 2 2 ^ R L N R T T L T H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH-^> <--TH 2 2 TH--> <v--LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 2 2 1 Approach 4: WATERMAN RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph B2B1 94(B2) 121- 94= 27(B1) 94 A2B1 27(B1) 275- 27= 248(A2) 27 A1A2 420(A1) OR 248(A2) 420 B4B3 28(B3) 83- 28= 55(B4) 28 A3B4 55(B4) 550- 55= 495(A3) 55 A3A4 495(A3) OR 220(A4) 495			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 150 Approach 3 2:RT= 80 TH=1100 TH= 550 RT= 290 v LT= 220 <--Approach 2 Approach 1--> 1:LT= 170 ^ 4: RT= 200 TH= 840 TH= 440 RT= 300 LT= 50 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 2 5 5 9 5 5 6 8 0 0 0 8 3 < v v >> 94 ^ 77 ^ 420 -> 420 -> 300 -v << ^ ^ ^ > 2 2 2 2 2 2 2 0 8 3 0 0 0				Step 7. SUM OF CRITICAL VOLUMES 121(B2B1)+420(A1)+83(B3B4)+495(A3) = 1119 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) D Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4 Exclusive right turns reduced 30 % V/C Ratio = .81							

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND BRADSHAW RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW RD 1 3 2 ^ R L N GERBER RD R T T L T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--> <--TH 2 2 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD R H 2 3 1 Approach 4:BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Phase in vph to next phase in vph B2B1 28(B1) 187- 28= 159(B2) 28 A1B2 159(B2) 115- 159= 0(A1) 159 A1A2 235(A2) OR 0(A1) 235 B4B3 22(B4) 88- 22= 66(B3) 22 A4B3 66(B3) 717- 66= 651(A4) 66 A3A4 651(A4) OR 193(A3) 651			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 40 Approach 3 2:RT= 220 TH= 580 TH= 470 RT= 200 v LT= 50 <--Approach 2 Approach 1--> 1:LT= 340 ^ 4: RT= 60 TH= 230 TH=2150 RT= 50 LT= 160 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 2 1 1 1 0 9 9 1 2 0 3 3 3 8 2 < v v v v > ^- 220 <- 235 <- 235 v- 23 v- 28 187 -^ 153 -^ 115 -> 115 -> << ^ ^ ^ ^ > 50 -v 7 7 7 8 7 1 1 1 6 8 2 7 7 7 0				Step 7. SUM OF CRITICAL VOLUMES 187(B1B2)+235(A2)+88(B4B3)+651(A4) = 1161 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) D Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4 COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .84							

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND BRADSHAW RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD 1 3 2 ^ R L N GERBER RD R T T T L T H H H T Approach 1 << >> ^-RT 1 2 LT--^ v v v <^-RTH LTH-^> <--TH 2 2 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 2 3 1 Approach 4: BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 28(B1) 154- 28= 126(B2) 28 A1B2 126(B2) 230- 126= 104(A1) 126 A1A2 130(A2) OR 104(A1) 130 B4B3 39(B3) 83- 39= 44(B4) 39 A3B4 44(B4) 893- 44= 849(A3) 44 A3A4 849(A3) OR 260(A4) 849			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3 3: LT= 150 2:RT= 80 TH=2680 TH= 260 RT= 500 v LT= 50 <--Approach 2		5 8 8 8 0 9 9 6 8 0 3 3 8 3 < v v v > 154 ^- 126 ^- 230 -> 230 -> 220 -v 2 2 2 3 3 6 6 6 9 2 0 0 0		154(B1B2)+130(A2)+83(B3B4)+849(A3) = 1216 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) D	
Approach 1--> 1:LT= 280 ^ 4: RT= 60 TH= 460 TH= 780 RT= 220 LT= 70 Approach 4				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND VINEYARD RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: VINEYARD RD 1 2 2 ^ R L N GERBER RD R T T T L T H H H T Approach 1 < < > > ^--RT 1 2 LT--^ v v v <^--RTH LTH--^> <--TH 2 2 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 2 1 RT--v < < > > Approach 2 L L T R R T T H T T GERBER RD H H 2 2 1 Approach 4: VINEYARD RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Phase in vph to next phase in vph B2B1 6(B1) 264- 6= 258(B2) 6 A1B2 258(B2) 80- 258= 0(A1) 258 A1A2 126(A2) OR 0(A1) 126 B4B3 6(B4) 55- 6= 49(B3) 6 A4B3 49(B3) 600- 49= 551(A4) 49 A3A4 551(A4) OR 112(A3) 551			
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
3: LT= 10 Approach 3 TH= 10 2:RT= 180 RT= 160 v TH= 170 LT= 10 <--Approach 2 Approach 1--> 1:LT= 480 ^ 4: RT= 10 TH= 160 TH=1200 RT= 20 LT= 100 Approach 4		1 ----- 6 0 5 5 5 6 < v v v > ^- 180 <- 85 <- 85 v- 5 v- 6 264 -^ 216 -^ 80 -> 80 -> < < ^ ^ > 20 -v 6 6 5 4 0 0 1 5 5 0 0 0				264(B1B2)+126(A2)+55(B4B3)+551(A4) = 996 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) C			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				Step 9. RECALCULATE			
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Approach 4				Exclusive right turns reduced 30 % V/C Ratio = .72			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND VINEYARD RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: VINEYARD RD 1 2 2 ^ R L N R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 6(B1) 94- 6= 88(B2) 6 A1B2 88(B2) 65- 88= 0(A1) 88 A1A2 60(A2) OR 0(A1) 60 B4B3 17(B3) 94- 17= 77(B4) 17 A3B4 77(B4) 275- 77= 198(A3) 77 A3A4 198(A3) OR 30(A4) 198			
Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--^ <--TH 2 2 TH--> <v--LTH RTH--v ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 2 2 1 Approach 4: VINEYARD RD									

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3 3: LT= 170 2:RT= 50 TH= 550 TH= 120 RT= 225 v LT= 10 <--Approach 2		2 2 2 2 7 7 7 9 5 5 5 7 4 < v v >>		94(B1B2)+60(A2)+94(B3B4)+198(A3) = 446 vph	
Approach 1--> 1:LT= 170 ^ 4: RT= 10 TH= 130 TH= 60 RT= 70 LT= 30 Approach 4		94 -^ 77 -^ 65 -> 65 -> << ^ ^ ^ > 70 -v		Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) A	
		1 1 3 3 1 7 4 0 0 0		Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND EXCELSIOR RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: EXCELSIOR RD 1 1 1 ^ R L N GERBER RD R T T T L T H H H T Approach 1 << >> ^--RT 1 1 LT--^ v v v <^--RTH LTH--^> <--TH 1 1 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 1 1 1 Approach 4: EXCELSIOR RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- Possible Critical Adjusted able Volume Carryover Critical Phase in vph to next Volume in vph in vph ----- B2B1 10(B1) 220- 10= 210(B2) 10 A1B2 210(B2) 10- 210= 0(A1) 210 A1A2 10(A2) OR 0(A1) 10 B4B3 10(B4) 20- 10= 10(B3) 10 A4B3 10(B3) 660- 10= 650(A4) 10 A3A4 650(A4) OR 70(A3) 650			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 10 Approach 3 2:RT= 10 TH= 70 TH= 10 RT= 60 v LT= 10 ----- <--Approach 2 ----- Approach 1--> ----- 1:LT= 220 ^ 4: RT= 10 TH= 10 TH= 660 RT= 10 LT= 20 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph ----- 6 7 1 0 0 0 < v > ----- 220 -^ < ^ > 10 -> 10 -v ----- 6 ----- 2 6 1 0 0 0				Step 7. SUM OF CRITICAL VOLUMES ----- 220(B1B2)+10(A2)+20(B4B3)+650(A4) ----- = 900 vph ----- Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- B ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v ----- A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS ----- Exclusive right turns reduced 30 % V/C Ratio = .65			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND EXCELSIOR RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: EXCELSIOR RD 1 1 1 ^ R L N GERBER RD R T T T L T H H H T Approach 1 << >> ^--RT 1 1 LT--^ v v v <^--RTH LTH--^> <--TH 1 1 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 1 1 1 Approach 4: EXCELSIOR RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph B2B1 10(B2) 10- 10= 0(B1) 10 A2B1 0(B1) 10- 0= 10(A2) 0 A1A2 35(A1) OR 10(A2) 35 B4B3 10(B4) 10- 10= 0(B3) 10 A4B3 0(B3) 210- 0= 210(A4) 0 A3A4 700(A3) OR 210(A4) 700			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 10 Approach 3 2:RT= 10 TH= 700 TH= 10 RT= 150 v LT= 10 <--Approach 2 Approach 1--> 1:LT= 10 ^ 4: RT= 10 TH= 10 TH= 210 RT= 50 LT= 10 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 1 7 5 0 1 0 0 0 < v > 10 ^> 10 -> 50 -v < ^ > 2 1 1 1 0 0 0				Step 7. SUM OF CRITICAL VOLUMES 10(B2B1)+35(A1)+10(B4B3)+700(A3) = 755 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) A Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .55			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELK GROVE FLORIN AND CALVINE
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE FLO 1 3 2 ^ R L N CALVINE RD R T T T L T H H H T Approach 1 < < > > ^--RT 1 2 LT--^ v v v <^--RTH LTH--> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v < < > > Approach 2 L L T R R CALVINE RD T T H T T H H 2 3 1 Approach 4: ELK GROVE FLO		Approach : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 176(B1) 253- 176= 77(B2) 176 A1B2 77(B2) 253- 77= 176(A1) 77 A1A2 377(A2) OR 176(A1) 377 B4B3 50(B4) 242- 50= 192(B3) 50 A4B3 192(B3) 530- 192= 338(A4) 192 A3A4 338(A4) OR 308(A3) 338			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 90 Approach 3 2:RT= 100 TH= 870 TH=1130 RT= 440 v LT= 320 <--Approach 2 Approach 1--> 1:LT= 460 ^ 4: RT= 480 TH= 760 TH=1590 RT= 210 LT= 440 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 4 2 2 2 4 9 9 4 5 0 0 0 0 1 0 < v v v > 253 -^ 207 -^ 253 -> 253 -> < < ^ ^ ^ > 253 -> 210 -v 2 1 5 5 4 4 9 3 3 8 2 8 0 0 0				Step 7. SUM OF CRITICAL VOLUMES 253(B1B2)+377(A2)+242(B4B3)+338(A4) = 1210 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) D			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .88			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELK GROVE FLORIN AND CALVINE
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP					
Approach 3: ELK GROVE FLO 1 3 2 ^ R L N R T T L T H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--> <--TH 3 3 TH--> <v--LTH RTH--v ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R CALVINE RD T T H T T H H 2 3 1 Approach 4: ELK GROVE FLO		a.No. of change intervals/hour	: -1- -2- -3- -4-	0	0	0	0	Prob-able Phase	Possible Critical Volume in vph	Volume Carryover to next phase	Adjusted Critical Volume in vph
		b.LT capacity on change (vph)	:	0	0	0	0	B2B1	275(B2) 286- 275=	11(B1) 275	
		c.G/C ratio	:	0	0	0	0	A2B1	11(B1) 337- 11=	326(A2) 11	
		d.Opposing volume in vph	:	0	0	0	0	A1A2	400(A1) OR 326(A2)	400	
		e.LT capacity on green (vph)	:	0	0	0	0	B4B3	55(B4) 171- 55=	116(B3) 55	
		f.LT capacity in vph (b+e)	:	0	0	0	0	A4B3	116(B3) 393- 116=	277(A4) 116	
		g.Left turn volume in vph	:	0	0	0	0	A3A4	507(A3) OR 277(A4)	507	
		h.Is volume > cap. (g>f) ?	:								

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
3: LT= 100 TH=1520 RT= 490 Approach 3 2:RT= 80 TH=1010 LT= 520 <--Approach 2		4 5 5 5 9 0 0 4 5 0 7 7 7 5 < v v v >		^- 80 <- 337 <- 337 <- 337 v- 234 v- 286	
Approach 1--> 1:LT= 500 TH=1200 RT= 460 Approach 4 4: RT= 350 TH=1180 LT= 310		275 -^ 225 -^ 400 -> 400 -> 400 -> 400 -> 460 -v << ^ ^ ^ ^ > 1 1 3 3 3 3 7 4 9 9 9 5 1 0 3 3 3 0		286(B2B1)+400(A1)+171(B4B3)+507(A3) = 1364 vph	
Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) E					
Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:					

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				Exclusive right turns reduced 30 % V/C Ratio = .99

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: WATERMAN AND CALVINE
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: WATERMAN RD 1 2 2 ^ R L N CALVINE RD R T T T L T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--> <--TH 3 3 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R CALVINE RD T T H T T H H 2 2 1 Approach 4: WATERMAN RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Prob- Critical Carryover Critical able Volume to next Volume Phase in vph phase in vph ----- B2B1 94(B2) 94- 94= 0(B1) 94 A2B1 0(B1) 367- 0= 367(A2) 0 A1A2 367(A2) OR 363(A1) 367 B4B3 44(B3) 72- 44= 28(B4) 44 A3B4 28(B4) 215- 28= 187(A3) 28 A3A4 555(A4) OR 187(A3) 555			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 130 Approach 3 2:RT= 160 TH= 430 TH=1100 RT= 180 v LT= 170 <--Approach 2 Approach 1--> 1:LT= 170 ^ 4: RT= 190 TH=1090 TH=1110 RT= 120 LT= 80 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 1 2 2 8 1 1 5 7 0 5 5 9 2 < v v >> 94 -^ 77 -^ 363 -> 363 -> < < ^ ^ > 363 -> 120 -v 5 5 1 4 3 5 5 9 4 6 5 5 0				Step 7. SUM OF CRITICAL VOLUMES 94(B2B1)+367(A2)+72(B3B4)+555(A4) = 1088 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) C			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .79			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: WATERMAN AND CALVINE

Design Hour: PM

Problem Statement: CUMULATIVE NO PROJECT

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: WATERMAN RD 1 2 2 ^ R L N CALVINE RD R T T T L T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH-> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T CALVINE RD H H 2 2 1 Approach 4: WATERMAN RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 110(B2) 143- 110= 33(B1) 110 A2B1 33(B1) 430- 33= 397(A2) 33 A1A2 397(A2) OR 370(A1) 397 B4B3 61(B4) 88- 61= 27(B3) 61 A4B3 27(B3) 300- 27= 273(A4) 27 A3A4 465(A3) OR 273(A4) 465			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
3: LT= 110 Approach 3 TH= 930 2:RT= 160 RT= 180 v TH=1290 LT= 260 <--Approach 2 Approach 1--> 1:LT= 200 ^ 4: RT= 210 TH=1110 TH= 600 RT= 210 LT= 160 Approach 4		1 4 4 8 6 6 5 6 0 5 5 0 1 < v v >> 110 -^ 90 -^ 370 -> 370 -> 370 -> 210 -v 3 3 2 8 7 0 0 1 8 2 0 0 0		143(B2B1)+397(A2)+88(B4B3)+465(A3) = 1093 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) C Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: CALVINE RD AND BRADSHAW RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW 1 3 2 ^ R L N CALVINE RD R T T T L T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--> <--TH 3 3 TH--> <v-LTH RTH--v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T CALVINE RD H H 2 3 1 Approach 4:BRADSHAW		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Prob- Critical Carryover Critical able Volume to next Volume Phase in vph phase in vph B2B1 28(B1) 446- 28= 418(B2) 28 A1B2 418(B2) 130- 418= 0(A1) 418 A1A2 245(A2) OR 0(A1) 245 B4B3 44(B4) 55- 44= 11(B3) 44 A4B3 11(B3) 380- 11= 369(A4) 11 A3A4 369(A4) OR 280(A3) 369			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 80 Approach 3 2:RT= 350 TH= 600 TH= 550 RT= 400 v LT= 50 <--Approach 2 Approach 1--> 1:LT= 810 ^ 4: RT= 70 TH= 390 TH=1140 RT= 50 LT= 100 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 4 2 2 2 0 0 0 0 3 4 0 0 0 0 6 4 < v v v > 446 ^ 365 ^ 130 -> 130 -> 130 -> 50 -v << ^ ^ ^ ^ > 3 3 3 5 4 8 8 8 7 5 5 0 0 0 0				Step 7. SUM OF CRITICAL VOLUMES 446(B1B2)+245(A2)+55(B4B3)+369(A4) = 1115 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) D Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .81			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: CALVINE RD AND BRADSHAW RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW		-----Approach-----				Possible Volume Adjusted			
1 3 2 ^		: -1- -2- -3- -4-				Prob- able Critical Carryover Critical			
R L N		a.No. of change : 0 0 0 0				Phase in vph to next phase in vph			
CALVINE RD R T T T L		b.LT capacity on : 0 0 0 0				B2B1 17(B1) 270- 17= 253(B2) 17			
T H H H T		c.G/C ratio : 0 0 0 0				A1B2 253(B2) 173- 253= 0(A1) 253			
Approach 1 << >> ^-RT 1		d.Opposing volume : 0 0 0 0				A1A2 157(A2) OR 0(A1) 157			
2 LT--^ v v v <^-RTH		e.LT capacity on : 0 0 0 0				B4B3 17(B3) 154- 17= 137(B4) 17			
LTH-^>		f.LT capacity in : 0 0 0 0				A3B4 137(B4) 616- 137= 479(A3) 137			
3 TH-->		g.Left turn volume : 0 0 0 0				A3A4 479(A3) OR 257(A4) 479			
RTH-v> ^ ^ ^ v--LT 2		h.Is volume > cap. : (g>f) ?							
1 RT--v << >> Approach 2									
L L T R R									
T T H T T CALVINE RD									
H H									
2 3 1									
Approach 4:BRADSHAW									

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		8 4 4 4 1 1		270(B1B2)+157(A2)+154(B3B4)+479(A3)	
3: LT= 280		8 1 1 1 2 5		= 1060 vph	
TH=1240		0 3 3 3 6 4			
RT= 880 v		< v v v >		Step 8. INTERSECTION LEVEL OF SERVICE	
				(compare step 7 with table 6)	
<--Approach 2		270 ^		C	
		221 ^			
		173 ->			
		173 ->			
Approach 1-->		< < ^ ^ ^ >		Step 9. RECALCULATE	
		173 ->		Geometric Change:	
1:LT= 49D ^				Signal Change:	
TH= 520		2 2 2		Volume Change:	
RT= 70		1 1 5 5 4			
Approach 4		7 4 7 7 7 0			
4: RT= 40					
TH= 770					
LT= 30					

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1		Approach 3		
--^ AND <-- A1B2 AND		Approach 1		
--> OR v-- /OR A2B1		See Step 6b.		
--> <-- A1A2		Approach 2		
< B4B3				
>				
AND < ^ A3B4 AND				
v > OR /OR A4B3				
^ A3A4				
v				
A1 --> A3 B1 v-- B3 <		Approach 4		Exclusive right turns reduced 30 % V/C Ratio = .77
A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: CALVINE RD AND VINEYARD RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: VINEYARD 1 2 ^ R L N R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 410 0 210 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : NO NO (g>f) ? :				Possible Volume Adjusted Critical Volume to next Critical Phase in vph phase in vph A1B2 556(B2) 137- 556= 0(A1) 556 A1A2 168(A2) OR 0(A1) 168 A3B4 147(A3) OR 17(B4) 147			
Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--> <--TH 3 3 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT RT--v << >> Approach 2 L L T R R T T H T T CALVINE RD H H Approach 4: VINEYARD									
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 30 Approach 3 2:RT= 240 TH= 0 TH= 380 RT= 210 v LT= 0 <--Approach 2 Approach 1--> 1:LT=1010 ^ 4: RT= 0 TH= 410 TH= 0 RT= 0 LT= 0 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 2 1 1 1 0 4 7 < > >				Step 7. SUM OF CRITICAL VOLUMES 724(B2A2)+147(A3)+0()+0() = 871 vph			
						Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) B			
						Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ A1B2 --> --> <-- A1A2 A3B4 v >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .61			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: CALVINE RD AND VINEYARD RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: VINEYARD 1 2 ^ R L N CALVINE RD R T T T L T H H H T Approach 1 < < > > ^-RT 1 2 LT--^ v v v <^-RTH LTH-^> <--TH 3 3 TH--> <v-LTH RTH-v^ ^ ^ v--LT RT--v < < > > Approach 2 L L T R R T T H T T CALVINE RD H H Approach 4: VINEYARD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 380 0 800 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : NO NO: (g>f) ? :				Prob- Possible Volume Adjusted able Critical Carryover Critical Phase Volume to next Volume in vph phase in vph ----- A1B2 121(B2) 127- 121= 6(A1) 121 A1A2 153(A2) OR 6(A1) 153 A3B4 560(A3) OR 88(B4) 560			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3 3: LT= 160 2:RT= 40 TH= 0 TH= 460 RT= 800 v LT= 0 <--Approach 2 Approach 1--> 1:LT= 220 ^ 4: RT= 0 TH= 380 TH= 0 RT= 0 LT= 0 Approach 4		8 0 7 8 0 2 8 < > >		274(B2A2)+560(A3)+0()+0() = 834 vph	
				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- A -----	
				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ A1B2 --> --> <-- A1A2 A3B4 v >		Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: CALVINE RD AND EXCELSIOR RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:EXCELSIOR 1 1 1 ^ R L N CALVINE RD R T T T L T H H H T Approach 1 << >> ^--RT 1 LT--^ v v v <^--RTH 1 LTH-^> <--TH 2 2 TH--> <v-LTH 1 RTH-v> ^ ^ ^ v--LT 1 RT--v << >> Approach 2 L L T R R T T H T T CALVINE RD H H 1 1 1 Approach 4:EXCELSIOR		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Prob- Critical Volume Critical able Critical Carryover Volume Phase in vph to next phase in vph B2B1 10(B1) 150- 10= 140(B2) 10 A1B2 140(B2) 103- 140= 0(A1) 140 A1A2 83(A2) OR 0(A1) 83 B4B3 10(B4) 250- 10= 240(B3) 10 A4B3 240(B3) 420- 240= 180(A4) 240 A3A4 180(A4) OR 50(A3) 180			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 10 Approach 3 TH= 50 2:RT= 30 RT= 20 v TH= 220 LT= 10 <--Approach 2 Approach 1--> 1:LT= 150 ^ 4: RT= 50 TH= 270 TH= 420 RT= 40 LT= 250 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 2 5 1 0 0 0 < v > 150 -^ 103 -> 103 -> < ^ > 63 +> 40 +v 2 4 ----- 5 2 5 0 0 0				Step 7. SUM OF CRITICAL VOLUMES 150(B1B2)+83(A2)+250(B4B3)+180(A4) = 663 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) A Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4 Exclusive right turns reduced 30 % V/C Ratio = .48							

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: CALVINE RD AND EXCELSIOR RD
Problem Statement: CUMULATIVE NO PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: EXCELSIOR 1 1 1 ^ R L N R T T T L T H H H T Approach 1 << >> ^~RT 1 LT--^ v v v <^~RTH 1 LTH-^> <--TH 2 2 TH--> <v-LTH 1 RTH-v> ^ ^ ^ v--LT 1 RT--v << >> Approach 2 L L T R R T T H T T CALVINE RD H H 1 1 1 Approach 4: EXCELSIOR		-----Approach----- : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 0 0 d.Opposing volume in vph : 0 0 0 0 e.LT capacity on green (vph) : 0 0 0 0 f.LT capacity in vph (b+e) : 0 0 0 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? :				Probable Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph B2B1 10(B1) 80- 10= 70(B2) 10 A1B2 70(B2) 160- 70= 90(A1) 70 A1A2 90(A1) OR 83(A2) 90 B4B3 30(B4) 50- 30= 20(B3) 30 A4B3 20(B3) 90- 20= 70(A4) 20 A3A4 450(A3) OR 70(A4) 450			
Step 2. IDENTIFY VOLUMES, in vph Approach 3 1: LT= 30 2: RT= 20 TH= 450 TH= 230 RT= 190 v LT= 10 <--Approach 2 Approach 1--> 1: LT= 80 ^ 4: RT= 10 TH= 200 TH= 90 RT= 160 LT= 50 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 1 4 ----- 9 5 3 0 0 0 < v > 80 -^ 100 -> 100 -> 0 +> 160 +v ----- 5 9 1 0 0 0				Step 7. SUM OF CRITICAL VOLUMES 80(B1B2)+90(A1)+50(B4B3)+450(A3) = 670 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) [A] Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v 1 --> A3 B1 v-- B3 < v ^ 2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .49			

APPENDIX E

Cumulative Plus Project Level of Service Calculations

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND S. WATT AVE.
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: S. WATT AVE.		-----Approach-----				Possible Volume Adjusted			
1 3 2 ^		: -1- -2- -3- -4-				Critical Volume to next phase in vph			
R L N		a.No. of change intervals/hour				Prob- able Phase			
JACKSON HWY	R T T T L	: 0 0 0 0				Critical Volume to next phase in vph			
T H H H T		b.LT capacity on change (vph)				B2B1 44(B2) 116- 44= 72(B1) 44			
Approach 1	< < > > ^--RT 1	: 0 0 0 0				A2B1 72(B1) 437- 72= 365(A2) 72			
2 LT--^	v v v <^--RTH	c.G/C ratio				A1A2 365(A2) OR 154(A1) 365			
LTH--^>	<--TH 3	: 0 0 0 0				B4B3 83(B4) 424- 83= 341(B3) 83			
3 TH-->	<v--LTH	d.Opposing volume in vph				A4B3 341(B3) 453- 341= 112(A4) 341			
RTH--v>	^ ^ ^ v--LT 2	e.LT capacity on green (vph)				A3A4 607(A3) OR 112(A4) 607			
1 RT--v	< < > > Approach 2	: 0 0 0 0							
L L T R R		f.LT capacity in vph (b+e)							
T T H T T JACKSON HWY		: 0 0 0 0							
H H		g.Left turn volume in vph							
2 3 1		: 0 0 0 0							
Approach 4: S. WATT AVE.		h.Is volume > cap. (g>f) ?							

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		4 6 6 6		116(B2B1)+365(A2)+424(B4B3)+607(A3)	
3: LT= 150	2:RT= 610	5 0 0 6 8	^ 610	= 1512 vph	
TH=1820	TH=1310	0 7 7 7 8 3	<- 437		
RT= 450	LT= 210	<v v v v >	<- 437	Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)	
<--Approach 2		44 -^	v- 95	-----	
		36 -^	v- 116	F	
Approach 1-->		67 ->			
		67 ->	< < ^ ^ ^ >		
1:LT= 80	4: RT= 60	67 ->		Step 9. RECALCULATE	
TH= 200	TH=1360	220 -v	4 3 4 4 4	Geometric Change:	
RT= 220	LT= 770		2 4 5 5 5 6	Signal Change:	
Approach 4			4 7 3 3 3 0	Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1		Approach 3		
--^ AND <-- A1B2 AND				
--> OR v-- /OR A2B1				
--> <-- A1A2		Approach 1		
< B4B3		See Step 6b.		
>		Approach 2		
AND < ^ A3B4 AND				
v > OR /OR A4B3				
^ A3A4				
v				
A1 --> A3	B1 v-- B3 <	Approach 4		Exclusive right turns reduced 30 % V/C Ratio = 1.1
A2 <-- A4	B2 --^ B4 >			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND BRADSHAW RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW RD		-----Approach-----				Possible Volume Adjusted			
1 3 2 ^		: -1- -2- -3- -4-				Critical Volume Critical			
R L N		a.No. of change				Prob- able Volume to next phase			
JACKSON HWY	R T T T L	: 0 0 0 0				Volume in vph			
T H H H T		b.LT capacity on change (vph)				Volume in vph			
Approach 1	<< >> ^--RT 1	: 0 0 0 0				B2B1 127(B1) 154- 127= 27(B2) 127			
2 LT--^	v v v <^--RTH	: 0 0 0 0				A1B2 27(B2) 112- 27= 85(A1) 27			
LTH--^>	<--TH 3	: 0 0 0 0				A1A2 360(A2) OR 85(A1) 360			
3 TH-->	<v--LTH	: 0 0 0 0				B4B3 28(B4) 556- 28= 528(B3) 28			
RTH--v>	^ ^ ^ v--LT 2	: 0 0 0 0				A4B3 528(B3) 680- 528= 152(A4) 528			
1 RT--v	<< >> Approach 2	f.LT capacity in vph (b+e)				A3A4 411(A3) OR 152(A4) 411			
L L T R R		: 0 0 0 0							
T T H T T JACKSON HWY		g.Left turn volume in vph							
H H		: 0 0 0 0							
2 3 1		h.Is volume > cap. (g>f) ?							
Approach 4:BRADSHAW RD									

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		3 4 4 4		154(B1B2)+360(A2)+556(B4B3)+411(A3)	
3: LT= 50		2:RT= 260		= 1481 vph	
TH=1232		TH=1080		-----	
RT= 320	v	LT= 230		Step 8. INTERSECTION LEVEL OF SERVICE	
-----		-----		(compare step 7 with table 6)	
<--Approach 2		154 ^		-----	
-----		126 ^		F	
Approach 1-->		87 ->		-----	
-----		87 ->		Step 9. RECALCULATE	
1:LT= 280		4: RT= 40		Geometric Change:	
TH= 260		TH=2040		Signal Change:	
RT= 160		LT=1010		Volume Change:	
Approach 4		5 4 6 6 6			
-----		5 5 8 8 8 4			
-----		6 5 0 0 0 0			

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1		Approach 3		
--^ AND <-- A1B2 AND				
--> OR v-- /OR A2B1				
--> <-- A1A2		-----		
< B4B3		Approach 1		See Step 6b.
>				
AND < ^ A3B4 AND		-----		
v > OR /OR A4B3		Approach 2		
^ A3A4				
v		-----		
A1 -->	A3 B1 v-- B3 <	Approach 4		Exclusive right turns reduced 30 % V/C Ratio = 1.08
A2 <--	A4 B2 --^ B4 >			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND BRADSHAW RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP				
Approach 3:BRADSHAW RD		Approach				Possible Volume Adjusted				
1 3 2 ^		: -1- -2- -3- -4-				Critical Volume				
R L N		: 0 0 0 0				Carryover Volume				
JACKSON HWY	R T T T L	a.No. of change intervals/hour				Prob- able Phase				
	T H H H T	b.LT capacity on change (vph)				in vph				
Approach 1	<< >> ^--RT 1	c.G/C ratio				B2B1 17(B1) 149- 17= 132(B2) 17				
2 LT--^	v v v <^RTH	d.Opposing volume in vph				A1B2 132(B2) 840- 132= 708(A1) 132				
LTH-^>	<--TH 3	e.LT capacity on green (vph)				A1A2 708(A1) OR 93(A2) 708				
3 TH-->	<v-LTH	f.LT capacity in vph (b+e)				B4B3 138(B4) 286- 138= 148(B3) 138				
RTH-v>	^ ^ ^ v--LT 2	g.Left turn volume in vph				A4B3 148(B3) 513- 148= 365(A4) 148				
1 RT--v	<< >> Approach 2	h.Is volume > cap. (g>f) ?				A3A4 644(A3) OR 365(A4) 644				
	L L T R R									
	T T H T T JACKSON HWY									
	H H									
	2 3 1									
	Approach 4:BRADSHAW RD									
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES				
Approach 3		2 6 6 6 1 1				149(B1B2)+708(A1)+286(B4B3)+644(A3)				
3: LT= 250		5 4 4 4 1 3				= 1787 vph				
TH=1933										
RT= 250	v	< v v v >								
		^-- 110								
		<- 93								
		<- 93				Step 8. INTERSECTION LEVEL OF SERVICE				
		v- 14				(compare step 7 with table 6)				
		v- 17				F				
	<--Approach 2	149 ^								
		122 ^								
		280 ->								
Approach 1-->		280 ->								
		<< ^ ^ ^ ^ >								
1:LT= 270	^					Step 9. RECALCULATE				
TH= 840		2 2 5 5 5 2				Geometric Change:				
RT=1200		8 3 1 1 1 3				Signal Change:				
	Approach 4	6 4 3 3 3 0				Volume Change:				
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS				
--^ v-- B2B1		Approach 3								
--^ AND <-- A1B2 AND										
--> OR v-- /OR A2B1										
--> <-- A1A2										
< B4B3		Approach 1								
>		See Step 6b.								
AND < ^ A3B4 AND		Approach 2								
v > OR /OR A4B3										
^ A3A4										
v										
A1 -->	A3 B1 v-- B3 <	Approach 4				Exclusive right turns reduced 30 %				
A2 <--	A4 B2 --^ B4 >					V/C Ratio = 1.3				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND EXCELSIOR RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
JACKSON HWY Approach 3: EXCELSIOR RD 1 1 1 ^ R L N R T T T L T H H H T Approach 1 << >> ^-RT 1 1 LT--^ v v v <^-RTH LTH-^> <--TH 2 2 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T JACKSON HWY H H 1 1 1 Approach 4: EXCELSIOR RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 40(B2) 160- 40= 120(B1) 40 A2B1 120(B1) 575- 120= 455(A2) 120 A1A2 455(A2) OR 75(A1) 455 B4B3 20(B4) 140- 20= 120(B3) 20 A4B3 120(B3) 1350- 120=1230(A4) 120 A3A4 1230(A4) OR 230(A3) 1230			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 20 Approach 3 2:RT= 80 TH= 230 TH=1150 RT= 90 v LT= 160 <--Approach 2 Approach 1--> 1:LT= 40 ^ 4: RT= 160 TH= 150 TH=1350 RT= 30 LT= 140 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 2 9 3 2 0 0 0 < v > 40 -^ 75 -> 75 -> 30 -v < ^ > 1 1 3 1 4 5 6 0 0 0				Step 7. SUM OF CRITICAL VOLUMES 160(B2B1)+455(A2)+140(B4B3)+1230(A4) = 1985 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.44			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND EXCELSIOR RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP				
Approach 3: EXCELSIOR RD 1 1 1 ^ R L N JACKSON HWY R T T T L T H H H T Approach 1 << >> ^--RT 1 1 LT--^ v v v <^--RTH LTH--> <--TH 2 2 TH--> <v-LTH RTH--v ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T JACKSON HWY H H 1 1 1 Approach 4: EXCELSIOR RD		Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Prob- Critical Carryover Critical able Volume to next Volume Phase in vph phase in vph B2B1 80(B2) 180- 80= 100(B1) 80 A2B1 100(B1) 170- 100= 70(A2) 100 A1A2 555(A1) OR 70(A2) 555 B4B3 30(B4) 30- 30= 0(B3) 30 A4B3 0(B3) 380- 0= 380(A4) 0 A3A4 1340(A3) OR 380(A4) 1340				
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 30 Approach 3 TH=1340 2:RT= 30 RT= 30 v TH= 340 <--Approach 2 Approach 1--> 1:LT= 80 ^ 4: RT= 200 TH=1110 TH= 380 RT= 130 LT= 30 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 1 3 3 4 3 0 0 0 < v > 80 -^ 555 -> < ^ > 555 -> 130 -v 3 2 3 8 0 0 0 0				Step 7. SUM OF CRITICAL VOLUMES 180(B2B1)+555(A1)+30(B4B3)+1340(A3) = 2105 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:				
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.53				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELDER CREEK RD AND S. WATT AVE.
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: S. WATT AVE.		-----Approach-----				Possible Volume Adjusted			
1 3 2 ^		: -1- -2- -3- -4-				Critical Volume			
R L N		a.No. of change				Prob- able Phase			
ELDER CREEK R R T T T L		: 0 0 0 0				Critical Volume			
T H H H T		: intervals/hour				to next phase			
-----		: b.LT capacity on				in vph			
Approach 1 << >> ^--RT 1		: change (vph)				-----			
2 LT--^ v v v <^--RTH		: c.G/C ratio				B2B1 66(B1) 83- 66= 17(B2) 66			
LTH--^ <--TH 2		: d.Opposing volume				A1B2 17(B2) 130- 17= 113(A1) 17			
2 TH--> <v--LTH		: in vph				A1A2 469(A2) OR 113(A1) 469			
RTH--v ^ ^ ^ v--LT 2		: e.LT capacity on				B4B3 83(B4) 385- 83= 302(B3) 83			
1 RT--v << >> Approach 2		: green (vph)				A4B3 302(B3) 1100- 302= 798(A4) 302			
-----		: f.LT capacity in				A3A4 798(A4) OR 383(A3) 798			
L L T R R		: vph (b+e)				-----			
T T H T T ELDER CREEK R		: g.Left turn volume				-----			
H H		: in vph				-----			
2 3 1		: h.Is volume > cap.				-----			
Approach 4: S. WATT AVE.		: (g>f) ?				-----			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		2 3 3 3		83(B1B2)+469(A2)+385(B4B3)+798(A4)	
3: LT= 150		1 8 8 8 6 8		= 1735 vph	
TH=1150		0 3 3 3 8 3		-----	
RT= 210				Step 8. INTERSECTION LEVEL OF SERVICE	
v		< v v v v >		(compare step 7 with table 6)	
-----		-----		-----	
Approach 2		83 ^		F	
Approach 1-->		68 ^		-----	
-----		130 ->		Step 9. RECALCULATE	
1:LT= 150		130 ->		Geometric Change:	
TH= 260		120 -v		Signal Change:	
RT= 120		-----		Volume Change:	
Approach 4		3 3 1 1 1 2		-----	
-----		8 1 0 0 0 2		-----	
-----		5 5 0 0 0 0		-----	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1		Approach 3		
--^ AND <-- A1B2 AND		-----		
--> OR v-- /OR A2B1		-----		
--> <-- A1A2		Approach 1		
< B4B3		See Step 6b.		
>		-----		
AND < ^ A3B4 AND		Approach 2		
v > OR /OR A4B3		-----		
^ A3A4		-----		
v		-----		
A1 --> A3 B1 v-- B3 <		Approach 4		Exclusive right turns reduced 30 % V/C Ratio = 1.26
A2 <-- A4 B2 --^ B4 >		-----		

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELDER CREEK RD AND S. WATT AVE.
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP					
Approach 3: S. WATT AVE.		Approach				Possible Volume Adjusted					
1 3 2 ^		: -1- -2- -3- -4-				Critical Carryover Critical					
R L N		: 0 0 0 0				Volume to next Volume					
ELDER CREEK R R T T T L		a.No. of change intervals/hour				Prob- able Phase					
T H H H T		: 0 0 0 0				in vph in vph					
Approach 1 << >> ^--RT 1		b.LT capacity on change (vph)				B2B1 88(B2) 121- 88= 33(B1) 88					
2 LT--^ v v v <^--RTH		: 0 0 0 0				A2B1 33(B1) 205- 33= 172(A2) 33					
LTH-^>		: 0 0 0 0				A1A2 430(A1) OR 172(A2) 430					
2 TH-->		: 0 0 0 0				B4B3 132(B3) 352- 132= 220(B4) 132					
RTH-v>		: 0 0 0 0				A3B4 220(B4) 933- 220= 713(A3) 220					
1 RT--v << >> v--LT 2		f.LT capacity in vph (b+e)				A3A4 713(A3) OR 577(A4) 713					
L L T R R		: 0 0 0 0									
T T H T T ELDER CREEK R		g.Left turn volume in vph									
H H		: 0 0 0 0									
2 3 1		h.Is volume > cap. (g>f) ?									
Approach 4: S. WATT AVE.											
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES					
3: LT= 640 TH=2800 RT= 210		2 9 9 9 2 3 1 3 3 3 8 5 0 3 3 3 8 2 < v v v >>				121(B2B1)+430(A1)+352(B3B4)+713(A3)					
Approach 3 2:RT= 240 TH= 410 LT= 220		^-- 240 <-- 205 <-- 205 v-- 99 v-- 121				= 1616 vph					
Approach 1-->		88 ^-- 72 ^-- 430 --> 430 --> 500 -v				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)					
1:LT= 160 TH= 860 RT= 500		4: RT= 150 TH=1730 LT= 240				F					
Approach 4		1 1 5 5 5 1 3 0 7 7 7 5 2 8 7 7 7 0				Step 9. RECALCULATE					
						Geometric Change: Signal Change: Volume Change:					
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS					
--^ v-- B2B1		Approach 3									
--^ AND <-- A1B2 AND		Approach 1									
--> OR v-- /OR A2B1		See Step 6b.									
--> <-- A1A2		Approach 2									
< B4B3											
>											
AND < ^ A3B4 AND											
v > OR /OR A4B3											
^ A3A4											
v											
A1 --> A3 B1 v-- B3 <		Approach 4				Exclusive right turns reduced 30 % V/C Ratio = 1.18					
A2 <-- A4 B2 --^ B4 >											

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELDER CREEK RD AND BRADSHAW RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW RD		-----Approach-----				Possible Volume Adjusted			
1 3 2 ^		: -1- -2- -3- -4-				Critical Volume to next phase in vph			
R L M		a.No. of change intervals/hour				Prob- able Phase			
ELDER CREEK R R T T T L		: 0 0 0 0				6(B1) 171- 6= 165(B2) 6			
T H H H T		b.LT capacity on change (vph)				A1B2 165(B2) 120- 165= 0(A1) 165			
Approach 1 < < > > ^--RT 1		: 0 0 0 0				A1A2 165(A2) OR 0(A1) 165			
2 LT--^ v v v <^--RTH		c.G/C ratio				B2B1 6(B1) 171- 6= 165(B2) 6			
LTH--> <--TH 2		: 0 0 0 0				A4B3 6(B4) 622- 6= 616(B3) 6			
2 TH--> <v--LTH		d.Opposing volume in vph				A3A4 310(A3) OR 294(A4) 310			
RTH--v> ^ ^ ^ v--LT 2		e.LT capacity on green (vph)							
1 RT--v < < > > Approach 2		: 0 0 0 0							
L L T R R		f.LT capacity in vph (b+e)							
T T H T T ELDER CREEK R		: 0 0 0 0							
H H		g.Left turn volume in vph							
2 3 1		h.Is volume > cap. (g>f) ?							
Approach 4:BRADSHAW RD									

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		2 3 3 3		171(B1B2)+165(A2)+622(B4B3)+310(A3)	
3: LT= 10		3 1 1 1		= 1268 vph	
TH= 930		0 0 0 0 5 6			
RT= 230		< v v v v >		Step 8. INTERSECTION LEVEL OF SERVICE	
v				(compare step 7 with table 6)	
2:RT= 130				E	
TH= 330					
LT= 10					
<--Approach 2					
Approach 1-->		171 -^			
		140 -^			
		120 ->			
		< < ^ ^ ^ >			
		120 ->			
		30 -v			
1:LT= 310		6 5 9 9 9 1		Step 9. RECALCULATE	
TH= 240		2 0 1 1 1 1		Geometric Change:	
RT= 30		2 9 0 0 0 0		Signal Change:	
Approach 4				Volume Change:	
4: RT= 110					
TH=2730					
LT=1130					

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1		Approach 3		
--^ AND <-- A1B2 AND				
--> OR v-- /OR A2B1				See Step 6b.
--> <-- A1A2		Approach 1		
< B4B3				Approach 2
>				
AND < ^ A3B4 AND				Approach 4
v > OR /OR A4B3				
^ A3A4				Exclusive right turns reduced 30 % V/C Ratio = .92
v				
A1 --> A3 B1 v-- B3 <				
v ^				
A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELDER CREEK RD AND BRADSHAW RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW RD 1 3 2 ^ R L N R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph ----- B2B1 50(B1) 149- 50= 99(B2) 50 A1B2 99(B2) 714- 99= 615(A1) 99 A1A2 615(A1) OR 165(A2) 615 B4B3 11(B4) 55- 11= 44(B3) 11 A4B3 44(B3) 530- 44= 486(A4) 44 A3A4 970(A3) OR 486(A4) 970			
Approach 1 << >> ^-RT 1 2 LT--^ v v v <^-RTH LTH-^> <--TH 2 2 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2									
----- L L T R R ELDER CREEK R T T H T T H H 2 3 1 Approach 4:BRADSHAW RD									

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
3: LT= 20 TH=2910 RT= 280 Approach 3 2:RT= 10 TH= 330 LT= 90 <--Approach 2		2 9 9 9 8 7 7 7 1 0 0 0 0 9 1 < v v v v >		149(B1B2)+615(A1)+55(B4B3)+970(A3) = 1789 vph	
Approach 1--> 1:LT= 270 TH= 360 RT=1020 Approach 4 4: RT= 30 TH=1590 LT= 100		149 ^- 122 ^- 180 -> 180 -> 1020 -v ----- << ^ ^ ^ ^ > 5 5 5 5 4 3 3 3 3 5 5 0 0 0 0		Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- F ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING	Step 6a. CRITICAL VOLUMES, in vph (two phase signal)	COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v	Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4	Exclusive right turns reduced 30 % V/C Ratio = 1.3
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELDER CREEK RD AND EXCELSIOR RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: EXCELSIOR RD 1 1 ^ R L N R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 1 0 d.Opposing volume in vph : 0 10 1280 0 e.LT capacity on green (vph) : 0 0 0 0 f.LT capacity in vph (b+e) : 0 0 0 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? : NO NO				Prob-able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph A4B3 160(B3) 1280-160=1120(A4) 160 A3A4 1120(A4) OR 210(A3) 1120 A1B2 360(B2) OR 7(A1) 360			
Approach 1 1 LT--^ v v v <^--RT LTH--> <--TH TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 1 RT--v << >> Approach 2 L L T R R T T H T T ELDER CREEK R H H 1 1 Approach 4:									

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3 3: LT= 0 2:RT= 0 TH= 100 TH= 0 RT= 300 v LT= 0		3 1 0 0 0 0 < v		1280(B3A4)+360(B2)+0()+0() = 1640 vph	
<--Approach 2 Approach 1-->				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F	
Approach 4 1:LT= 360 ^ 4: RT= 0 360 -^ TH= 0 TH=1280 10 -v RT= 10 LT= 160		< ^ 1 1 2 6 8 0 0		Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
< ^ A4B3 v ^ A3A4 --^ A1B2 -->		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		
A1 --> A3 v ^ B1 v-- B3 < A2 <-- A4 B2 --^ B4 >				Exclusive right turns reduced 30 % V/C Ratio = 1.15

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELDER CREEK RD AND EXCELSIOR RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY	Step 4. LEFT TURN CHECK	Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP
<p>Approach 3: EXCELSIOR RD</p> <p>1 1 ^</p> <p>R L N</p> <p>ELDER CREEK R R T T T L </p> <p>T H H H T </p> <p>-----</p> <p>Approach 1 << >> ^--RT</p> <p>1 LT--^ v v v <^--RTH</p> <p>LTH--^> <--TH</p> <p>TH--> <v--LTH</p> <p>RTH--v> ^ ^ ^ v--LT</p> <p>1 RT--v << >> Approach 2</p> <p>-----</p> <p>L L T R R ELDER CREEK R</p> <p>T T H T T </p> <p>H H </p> <p>1 1 </p> <p>Approach 4: .</p>	<p>-----Approach-----</p> <p>: -1- -2- -3- -4-</p> <p>a.No. of change : 0 0 0 0</p> <p>intervals/hour : :</p> <p>b.LT capacity on : 0 0 0 0</p> <p>change (vph) : :</p> <p>c.G/C ratio : 0 0 1 0</p> <p>d.Opposing volume : 0 420 260 0</p> <p>in vph : :</p> <p>e.LT capacity on : 0 0 940 0</p> <p>green (vph) : :</p> <p>f.LT capacity in : 0 0 940 0</p> <p>vph (b+e) : :</p> <p>g.Left turn volume : 0 0 0 0</p> <p>in vph : :</p> <p>h.Is volume > cap. : NO NO</p> <p>(g>f) ? : :</p>	<p>Possible Volume Adjusted</p> <p>Critical Carryover Critical</p> <p>Volume to next Volume</p> <p>in vph phase in vph</p> <p>-----</p> <p>A4B3 20(B3) 260- 20= 240(A4) 20</p> <p>A3A4 1230(A3) OR 240(A4) 1230</p> <p>A1B2 350(B2) OR 294(A1) 350</p>
<p>Step 2. IDENTIFY VOLUMES, in vph</p> <p>3: LT= 0 Approach 3 2:RT= 0</p> <p>TH=1230 TH= 0</p> <p>RT= 30 v LT= 0</p> <p>-----</p> <p><--Approach 2</p> <p>-----</p> <p>Approach 1--></p> <p>-----</p> <p>1:LT= 350 ^ 4: RT= 0 350 --^</p> <p>TH= 0 TH= 260 420 --v</p> <p>RT= 420 LT= 20</p> <p>-----</p> <p>Approach 4 </p>	<p>Step 5. ASSIGN LANE VOLUMES, in vph</p> <p>1</p> <p>2</p> <p>-----</p> <p>3 3</p> <p>0 0</p> <p> </p> <p>< v</p> <p>-----</p> <p>< ^</p> <p> </p> <p>-----</p> <p>2 -----</p> <p>2 6 </p> <p>0 0 </p>	<p>Step 7. SUM OF CRITICAL VOLUMES</p> <p>1250(B3A3)+350(B2)+0()+0()</p> <p>= 1600 vph</p> <p>-----</p> <p>Step 8. INTERSECTION LEVEL OF SERVICE</p> <p>(compare step 7 with table 6)</p> <p>-----</p> <p> F </p> <p>-----</p> <p>Step 9. RECALCULATE</p> <p>Geometric Change:</p> <p>Signal Change:</p> <p>Volume Change:</p>
<p>Step 3. IDENTIFY PHASING</p> <p>< ^ A4B3</p> <p> </p> <p>v ^ A3A4</p> <p>-----</p> <p>--^ A1B2</p> <p>--></p>	<p>Step 6a. CRITICAL VOLUMES, in vph</p> <p>(two phase signal)</p> <p>Approach 3</p> <p>-----</p> <p>Approach 1</p> <p>See Step 6b.</p> <p>-----</p> <p>Approach 2</p> <p>-----</p> <p>Approach 4</p>	<p>COMMENTS</p> <p>-----</p> <p>Exclusive right turns reduced 30 %</p> <p>V/C Ratio = 1.12</p>
<p>A1 --> A3 B1 v-- B3 < </p> <p>v ^</p> <p>A2 <-- A4 B2 --^ B4 > </p>		

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND ELK GROVE-FLORIN RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP				
Approach 3: ELK GROVE-FLO		Approach				Possible Critical Adjusted				
1 3 2 ^		: -1- -2- -3- -4-				Volume Carryover Volume				
R L N		a.No. of change				Prob- able Phase				
R T T T L		: 0 0 0 0				Critical Volume in vph				
T H H H T		b.LT capacity on				to next phase				
Approach 1 << >> ^--RT 1		: 0 0 0 0				change (vph)				
2 LT--^ v v v <^--RTH		: 0 0 0 0				c.G/C ratio				
LTH-^>		: 0 0 0 0				d.Opposing volume				
3 TH-->		: 0 0 0 0				in vph				
RTH-v>		: 0 0 0 0				e.LT capacity on				
1 RT--v << >> v--LT 2		: 0 0 0 0				green (vph)				
Approach 2		: 0 0 0 0				f.LT capacity in				
L L T R R		: 0 0 0 0				vph (b+e)				
T T H T T FLORIN RD		: 0 0 0 0				g.Left turn volume				
H H		: 0 0 0 0				in vph				
2 3 1		: 0 0 0 0				h.Is volume > cap.				
Approach 4: ELK GROVE-FLO		: 0 0 0 0				(g>f) ?				
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES				
Approach 3		5 8 8 8 2 2				143(B1B2)+570(A1)+270(B3B4)+742(A3)				
3: LT= 490		7 3 3 3 2 7				= 1725 vph				
TH=2490		0 0 0 0 1 0				Step 8. INTERSECTION LEVEL OF SERVICE				
RT= 570		< v v v >>				(compare step 7 with table 6)				
2: RT= 240		143 ^				F				
TH= 910		117 ^								
RT= 210		597 ->								
Approach 2		597 ->								
Approach 1-->		597 ->								
1: LT= 260		597 ->								
TH=1790		550 -v								
RT= 550		1 1 5 5 5 1				Step 9. RECALCULATE				
Approach 4		8 4 0 0 0 0				Geometric Change:				
		2 9 7 7 7 0				Signal Change:				
						Volume Change:				
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS				
--^ v-- B2B1		Approach 3								
--^ AND <-- A1B2 AND		Approach 1								
--> OR v-- /OR A2B1		See Step 6b.								
--> <-- A1A2		Approach 2								
< B4B3										
>										
AND < ^ A3B4 AND										
v > OR /OR A4B3										
^ A3A4										
v										
A1 --> A3 B1 v-- B3 <		Approach 4				Exclusive right turns reduced 30 %				
A2 <-- A4 B2 --^ B4 >						V/C Ratio = 1.25				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND WATERMAN RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: WATERMAN RD		Approach				Possible Volume Adjusted			
R L M		-1- -2- -3- -4-				Critical Volume to next phase			
R T T T L		a.No. of change intervals/hour				Prob-able Phase			
T H H H T		: 0 0 0 0				in vph			
Approach 1 << >> ^--RT		b.LT capacity on change (vph)				A2B1 165(B1) 53- 165= 0(A2) 165			
LT--^ v v v <^--RTH		: 0 0 1 0				A1A2 161(A1) OR 0(A2) 161			
LTH--> <--TH 3		c.G/C ratio				A4B3 633(B3) OR 399(A4) 633			
3 TH--> <v--LTH		: 160 0 570 0							
RTH--v ^ ^ ^ v--LT 2		d.Opposing volume in vph							
1 RT--v << >> Approach 2		e.LT capacity on green (vph)							
L L T R R		: 0 0 630 0							
T T H T T FLORIN RD		f.LT capacity in vph (b+e)							
H H		: 0 0 0 0							
2 1		g.Left turn volume in vph							
Approach 4: WATERMAN RD		h.Is volume > cap. (g>f) ?							
		: NO NO							
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
3: LT= 0 Approach 3						326(B1A1)+633(B3)+0()+0()			
TH= 0						= 959 vph			
RT= 0 v									
						Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)			
						B			
Approach 1-->		83 ->							
		83 ->							
		83 ->							
		230 -v							
1:LT= 0 Approach 4						Step 9. RECALCULATE			
TH= 250						Geometric Change:			
RT= 230						Signal Change:			
						Volume Change:			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
<-- A2B1		Approach 3							
v--									
--> <-- A1A2		Approach 1							
< ^ A4B3		See Step 6b.							
		Approach 2							
A1 --> A3 B1 v-- B3 <		Approach 4				Exclusive right turns reduced 30 % V/C Ratio = .67			
v ^									
2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND WATERMAN RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: WATERMAN RD R L N R T T T L T H H H T Approach 1 << >> ^--RT LT--^ v v v <^--RTH LTH-^> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 2 1 Approach 4: WATERMAN RD		-----Approach----- : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 1 0 d.Opposing volume in vph : 470 0 350 0 e.LT capacity on green (vph) : 0 0 850 0 f.LT capacity in vph (b+e) : 0 0 850 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? : NO NO				Prob- able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph A2B1 270(B1) 157- 270= 0(A2) 270 A1A2 749(A1) OR 0(A2) 749 A4B3 391(B3) OR 245(A4) 391			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 0 TH= 0 RT= 0 Approach 3 2: RT= 0 TH= 470 LT= 490 Approach 2 Approach 1--> 1: LT= 0 TH=1560 RT=1070 Approach 4 4: RT= 350 TH= 0 LT= 710		Step 5. ASSIGN LANE VOLUMES, in vph 520 -> 520 -> 520 -> 1070 -v << >> 3 3 3 9 2 5 1 0 0				Step 7. SUM OF CRITICAL VOLUMES 1019(B1A1)+391(B3)+0()+0() = 1410 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) E Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING <-- A2B1 v-- --> <-- A1A2 < ^ A4B3 		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .99			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND BRADSHAW RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW RD		-----Approach-----				Possible Volume Adjusted			
1 3 2 ^		: -1- -2- -3- -4-				Critical Volume Critical			
R L N		a.No. of change				Prob- able Phase			
R T T T L		: 0 0 0 0				Volume to next phase			
T H H H T		: 0 0 0 0				in vph			
Approach 1 << >> ^--RT 1		b.LT capacity on change (vph)				B2B1 17(B1) 413- 17= 396(B2) 17			
2 LT--^ v v v <^--RTH		: 0 0 0 0				A1B2 396(B2) 63- 396= 0(A1) 396			
LTH--> <--TH 3		c.G/C ratio				A1A2 504(A2) OR 0(A1) 504			
3 TH--> <v--LTH		: 0 0 0 0				B4B3 6(B4) 440- 6= 434(B3) 6			
RTH-v> ^ ^ ^ v--LT 2		d.Opposing volume in vph				A4B3 434(B3) 827- 434= 393(A4) 434			
1 RT--v << >> Approach 2		e.LT capacity on green (vph)				A3A4 393(A4) OR 237(A3) 393			
L L T R R		: 0 0 0 0							
T T H T T FLORIN RD		f.LT capacity in vph (b+e)							
H H		: 0 0 0 0							
2 3 1		g.Left turn volume in vph							
Approach 4:BRADSHAW RD		: 0 0 0 0							
		h.Is volume > cap. (g>f) ?							

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		2 2 2 2		413(B1B2)+504(A2)+440(B4B3)+393(A4)	
3: LT= 10		6 3 3 3		= 1750 vph	
TH= 710		0 7 7 7 5 6			
RT= 260					
v		< v v v v >		Step 8. INTERSECTION LEVEL OF SERVICE	
				(compare step 7 with table 6)	
				F	
Approach 1-->		413 ^		Step 9. RECALCULATE	
		338 ^		Geometric Change:	
		63 ->		Signal Change:	
		63 ->		Volume Change:	
		63 ->			
		63 ->			
		70 -v			
		4 3 8 8 8 4			
		4 6 2 2 2 0			
		0 0 7 7 7 0			

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1		Approach 3		
--^ AND <-- A1B2 AND				
--> OR v-- /OR A2B1				
--> <-- A1A2				
< B4B3		Approach 1		See Step 6b.
>				
AND < ^ A3B4 AND		Approach 2		
v > OR /OR A4B3				
^ A3A4				
v				
A1 --> A3 B1 v-- B3 <		Approach 4		Exclusive right turns reduced 30 % V/C Ratio = 1.27
v ^				
A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND BRADSHAW RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW RD 1 3 2 ^ R L N R T T T L T H H H T		Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph ----- B2B1 165(B2) 193- 165= 28(B1) 165 A2B1 28(B1) 140- 28= 112(A2) 28 A1A2 476(A1) OR 112(A2) 476 B4B3 72(B3) 385- 72= 313(B4) 72 A3B4 313(B4) 850- 313= 537(A3) 313 A3A4 537(A3) OR 407(A4) 537			
Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--^> <--TH 3 3 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R		FLORIN RD T T H T T H H 2 3 1 Approach 4:BRADSHAW RD							
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 700 Approach 3 2:RT= 200 TH=2550 TH= 310 RT= 770 v LT= 350 <--Approach 2		Step 5. ASSIGN LANE VOLUMES, in vph 7 8 8 8 3 3 7 5 5 5 1 8 0 0 0 0 5 5 < v v v > >				Step 7. SUM OF CRITICAL VOLUMES 193(B2B1)+476(A1)+385(B3B4)+537(A3) = 1591 vph			
Approach 1--> 1:LT= 300 ^ 4: RT= 40 TH=1030 TH=1220 RT= 680 LT= 130 Approach 4		165 -^ 135 -^ 343 -> 343 -> << ^ ^ ^ > 343 -> 680 -v ----- 4 4 4 ----- 7 5 0 0 0 4 2 9 7 7 7 0				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- F ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS ----- Exclusive right turns reduced 30 % V/C Ratio = 1.16			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND VINEYARD RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: VINEYARD RD		-----Approach-----				Possible Volume Adjusted			
R L N		: -1- -2- -3- -4-				Critical Volume Critical			
FLORIN RD	R T T T L	a.No. of change intervals/hour				Prob- able Phase			
	T H H H T	: 0 0 0 0				Critical Volume in vph			
Approach 1	<< >> ^--RT	b.LT capacity on change (vph)				A2B1 66(B1) 180- 66= 114(A2) 66			
	LT--^ v v v <^--RTH	: 0 0 1 0				A1A2 213(A1) OR 114(A2) 213			
	LTH--> <--TH 3	c.G/C ratio				A4B3 560(A4) OR 391(B3) 560			
3	TH--> <v-LTH	d.Opposing volume in vph							
	RTH--v ^ ^ ^ v--LT 2	: 540 0 800							
1	RT--v << >> Approach 2	e.LT capacity on green (vph)							
	L L T R R	: 0 0 400 0							
	T T H T T FLORIN RD	f.LT capacity in vph (b+e)							
	H H	: 0 0 0 0							
	2 1	g.Left turn volume in vph							
	Approach 4: VINEYARD RD	h.Is volume > cap. (g>f) ?							
		: NO NO							

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3				279(B1A1)+560(A4)+0()+0()	
3: LT= 0	2:RT= 0			= 839 vph	
TH= 0	TH= 540			Step 8. INTERSECTION LEVEL OF SERVICE	
RT= 0	LT= 120			(compare step 7 with table 6)	
	<--Approach 2			A	
Approach 1-->		213 ->			
		213 ->			
1:LT= 0	4: RT= 800	213 ->		Step 9. RECALCULATE	
TH= 640	TH= 0	20 -v		Geometric Change:	
RT= 20	LT= 710			Signal Change:	
	Approach 4			Volume Change:	
				3 3 8	
				9 2 0	
				1 0 0	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
<-- A2B1		Approach 3		
v--				
--> <-- A1A2				
< ^ A4B3		Approach 1		
		See Step 6b.		
		Approach 2		
				Exclusive right turns reduced 30 % V/C Ratio = .59
A1 --> A3	B1 v-- B3 <	Approach 4		
A2 <-- A4	B2 --^ B4 >			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND VINEYARD RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP						
Approach 3: VINEYARD RD		Approach				Possible Volume Adjusted						
R L N		-1- -2- -3- -4-				Critical Carryover Critical						
FLORIN RD	R T T T L	a.No. of change intervals/hour	: 0	0	0	0	Prob-able Phase	352(B1)	203-	352=	0(A2)	352
	T H H H T	b.LT capacity on change (vph)	: 0	0	0	0		483(A1) OR	0(A2)			483
Approach 1	<< >> ^--RT	c.G/C ratio	: 0	0	1	0	A2B1	147(A4) OR	50(B3)			147
	LT--^ v v v <^RTH	d.Opposing volume in vph	: 610	0	210	0	A1A2					
	LTH-^>	e.LT capacity on green (vph)	: 0	0	990	0						
3	TH-->	f.LT capacity in vph (b+e)	: 0	0	990	0						
	RTH-v>	g.Left turn volume in vph	: 0	0	0	0						
1	RT--v << >> Approach 2	h.Is volume > cap. (g>f) ?	: NO		NO							
	L L T R R											
	T T H T T FLORIN RD											
	H H											
	2 1											
	Approach 4: VINEYARD RD											

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3				835(B1A1)+147(A4)+0()+0()	
3: LT= 0	2:RT= 0			= 982 vph	
TH= 0	TH= 610				
RT= 0	LT= 640				
<--Approach 2					
Approach 1-->		160 ->			
		160 ->			
		160 ->			
		690 -v			
Approach 4				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)	
				B	
				Step 9. RECALCULATE	
				Geometric Change:	
				Signal Change:	
				Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS	
<-- A2B1		Approach 3			
v--					
--> <-- A1A2					
< ^ A4B3		Approach 1			
		See Step 6b.			
		Approach 2			
		Approach 4		Exclusive right turns reduced 30 % V/C Ratio = .69	
A1 -->	A3 B1 v-- B3 <				
	v ^				
A2 <--	A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND EXCELSIOR RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: EXCELSIOR RD 1 1 1 ^ R L N R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Volume Critical Volume to next Volume in vph phase in vph ----- B2B1 50(B1) 610- 50= 560(B2) 50 A1B2 560(B2) 425- 560= 0(A1) 560 A1A2 290(A2) OR 0(A1) 290 B4B3 10(B4) 30- 10= 20(B3) 10 A4B3 20(B3) 810- 20= 790(A4) 20 A3A4 790(A4) OR 50(A3) 790			
Approach 1 1 LT--^> ^--RT 1 LTH--> <--RTH 2 2 TH--> <v-LTH RTH-v> ^^^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 1 1 1 Approach 4: EXCELSIOR RD									

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3 3: LT= 10 2:RT= 20 TH= 50 TH= 580 RT= 50 v LT= 50 ----- <--Approach 2		----- 5 5 1 0 0 0 < v >		----- ^- 20 <- 290 <- 290 v- 50	
Approach 1--> 1:LT= 610 ^ 4: RT= 110 TH= 850 TH= 810 RT= 10 LT= 30 Approach 4		----- 610 -^ 425 -> 425 -> < ^ > 10 -v ----- 8 1 3 1 1 0 0 0		----- 610(B1B2)+290(A2)+30(B4B3)+790(A4) = 1720 vph ----- Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- F ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND EXCELSIOR RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: EXCELSIOR RD 1 1 1 ^ R L N R T T T L T H H H T Approach 1 << >> ^--RT 1 1 LT--^ v v v <^RTH LTH-^> <--TH 2 2 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 1 1 1 Approach 4: EXCELSIOR RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 120(B2) 210- 120= 90(B1) 120 A2B1 90(B1) 305- 90= 215(A2) 90 A1A2 265(A1) OR 215(A2) 265 B4B3 10(B4) 20- 10= 10(B3) 10 A4B3 10(B3) 140- 10= 130(A4) 10 A3A4 610(A3) OR 130(A4) 610			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 10 Approach 3 2:RT= 10 TH= 610 TH= 610 RT= 640 v LT= 210 <--Approach 2 Approach 1--> 1:LT= 120 ^ 4: RT= 50 TH= 530 TH= 140 RT= 30 LT= 20 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 6 6 4 1 1 0 0 0 < ^ > < 305 < 305 v- 210				Step 7. SUM OF CRITICAL VOLUMES 210(B2B1)+265(A1)+20(B4B3)+610(A3) = 1105 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) D			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .8			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND ELK GROVE-FLORIN RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE-FLO 1 3 2 ^ R L N R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 0 0 d.Opposing volume in vph : 0 0 0 0 e.LT capacity on green (vph) : 0 0 0 0 f.LT capacity in vph (b+e) : 0 0 0 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? :				Possible Volume Adjusted Prob- Critical Carryover Critical able Volume to next Volume Phase in vph phase in vph B2B1 99(B1) 209- 99= 110(B2) 99 A1B2 110(B2) 210- 110= 100(A1) 110 A1A2 600(A2) OR 100(A1) 600 B4B3 154(B4) 440- 154= 286(B3) 154 A4B3 286(B3) 1060- 286= 774(A4) 286 A3A4 774(A4) OR 303(A3) 774			
Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH-> <--TH 2 2 TH--> <v--LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 2 3 1 Approach 4: ELK GROVE-FLO									

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES			
Approach 3 3: LT= 280 TH= 910 RT= 110 v		2: RT= 510 TH=1200 LT= 180 <--Approach 2		1 3 3 3 1 1 1 0 0 0 2 5 0 3 3 3 6 4 < v v v v > ^- 510 <- 600 <- 600 v- 81 v- 99		209(B1B2)+600(A2)+440(B4B3)+774(A4) = 2023 vph	
Approach 1--> 1: LT= 380 TH= 410 RT= 300 Approach 4		4: RT= 80 TH=3180 LT= 800		<< ^ ^ ^ ^ > 1 1 1 4 3 0 0 0 4 6 6 6 6 8 0 0 0 0 0 0		Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F	
				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND ELK GROVE-FLORIN RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE-FLO 1 3 2 ^ R L N R T T T L T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH-^> <--TH 2 2 TH--> <v--LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R GERBER RD T T H T T H H 2 3 1 Approach 4: ELK GROVE-FLO		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 72(B2) 110- 72= 38(B1) 72 A2B1 38(B1) 330- 38= 292(A2) 38 A1A2 595(A1) OR 292(A2) 595 B4B3 193(B3) 292- 193= 99(B4) 193 A3B4 99(B4) 877- 99= 778(A3) 99 A3A4 778(A3) OR 497(A4) 778			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 530 Approach 3 2:RT= 410 TH=2630 TH= 660 RT= 300 v LT= 200 <--Approach 2 Approach 1--> 1:LT= 130 ^ 4: RT= 120 TH=1110 TH=1490 RT= 850 LT= 350 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 3 8 8 8 2 2 0 7 7 7 3 9 0 7 7 7 9 2 < v v v >> 72 -^ 59 -^ 555 -> << ^ ^ ^ ^ > 555 -> 850 -v 1 1 4 4 4 1 9 5 9 9 9 2 3 8 7 7 7 0				Step 7. SUM OF CRITICAL VOLUMES 110(B2B1)+595(A1)+292(B3B4)+778(A3) = 1775 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4 Exclusive right turns reduced 30 % V/C Ratio = 1.29							

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND WATERMAN RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: WATERMAN RD		-----Approach-----				Possible Volume Adjusted			
1 2 2 ^		: -1- -2- -3- -4-				Critical Volume Critical			
R L M		a.No. of change				Prob- able Phase			
R T T T L		: 0 0 0 0				Volume to next phase			
T H H H T		: 0 0 0 0				in vph			
Approach 1 < < > > ^-RT 1		b.LT capacity on change (vph)				B2B1 39(B1) 55- 39= 16(B2) 39			
2 LT--^ v v v <^-RTH		: 0 0 0 0				A1B2 16(B2) 220- 16= 204(A1) 16			
LTH-^>		: 0 0 0 0				A1A2 420(A2) OR 204(A1) 420			
2 TH-->		c.G/C ratio				B4B3 39(B4) 303- 39= 264(B3) 39			
RTH-v>		: 0 0 0 0				A4B3 264(B3) 455- 264= 191(A4) 264			
1 RT--v < < > > v--LT 2		e.LT capacity on green (vph)				A3A4 191(A4) OR 175(A3) 191			
L L T R R		: 0 0 0 0							
T T H T T GERBER RD		f.LT capacity in vph (b+e)							
H H		: 0 0 0 0							
2 2 1		g.Left turn volume in vph							
Approach 4: WATERMAN RD		: 0 0 0 0							
		h.Is volume > cap. (g>f) ?							

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		1 1 1		55(B1B2)+420(A2)+303(B4B3)+191(A4)	
3: LT= 70		9 7 7 3 3		= 969 vph	
TH= 350		0 5 5 2 9			
RT= 190 v					
		< v v v >			
				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)	
				C	
Approach 1-->		55 ^-			
		45 ^-			
		220 ->			
		< < ^ ^ >			
1:LT= 100 ^		220 ->		Step 9. RECALCULATE	
TH= 440		70 -v		Geometric Change:	
RT= 70				Signal Change:	
Approach 4				Volume Change:	
		3 2 4 4 2			
		0 4 5 5 6			
		3 8 5 5 0			

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1		Approach 3		
--^ AND <-- A1B2 AND				
--> OR v-- /OR A2B1				Approach 1
--> <-- A1A2				
< B4B3		See Step 6b.		Approach 2
>				
AND < ^ A3B4 AND				Approach 4
v > OR /OR A4B3				
^ A3A4				Exclusive right turns reduced 30 % V/C Ratio = .7
v				
A1 --> A3 B1 v-- B3 <				
v ^				
A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND WATERMAN RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: WATERMAN RD 1 2 2 ^ R L N R T T T L T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^RTH LTH-^> <--TH 2 2 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 2 2 1 Approach 4: WATERMAN RD		Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 149(B2) 220- 149= 71(B1) 149 A2B1 71(B1) 310- 71= 239(A2) 71 A1A2 440(A1) OR 239(A2) 440 B4B3 88(B3) 127- 88= 39(B4) 88 A3B4 39(B4) 450- 39= 411(A3) 39 A3A4 411(A3) OR 310(A4) 411			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 230 TH= 900 RT= 130 Approach 3 2:RT= 130 TH= 620 LT= 400 v <--Approach 2 Approach 1--> 1:LT= 270 TH= 880 RT= 310 Approach 4 4: RT= 140 TH= 620 LT= 160		Step 5. ASSIGN LANE VOLUMES, in vph 1 4 4 1 1 3 5 5 0 2 0 0 0 4 7 < v v >> 149 -^ 122 -^ 440 -> 440 -> 310 -v << ^ ^ > 3 3 1 8 7 1 1 4 8 2 0 0 0				Step 7. SUM OF CRITICAL VOLUMES 220(B2B1)+440(A1)+127(B3B4)+411(A3) = 1198 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) D Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .87			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND BRADSHAW RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD 1 3 2 ^ R L N R T T T L T H H H T Approach 1 << >> ^-RT 1 2 LT--^ v v v <^-RTH LTH--> <--TH 2 2 TH--> <v-LTH RTH--v ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 2 3 1 Approach 4: BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph B2B1 88(B1) 204- 88= 116(B2) 88 A1B2 116(B2) 165- 116= 49(A1) 116 A1A2 250(A2) OR 49(A1) 250 B4B3 22(B4) 209- 22= 187(B3) 22 A4B3 187(B3) 620- 187= 433(A4) 187 A3A4 433(A4) OR 170(A3) 433			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 40 Approach 3 2:RT= 290 TH= 510 TH= 500 RT= 230 v LT= 160 <--Approach 2 Approach 1--> 1:LT= 370 ^ 4: RT= 110 TH= 330 TH=1860 RT= 120 LT= 380 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 2 1 1 1 3 7 7 7 1 2 0 0 0 0 8 2 < v v v v > 204 -^ 167 -^ 165 -> 165 -> 120 -v < < ^ ^ ^ ^ > 2 1 6 6 6 1 0 7 2 2 2 1 9 1 0 0 0 0				Step 7. SUM OF CRITICAL VOLUMES 204(B1B2)+250(A2)+209(B4B3)+433(A4) = 1096 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) C Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .8			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND BRADSHAW RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD 1 3 2 ^ R L N R T T T L T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH-^> <--TH 2 2 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 2 3 1 Approach 4: BRADSHAW RD		Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 88(B1) 171- 88= 83(B2) 88 A1B2 83(B2) 266- 83= 183(A1) 83 A1A2 245(A2) OR 183(A1) 245 B4B3 105(B3) 110- 105= 5(B4) 105 A3B4 5(B4) 600- 5= 595(A3) 5 A3A4 595(A3) OR 277(A4) 595			
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
Approach 3 3: LT= 200 2:RT= 70 TH=1800 TH= 490 RT= 530 v LT= 160 <--Approach 2 Approach 1--> 1:LT= 310 ^ 4: RT= 200 TH= 480 TH= 830 RT= 380 LT= 190 Approach 4		5 6 6 6 1 3 0 0 0 9 1 0 0 0 0 0 0 < v v v v > 171 -^ 140 -^ 240 -> < < ^ ^ ^ ^ > 240 -> 380 -v 1 2 2 2 2 0 8 7 7 7 0 5 6 7 7 7 0				171(B1B2)+245(A2)+110(B3B4)+595(A3) = 1121 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) D			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				Exclusive right turns reduced 30 % V/C Ratio = .82			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND VINEYARD RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY	Step 4. LEFT TURN CHECK	Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP
Approach 3: VINEYARD RD 1 2 2 ^ R L N GERBER RD R T T T L T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--> <--TH 2 2 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 2 2 1 Approach 4: VINEYARD RD	-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :	Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 6(B1) 281- 6= 275(B2) 6 A1B2 275(B2) 75- 275= 0(A1) 275 A1A2 119(A2) OR 0(A1) 119 B4B3 6(B4) 66- 6= 60(B3) 6 A4B3 60(B3) 545- 60= 485(A4) 60 A3A4 485(A4) OR 70(A3) 485

Step 2. IDENTIFY VOLUMES, in vph	Step 5. ASSIGN LANE VOLUMES, in vph	Step 7. SUM OF CRITICAL VOLUMES
Approach 3 3: LT= 10 2:RT= 170 TH= 10 TH= 160 RT= 100 v LT= 10 <--Approach 2 Approach 1--> 1:LT= 510 ^ 4: RT= 20 TH= 150 TH=1090 RT= 20 LT= 120 Approach 4	1 0 0 5 5 5 6 < v v v > 281 ^ 230 ^ 75 -> 75 -> 20 -v << ^ ^ ^ > 5 5 6 5 4 4 2 6 4 5 5 0	281(B1B2)+119(A2)+66(B4B3)+485(A4) = 951 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) B Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:

Step 3. IDENTIFY PHASING	Step 6a. CRITICAL VOLUMES, in vph (two phase signal)	COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >	Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4	Exclusive right turns reduced 30 % V/C Ratio = .69

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND VINEYARD RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP					
Approach 3: VINEYARD RD 1 2 2 ^ R L N GERBER RD R T T T L T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--^> <--TH 2 2 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T GERBER RD H H 2 2 1 Approach 4: VINEYARD RD		Approach : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Prob- Critical Carryover Critical able Volume to next Volume Phase in vph phase in vph B2B1 61(B1) 83- 61= 22(B2) 61 A1B2 22(B2) 91- 22= 69(A1) 22 A1A2 69(A1) OR 65(A2) 69 B4B3 6(B4) 33- 6= 27(B3) 6 A4B3 27(B3) 25- 27= 0(A4) 27 A3A4 385(A3) OR 0(A4) 385					
Step 2. IDENTIFY VOLUMES, in vph Approach 3 3: LT= 10 2:RT= 50 TH= 770 TH= 130 RT= 60 v LT= 110 <--Approach 2 Approach 1--> 1:LT= 150 ^ 4: RT= 10 TH= 150 TH= 50 RT= 130 LT= 60 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 3 3 6 8 8 0 5 5 5 6 < v v > 83 -^ 68 -^ 75 -> 75 -> << ^ ^ > 130 -v 3 2 2 2 1 3 7 5 5 0				Step 7. SUM OF CRITICAL VOLUMES 83(B1B2)+69(A1)+33(B4B3)+385(A3) = 570 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) A					
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .41					

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND EXCELSIOR RD

Design Hour: AM

Problem Statement: CUMULATIVE PLUS PROJECT

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:EXCELSIOR RD		-----Approach-----				Possible Volume Adjusted			
1 1 1 ^		: -1- -2- -3- -4-				Critical Volume Critical			
R L N		a.No. of change				Prob- able Phase			
R T T T L		: 0 0 0 0				Critical Volume to next phase in vph			
T H H H T		: 0 0 0 0				Volume Carryover to next phase			
-----		: 0 0 0 0				Adjusted Critical Volume in vph			
Approach 1 << >> ^--RT 1		c.G/C ratio				B2B1 10(B1) 250- 10= 240(B2) 10			
1 LT--^ v v v <^--RTH		: 0 0 0 0				A1B2 240(B2) 10- 240= 0(A1) 240			
LTH-->		: 0 0 0 0				A1A2 10(A2) OR 0(A1) 10			
1 TH-->		: 0 0 0 0				B4B3 10(B4) 10- 10= 0(B3) 10			
RTH--v		: 0 0 0 0				A4B3 0(B3) 650- 0= 650(A4) 0			
1 RT--v << >> Approach 2		e.LT capacity on green (vph)				A3A4 650(A4) OR 70(A3) 650			
L L T R R		f.LT capacity in vph (b+e)							
T T H T T GERBER RD		: 0 0 0 0							
H H		g.Left turn volume in vph							
1 1 1		: 0 0 0 0							
Approach 4:EXCELSIOR RD		h.Is volume > cap. (g>f) ?							

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		5 7 1		250(B1B2)+10(A2)+10(B4B3)+650(A4)	
3: LT= 10		0 0 0		= 920 vph	
TH= 70				-----	
RT= 50		< v >		Step 8. INTERSECTION LEVEL OF SERVICE	
-----		-----		(compare step 7 with table 6)	
<--Approach 2		-----		B	
Approach 1-->		-----		-----	
1:LT= 250		250 --^		Step 9. RECALCULATE	
TH= 10		10 -->		Geometric Change:	
RT= 10		10 -v		Signal Change:	
Approach 4		-----		Volume Change:	
4: RT= 10		6			
TH= 650		1 5 1			
LT= 10		0 0 0			

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1		Approach 3		
--^ AND <-- A1B2 AND		-----		
--> OR v-- /OR A2B1		-----		
--> <-- A1A2		-----		
< B4B3		Approach 1		See Step 6b.
>		-----		
AND < ^ A3B4 AND		-----		Approach 2
v > OR /OR A4B3		-----		
^ A3A4		-----		
v		-----		
A1 --> A3 B1 v-- B3 <		-----		Exclusive right turns reduced 30 % V/C Ratio = .67
A2 <-- A4 B2 --^ B4 >		Approach 4		

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: GERBER RD AND EXCELSIOR RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:EXCELSIOR RD 1 1 1 ^ R L N R T T T L T H H H T Approach 1 << >> ^--RT 1 1 LT--^ v v v <^--RTH LTH-^> <--TH 1 1 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R GERBER RD T T H T T H H 1 1 1 Approach 4:EXCELSIOR RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 10(B1) 50- 10= 40(B2) 10 A1B2 40(B2) 10- 40= 0(A1) 40 A1A2 10(A2) OR 0(A1) 10 B4B3 0(B3) 10- 0= 10(B4) 0 A3B4 10(B4) 600- 10= 590(A3) 10 A3A4 590(A3) OR 200(A4) 590			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 10 Approach 3 2:RT= 10 TH= 600 TH= 10 RT= 240 v LT= 10 <--Approach 2 Approach 1--> 1:LT= 50 ^ 4: RT= 10 TH= 10 TH= 200 RT= 10 Approach 4 LT= 0		Step 5. ASSIGN LANE VOLUMES, in vph 2 6 4 0 1 0 0 0 < v > 50 -^ < ^ > 10 -> 10 -v 2 0 1 0 0 0				Step 7. SUM OF CRITICAL VOLUMES 50(B1B2)+10(A2)+10(B3B4)+590(A3) = 660 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) A			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4 COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .48							

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: CALVINE ROAD AND ELK GROVE-FLORIN RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: ELK GROVE-FLO 1 3 2 ^ R L N R T T T L T H H H T Approach 1 << >> ^-RT 1 2 LT--^ v v v <^-RTH LTH-^> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T H H 2 3 1 Approach 4: ELK GROVE-FLO		Approach : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- able Phase B2B1 226(B1) 248- 226= 22(B2) 226 A1B2 22(B2) 263- 22= 241(A1) 22 A1A2 417(A2) OR 241(A1) 417 B4B3 44(B4) 231- 44= 187(B3) 44 A4B3 187(B3) 530- 187= 343(A4) 187 A3A4 350(A3) OR 343(A4) 350			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
3: LT= 80 TH= 890 RT= 500 Approach 3 2: RT= 100 TH=1250 LT= 410 <--Approach 2 Approach 1-->		5 2 2 2 0 9 9 9 3 4 0 7 7 7 6 4 < v v v > 248 -^ 203 -^ 263 -> 263 -> 263 -> 220 -v 2 1 5 5 5 4 3 8 3 3 3 9 1 9 0 0 0 0		248(B1B2)+417(A2)+231(B4B3)+350(A3) = 1246 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) E Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	
1: LT= 450 TH= 790 RT= 220 Approach 4 4: RT= 490 TH=1590 LT= 420					

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: CALVINE ROAD AND ELK GROVE-FLORIN RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:ELK GROVE-FLO 1 3 2 ^ R L N CALVINE ROAD R T T T L T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH-> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R CALVINE ROAD T T H T T H H 2 3 1 Approach 4:ELK GROVE-FLO		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : D 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- Possible Volume Adjusted able Critical Carryover Critical Phase Volume to next Volume in vph phase in vph B2B1 297(B1) 336- 297= 39(B2) 297 A1B2 39(B2) 430- 39= 391(A1) 39 A1A2 391(A1) OR 347(A2) 391 B4B3 50(B4) 165- 50= 115(B3) 50 A4B3 115(B3) 413- 115= 298(A4) 115 A3A4 513(A3) OR 298(A4) 513			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 90 Approach 3 2:RT= 90 TH=1540 TH=1040 RT= 480 v LT= 540 <--Approach 2 Approach 1--> 1:LT= 610 ^ 4: RT= 440 TH=1290 TH=1240 RT= 400 LT= 300 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 4 5 5 5 8 1 1 1 4 5 0 3 3 3 1 0 < v v v v > 336 -^ 275 -^ 430 -> 430 -> 430 -> 400 -v << ^ ^ ^ ^ > 1 1 4 4 4 4 6 3 1 1 1 4 5 5 3 3 3 0				Step 7. SUM OF CRITICAL VOLUMES 336(B1B2)+391(A1)+165(B4B3)+513(A3) = 1405 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4 COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.02							

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: WATERMAN AND CALVINE

Design Hour: AM

Problem Statement: CUMULATIVE PLUS PROJECT

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: WATERMAN RD 1 2 2 ^ R L N CALVINE RD R T T T L T H H H T Approach 1 << >> ^-RT 1 2 LT--^ v v v <^-RTH LTH-^> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T CALVINE RD H H 2 2 1 Approach 4: WATERMAN RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- able Phase Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph B2B1 94(B1) 143- 94= 49(B2) 94 A1B2 49(B2) 340- 49= 291(A1) 49 A1A2 400(A2) OR 291(A1) 400 B4B3 55(B3) 72- 55= 17(B4) 55 A3B4 17(B4) 220- 17= 203(A3) 17 A3A4 495(A4) OR 203(A3) 495			
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
3: LT= 130 Approach 3 TH= 440 2:RT= 100 RT= 280 v TH=1200 LT= 170 <--Approach 2 Approach 1--> 1:LT= 260 ^ 4: RT= 270 TH=1020 TH= 990 RT= 120 LT= 100 Approach 4		2 2 2 8 2 2 5 7 0 0 0 9 2 < v v >> 143 -^ 117 -^ 340 -> 340 -> 340 -> << ^ ^ ^ > 120 -v 4 4 2 5 4 9 9 7 5 5 5 5 0				143(B1B2)+400(A2)+72(B3B4)+495(A4) = 1110 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) D Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				Exclusive right turns reduced 30 % V/C Ratio = .81			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: WATERMAN AND CALVINE
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: WATERMAN RD 1 2 2 ^ R L N R T T T L T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--> <--TH 3 3 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T CALVINE RD H H 2 2 1 Approach 4: WATERMAN RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Volume to next phase in vph Prob- able Phase B2B1 138(B2) 143- 138= 5(B1) 138 A2B1 5(B1) 430- 5= 425(A2) 5 A1A2 453(A1) OR 425(A2) 453 B4B3 61(B4) 94- 61= 33(B3) 61 A4B3 33(B3) 330- 33= 297(A4) 33 A3A4 485(A3) OR 297(A4) 485			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 110 Approach 3 2:RT= 160 TH= 970 TH=1290 RT= 230 v LT= 260 <--Approach 2 Approach 1--> 1:LT= 250 ^ 4: RT= 220 TH=1360 TH= 660 RT= 110 LT= 170 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 2 4 4 3 8 8 5 6 0 5 5 0 1 < v v v > 138 -^ 113 -^ 453 -> 453 -> 453 -> 110 -v << ^ ^ ^ > 3 3 2 9 7 3 3 2 4 7 0 0 0				Step 7. SUM OF CRITICAL VOLUMES 143(B2B1)+453(A1)+94(B4B3)+485(A3) = 1175 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) D			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .85			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: CALVINE RD AND BRADSHAW RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW 1 3 2 ^ R L N R T T T L T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--> <--TH 3 3 TH--> <v--LTH RTH--v ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R T T H T T CALVINE RD H H 2 3 1 Approach 4: BRADSHAW		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph B2B1 28(B1) 413- 28= 385(B2) 28 A1B2 385(B2) 153- 385= 0(A1) 385 A1A2 183(A2) OR 0(A1) 183 B4B3 22(B3) 33- 22= 11(B4) 22 A3B4 11(B4) 371- 11= 360(A3) 11 A3A4 430(A4) OR 360(A3) 430			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
3: LT= 60 Approach 3 2:RT= 210 TH= 660 TH= 550 RT= 530 v LT= 50 <--Approach 2		5 2 2 2 3 2 2 2 2 3 0 0 0 0 7 3 < v v v >		^- 210 <- 183 <- 183 <- 183 v- 23 v- 28	
Approach 1--> 1:LT= 750 ^ 4: RT= 30 TH= 460 TH=1290 RT= 40 LT= 40 Approach 4		413 -^ 338 -^ 153 -> 153 -> 153 -> 40 -v << ^ ^ ^ ^ > 4 4 4 2 1 3 3 3 3 2 8 0 0 0 0		413(B1B2)+183(A2)+33(B3B4)+430(A4) = 1059 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) C	
				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				Exclusive right turns reduced 30 % V/C Ratio = .77

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: CALVINE RD AND BRADSHAW RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW 1 3 2 ^ R L N CALVINE RD R T T L T H H H T ----- Approach 1 < < > > ^--RT 1 2 LT--^ v v v <^--RTH LTH-^> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 1 RT--v < < > > Approach 2 L L T R R ----- T T H T T CALVINE RD H H 2 3 1 Approach 4:BRADSHAW		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume Volume Volume in vph to next phase in vph B2B1 17(B1) 369- 17= 352(B2) 17 A1B2 352(B2) 193- 352= 0(A1) 352 A1A2 147(A2) OR 0(A1) 147 B4B3 17(B3) 110- 17= 93(B4) 17 A3B4 93(B4) 630- 93= 537(A3) 93 A3A4 537(A3) OR 300(A4) 537			
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
Approach 3 3: LT= 200 2:RT= 140 TH=1260 TH= 440 RT= 900 v LT= 30 <--Approach 2 Approach 1--> 1:LT= 670 ^ 4: RT= 30 TH= 580 TH= 900 RT= 50 LT= 30 Approach 4		9 4 4 4 1 0 2 2 2 9 1 0 0 0 0 0 0 < v v v v > 369 ^- 302 ^- 193 -> 193 -> < < ^ ^ ^ ^ > 193 -> 50 -v 3 3 3 1 1 0 0 0 3 7 4 0 0 0 0				369(B1B2)+147(A2)+110(B3B4)+537(A3) = 1163 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) D Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				Exclusive right turns reduced 30 % V/C Ratio = .85			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: CALVINE RD AND VINEYARD RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: VINEYARD 1 2 ^ R L N CALVINE RD R T T T L T H H H T Approach 1 << >> ^-RT 1 2 LT--^ v v v <^--RTH LTH-^> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT RT--v << >> Approach 2 L L T R R T T H T T CALVINE RD H H Approach 4: VINEYARD		-----Approach----- : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 0 0 d.Opposing volume in vph : 0 750 0 220 e.LT capacity on green (vph) : 0 0 0 0 f.LT capacity in vph (b+e) : 0 0 0 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? : NO NO				Prob- able Phase Possible Critical Volume in vph Volume to next phase Adjusted Critical Volume in vph A1B2 391(B2) 250- 391= 0(A1) 391 A1A2 120(A2) OR 0(A1) 120 A3B4 154(A3) OR 11(B4) 154			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
3: LT= 20 TH= 0 RT= 220 Approach 3 2:RT= 130 TH= 360 LT= 0 <--Approach 2 391 -^ 320 -^ 250 -> 250 -> 250 -> Approach 1--> 1:LT= 710 TH= 750 RT= 0 Approach 4		2 2 1 0 9 1 < > >		511(B2A2)+154(A3)+0()+0() = 665 vph ^- 130 <- 120 <- 120 <- 120	
				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- A -----	
				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ A1B2 --> --> <-- A1A2 A3B4 v >		Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4		
1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: CALVINE RD AND VINEYARD RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: VINEYARD 1 R L ^ 2 T T T L N T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--> <--TH 3 3 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT RT--v << >> Approach 2 L L T R R T T H T T CALVINE RD H H Approach 4: VINEYARD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 430 0 880 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : NO NO (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph A1B2 132(B2) 143- 132= 11(A1) 132 A1A2 173(A2) OR 11(A1) 173 A3B4 616(A3) OR 83(B4) 616			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 150 Approach 3 2:RT= 50 TH= 0 TH= 520 RT= 880 v LT= 0 <--Approach 2 Approach 1--> 1:LT= 240 ^ 4: RT= 0 TH= 430 TH= 0 RT= 0 LT= 0 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 8 8 6 8 0 8 3 < > > ^- 50 <- 173 <- 173 <- 173				Step 7. SUM OF CRITICAL VOLUMES 305(B2A2)+616(A3)+0()+0() = 921 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) B			
Step 3. IDENTIFY PHASING --^ A1B2 --> A1A2 A3B4 v >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .65			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: CALVINE RD AND EXCELSIOR RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: AM

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:EXCELSIOR 1 1 1 ^ R L N R T T T L T H H H T Approach 1 << >> ^-RT 1 LT--^ v v v <^-RTH 1 LTH-^> <--TH 2 2 TH--> <v-LTH 1 RTH-v ^ ^ ^ v--LT 1 RT--v << >> Approach 2 L L T R R T T H T T CALVINE RD H H 1 1 1 Approach 4:EXCELSIOR		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph B2B1 10(B1) 130- 10= 120(B2) 10 A1B2 120(B2) 217- 120= 97(A1) 120 A1A2 97(A1) OR 80(A2) 97 B4B3 10(B4) 140- 10= 130(B3) 10 A4B3 130(B3) 440- 130= 310(A4) 130 A3A4 310(A4) OR 50(A3) 310			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 10 Approach 3 TH= 50 2:RT= 30 RT= 20 v TH= 210 LT= 10 <--Approach 2 Approach 1--> 1:LT= 130 ^ 4: RT= 80 TH= 610 TH= 440 RT= 40 LT= 140 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 2 5 1 0 0 0 < v > 130 -^ 217 -> 217 -> 177 +> 40 +v < ^ > 1 4 4 4 8 0 0 0				Step 7. SUM OF CRITICAL VOLUMES 130(B1B2)+97(A1)+140(B4B3)+310(A4) = 677 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) A Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .49			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: CALVINE RD AND EXCELSIOR RD
Problem Statement: CUMULATIVE PLUS PROJECT

Design Hour: PM

Step 1. IDENTIFY LANE GEOMETRY	Step 4. LEFT TURN CHECK	Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP
<p>Approach 3:EXCELSIOR</p> <p>1 1 1 ^</p> <p>R L N</p> <p>R T T T L</p> <p>T H H H T</p> <p>Approach 1 < < > > ^--RT</p> <p>1 LT--^ v v v <^--RTH 1</p> <p>LTH--^> <--TH 2</p> <p>2 TH--> <v--LTH</p> <p>1 RTH--v> ^ ^ ^ v--LT 1</p> <p>RT--v < < > > Approach 2</p> <p>L L T R R</p> <p>T T H T T CALVINE RD</p> <p>H H</p> <p>1 1 1</p> <p>Approach 4:EXCELSIOR</p>	<p>-----Approach-----</p> <p>: -1- -2- -3- -4-</p> <p>a.No. of change : 0 0 0 0</p> <p>intervals/hour :</p> <p>b.LT capacity on : 0 0 0 0</p> <p>change (vph) :</p> <p>c.G/C ratio : 0 0 0 0</p> <p>d.Opposing volume : 0 0 0 0</p> <p>in vph :</p> <p>e.LT capacity on : 0 0 0 0</p> <p>green (vph) :</p> <p>f.LT capacity in : 0 0 0 0</p> <p>vph (b+e) :</p> <p>g.Left turn volume : 0 0 0 0</p> <p>in vph :</p> <p>h.Is volume > cap. : :</p> <p>(g>f) ? :</p>	<p>Possible Volume Adjusted</p> <p>Critical Volume Critical</p> <p>Phase in vph to next phase in vph</p> <p>B2B1 50(B1) 100- 50= 50(B2) 50</p> <p>A1B2 50(B2) 160- 50= 110(A1) 50</p> <p>A1A2 110(A1) OR 100(A2) 110</p> <p>B4B3 30(B4) 60- 30= 30(B3) 30</p> <p>A4B3 30(B3) 80- 30= 50(A4) 30</p> <p>A3A4 330(A3) OR 50(A4) 330</p>
<p>Step 2. IDENTIFY VOLUMES, in vph</p> <p>3: LT= 30</p> <p>TH= 330</p> <p>RT= 200</p> <p>Approach 3 </p> <p>2:RT= 20</p> <p>TH= 280</p> <p>LT= 50</p> <p>Approach 2</p> <p>Approach 1--></p> <p>1:LT= 100</p> <p>TH= 210</p> <p>RT= 160</p> <p>Approach 4 </p> <p>4: RT= 10</p> <p>TH= 80</p> <p>LT= 60</p>	<p>Step 5. ASSIGN LANE VOLUMES, in vph</p> <p>2 3</p> <p>0 3 3</p> <p>0 0 0</p> <p> </p> <p>< v ></p> <p>100 -^</p> <p>105 -></p> <p>105 -></p> <p>0 +></p> <p>160 +v</p> <p>6 8 1</p> <p>0 0 0</p>	<p>Step 7. SUM OF CRITICAL VOLUMES</p> <p>100(B1B2)+110(A1)+60(B4B3)+330(A3)</p> <p>= 600 vph</p> <p>Step 8. INTERSECTION LEVEL OF SERVICE</p> <p>(compare step 7 with table 6)</p> <p>A</p> <p>Step 9. RECALCULATE</p> <p>Geometric Change:</p> <p>Signal Change:</p> <p>Volume Change:</p>
<p>Step 3. IDENTIFY PHASING</p> <p>--^ v-- B2B1</p> <p>--^ AND <-- A1B2 AND</p> <p>--> OR v-- /OR A2B1</p> <p>--> <-- A1A2</p> <p> < B4B3</p> <p>> </p> <p> AND < ^ A3B4 AND</p> <p>v > OR /OR A4B3</p> <p> ^ A3A4</p> <p>v </p> <p>A1 --> A3 B1 v-- B3 < </p> <p>v ^ </p> <p>A2 <-- A4 B2 --^ B4 > </p>	<p>Step 6a. CRITICAL VOLUMES, in vph (two phase signal)</p> <p>Approach 3</p> <p>Approach 1</p> <p>See Step 6b.</p> <p>Approach 2</p> <p>Approach 4</p>	<p>COMMENTS</p> <p>Exclusive right turns reduced 30 %</p> <p>V/C Ratio = .44</p>

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND BRADSHAW RD

Design Hour: AM

Problem Statement: CUMULATIVE PLUS PROJECT (LIMIT GATED)

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD		-----Approach-----				Possible Volume Adjusted			
1 3 2 ^		: -1- -2- -3- -4-				Critical Volume Critical			
R L N		a.No. of change : 0 0 0 0				Prob- able Phase			
R T T T L		intervals/hour : :				Critical Volume in vph			
T H H H T		b.LT capacity on : 0 0 0 0				Volume to next phase			
Approach 1 << >>		change (vph) : :				-----			
2 LT--^ v v v		c.G/C ratio : 0 0 0 0				B2B1 127(B1) 154- 127= 27(B2) 127			
LTH-^>		d.Opposing volume : 0 0 0 0				A1B2 27(B2) 87- 27= 60(A1) 27			
3 TH-->		in vph : :				A1A2 360(A2) OR 60(A1) 360			
RTH-v>		e.LT capacity on : 0 0 0 0				B4B3 28(B4) 371- 28= 343(B3) 28			
2 RT--v << >> Approach 2		green (vph) : :				A4B3 343(B3) 680- 343= 337(A4) 343			
L L T R R		f.LT capacity in : 0 0 0 0				A3A4 411(A3) OR 337(A4) 411			
T T H T T JACKSON HWY		vph (b+e) : :							
H H		g.Left turn volume : 0 0 0 0							
3 3 1		in vph : :							
Approach 4: BRADSHAW RD		h.Is volume > cap. : :							
		(g>f) ? : :							

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		3 4 4 4		154(B1B2)+360(A2)+371(B4B3)+411(A3)	
3: LT= 50		2 1 1 1 2 2		= 1296 vph	
TH=1232		0 1 1 1 3 8			
RT= 320		< v v v >		-----	
				Step 8. INTERSECTION LEVEL OF SERVICE	
				(compare step 7 with table 6)	

				E	

				Step 9. RECALCULATE	
				Geometric Change:	
				Signal Change:	
				Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1		Approach 3		
--^ AND <-- A1B2 AND				
--> OR v-- /OR A2B1				
--> <-- A1A2				
< B4B3		Approach 1		See Step 6b.
>				
AND < ^ A3B4 AND		Approach 2		
v > OR /OR A4B3				
^ A3A4				
v				
A1 --> A3 B1 v-- B3 <		Approach 4		Exclusive right turns reduced 30 % V/C Ratio = .948
A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND BRADSHAW RD

Design Hour: PM

Problem Statement: CUMULATIVE PLUS PROJECT (MITIGATED)

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW RD 1 3 2 ^ R L N JACKSON HWY R T T T L T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH-^> <--TH 3 3 TH--> <v--LTH RTH-v> ^ ^ ^ v--LT 2 2 RT--v << >> Approach 2 L L T R R T T H T T JACKSON HWY H H 3 3 1 Approach 4:BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Prob- Critical Volume Critical able Volume to next Volume Phase in vph phase in vph B2B1 17(B1) 149- 17= 132(B2) 17 A1B2 132(B2) 420- 132= 288(A1) 132 A1A2 288(A1) OR 93(A2) 288 B4B3 138(B4) 190- 138= 52(B3) 138 A4B3 52(B3) 513- 52= 461(A4) 52 A3A4 644(A3) OR 461(A4) 644			
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
Approach 3 3: LT= 250 2:RT= 110 TH=1933 TH= 280 RT= 250 v LT= 30 <--Approach 2 Approach 1--> 1:LT= 270 ^ 4: RT= 230 TH= 840 TH=1540 RT=1200 LT= 520 Approach 4		2 6 6 6 1 1 5 4 4 4 1 3 0 4 4 4 3 8 < v v v v > 149 -^ 122 -^ 280 -> 280 -> 280 -> 600 -v 600 -v << < ^ ^ ^ > 1 1 1 5 5 5 2 9 7 5 1 1 1 3 0 3 6 3 3 3 0				149(B1B2)+288(A1)+190(B4B3)+644(A3) = 1271 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) E			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				Step 9. RECALCULATE			
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Approach 4				Exclusive right turns reduced 30 % V/C Ratio = .92			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND EXCELSIOR RD

Design Hour: AM

Problem Statement: CUMULATIVE PLUS PROJECT (MITIGATED)

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: EXCELSIOR RD		-----Approach-----				Possible Volume Adjusted			
1 2 1 ^		: -1- -2- -3- -4-				Critical Volume Critical			
R L N		a.No. of change : 0 0 0 0				Prob- able Phase			
R T T T L		: intervals/hour : 0 0 0 0				Critical Volume in vph			
T H H H T		: b.LT capacity on : 0 0 0 0				Volume to next phase			
Approach 1 << >> ^--RT 1		: change (vph) : 0 0 0 0				B2B1 40(B2) 160- 40= 120(B1) 40			
1 LT--^ v v v <^--RTH		: c.G/C ratio : 0 0 0 0				A2B1 120(B1) 575- 120= 455(A2) 120			
LTH-->		: d.Opposing volume : 0 0 0 0				A1A2 455(A2) OR 75(A1) 455			
2 TH-->		: in vph : 0 0 0 0				B4B3 20(B4) 140- 20= 120(B3) 20			
RTH--v ^ ^ ^ v--LT 1		: e.LT capacity on : 0 0 0 0				A4B3 120(B3) 675- 120= 555(A4) 120			
1 RT--v << >> Approach 2		: f.LT capacity in : 0 0 0 0				A3A4 555(A4) OR 115(A3) 555			
L L T R R		: vph (b+e) : 0 0 0 0							
T T H T T JACKSON HWY		: g.Left turn volume : 0 0 0 0							
H H		: in vph : 0 0 0 0							
1 2 1		: h.Is volume > cap. : 0 0 0 0							
Approach 4: EXCELSIOR RD		: (g>f) ? : 0 0 0 0							

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		1 1		160(B2B1)+455(A2)+140(B4B3)+555(A4)	
3: LT= 20		9 1 1 2		= 1310 vph	
TH= 230		0 5 5 0			
RT= 90		< v v >		Step 8. INTERSECTION LEVEL OF SERVICE	
v		^-- 80		(compare step 7 with table 6)	
<--Approach 2		<- 575		-----	
		v- 160		E	
Approach 1-->		40 --^		-----	
		75 -->		Step 9. RECALCULATE	
1:LT= 40		75 -->		Geometric Change:	
TH= 150		30 -v		Signal Change:	
RT= 30		-----		Volume Change:	
Approach 4		1 6 6 1			
4: RT= 160		4 7 7 6			
TH=1350		0 5 5 0			
LT= 140		-----			

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1		Approach 3		
--^ AND <-- A1B2 AND				
--> OR v-- /OR A2B1				
--> <-- A1A2				
< B4B3		Approach 1		See Step 6b.
>				
AND <^ A3B4 AND		Approach 2		
v > OR /OR A4B3				
^ A3A4				
v				
A1 --> A3 B1 v-- B3 <		Approach 4		Exclusive right turns reduced 30 % V/C Ratio = .954
A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: JACKSON HWY AND EXCELSIOR RD

Design Hour: PM

Problem Statement: CUMULATIVE PLUS PROJECT (MITIGATED)

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: EXCELSIOR RD		Approach				Possible Volume Adjusted			
1 2 1 ^		: -1- -2- -3- -4-				Critical Volume to next phase in vph			
R L N		a.No. of change intervals/hour				Prob- Critical Volume in vph			
JACKSON HWY R T T T L		: 0 0 0 0				B2B1 80(B2) 180- 80= 100(B1) 80			
T H H H T		b.LT capacity on change (vph)				A2B1 100(B1) 170- 100= 70(A2) 100			
Approach 1 << >> ^--RT 1		: 0 0 0 0				A1A2 555(A1) OR 70(A2) 555			
1 LT--^ v v v <^--RTH		c.G/C ratio				B4B3 30(B4) 30- 30= 0(B3) 30			
LTH--> <--TH 2		: 0 0 0 0				A4B3 0(B3) 190- 0= 190(A4) 0			
2 TH--> <v--LTH		d.Opposing volume in vph				A3A4 670(A3) OR 190(A4) 670			
RTH--v> ^ ^ ^ v--LT 1		e.LT capacity on green (vph)							
1 RT--v << >> Approach 2		: 0 0 0 0							
L L T R R		f.LT capacity in vph (b+e)							
T T H T T JACKSON HWY		: 0 0 0 0							
H H		g.Left turn volume in vph							
1 2 1		: 0 0 0 0							
Approach 4: EXCELSIOR RD		h.Is volume > cap. (g>f) ?							
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
3: LT= 30 Approach 3		6 6				180(B2B1)+555(A1)+30(B4B3)+670(A3)			
TH=1340		3 7 7 3				= 1435 vph			
RT= 30 v		0 0 0 0							
		<							
		< v v >							
		^-- 30							
		<-- 170							
		<-- 170							
		v- 180							
						Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)			
						F			
Approach 1-->		80 ^							
		555 ->							
		< ^ ^ ^							
1: LT= 80						Step 9. RECALCULATE			
TH=1110						Geometric Change:			
RT= 130						Signal Change:			
Approach 4						Volume Change:			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
--^ v-- B2B1		Approach 3							
--^ AND <-- A1B2 AND									
--> OR v-- /OR A2B1									
--> <-- A1A2									
< B4B3		Approach 1							
>		See Step 6b.							
AND < ^ A3B4 AND		Approach 2							
v > OR /OR A4B3									
^ A3A4									
v									
A1 --> A3 B1 v-- B3 <		Approach 4				Exclusive right turns reduced 30 %			
v ^						V/C Ratio = 1.04			
A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELDER CREEK RD AND BRADSHAW RD

Design Hour: AM

Problem Statement: CUMULATIVE PLUS PROJECT (MITIGATED)

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW RD 1 3 2 ^ R L N ELDER CREEK R R T T T L T H H H T Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--> <--TH 2 2 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 2 2 RT--v << >> Approach 2 L L T R R ELDER CREEK R T T H T T H H 2 3 1 Approach 4:BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Prob- Possible Volume Adjusted able Critical Carryover Critical Phase in vph to next Volume ----- B2B1 6(B1) 171- 6= 165(B2) 6 A1B2 165(B2) 120- 165= 0(A1) 165 A1A2 165(A2) OR 0(A1) 165 B4B3 6(B4) 622- 6= 616(B3) 6 A4B3 616(B3) 910- 616= 294(A4) 616 A3A4 310(A3) OR 294(A4) 310			
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
3: LT= 10 Approach 3 2:RT= 130 TH= 930 TH= 330 RT= 230 v LT= 10 <--Approach 2 Approach 1--> 1:LT= 310 ^ 4: RT= 110 TH= 240 TH=2730 RT= 30 LT=1130 Approach 4		2 3 3 3 3 1 1 1 0 0 0 0 5 6 < v v v v > ^- 130 <- 165 <- 165 v- 5 v- 6				171(B1B2)+165(A2)+622(B4B3)+310(A3) = 1268 vph			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)			
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				E			
Step 9. RECALCULATE		Step 9. RECALCULATE				COMMENTS			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Geometric Change: Signal Change: Volume Change:				Exclusive right turns reduced 30 % V/C Ratio = .92			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELDER CREEK RD AND BRADSHAW RD

Design Hour: PM

Problem Statement: CUMULATIVE PLUS PROJECT (MITIGATED)

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3:BRADSHAW RD 1 3 2 ^ R L N R T T T L T H H H T		-----Approach----- : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 0 0 d.Opposing volume in vph : 0 0 0 0 e.LT capacity on green (vph) : 0 0 0 0 f.LT capacity in vph (b+e) : 0 0 0 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? :				Possible Volume Adjusted Prob- Critical Volume Critical able Volume to next Volume Phase in vph phase in vph B2B1 50(B1) 149- 50= 99(B2) 50 A1B2 99(B2) 357- 99= 258(A1) 99 A1A2 258(A1) OR 165(A2) 258 B4B3 11(B4) 55- 11= 44(B3) 11 A4B3 44(B3) 530- 44= 486(A4) 44 A3A4 970(A3) OR 486(A4) 970			
ELDER CREEK R Approach 1 << >> ^--RT 1 2 LT--^ v v v <^--RTH LTH--^> <--TH 2 2 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 2 2 RT--v << >> Approach 2 L L T R R T T H T T ELDER CREEK R H H 2 3 1 Approach 4:BRADSHAW RD									
Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph				Step 7. SUM OF CRITICAL VOLUMES			
Approach 3 3: LT= 20 2:RT= 10 TH=2910 TH= 330 RT= 280 v LT= 90 <--Approach 2 149 -^ 122 -^ 180 -> 180 -> Approach 1--> 1:LT= 270 ^ 4: RT= 30 TH= 360 TH=1590 RT=1020 LT= 100 Approach 4		2 9 9 9 8 7 7 7 1 0 0 0 0 9 1 < v v v v > 510 -v 510 -v 5 5 5 5 4 3 3 3 3 5 5 0 0 0 0				149(B1B2)+258(A1)+55(B4B3)+970(A3) = 1432 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F			
Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)				COMMENTS			
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				Exclusive right turns reduced 30 % V/C Ratio = 1.04			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELDER CREEK RD AND EXCELSIOR RD

Design Hour: AM

Problem Statement: CUMULATIVE PLUS PROJECT (MITIGATED)

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: EXCELSIOR RD 1 2 ^ R L N ELDER CREEK R R T T T L T H H H T Approach 1 << >> ^--RT 1 LT--^ v v v <^--RTH LTH--> <--TH TH--> <v--LTH RTH--v ^ ^ ^ v--LT 1 RT--v << >> Approach 2 L L T R R T T H T T ELDER CREEK R H H 1 2 Approach 4:		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 1 0 d.Opposing volume : 0 10 1280 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (bte) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : NO NO (g>f) ? :				Prob- able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph A4B3 160(B3) 640- 160= 480(A4) 160 A3A4 480(A4) OR 210(A3) 480 A1B2 360(B2) OR 7(A1) 360			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
3: LT= 0 TH= 100 RT= 300 Approach 3 v 2: RT= 0 TH= 0 LT= 0 <--Approach 2		3 0 5 5 0 0 0 < v v		640(B3A4)+360(B2)+0()+0() = 1000 vph	
Approach 1--> 1: LT= 360 TH= 0 RT= 10 Approach 4		4: RT= 0 TH=1280 LT= 160 360 -^ 10 -v < ^ ^ 1 6 6 6 4 4 0 0 0		Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) B	
				Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
< ^ A4B3 ^ A3A4 v --^ A1B2 -->		Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				Exclusive right turns reduced 30 % V/C Ratio = .715

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: ELDER CREEK RD AND EXCELSIOR RD

Design Hour: PM

Problem Statement: CUMULATIVE PLUS PROJECT (MITIGATED)

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: EXCELSIOR RD 1 2 ^ R L N ELDER CREEK R R T T T L T H H H T Approach 1 << >> ^--RT 1 LT--^ v v v <^--RTH LTH--> <--TH TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 1 RT--v << >> Approach 2 L L T R R T T H T T ELDER CREEK R H H 1 2 Approach 4:		Approach : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 1 0 d.Opposing volume in vph : 0 420 260 0 e.LT capacity on green (vph) : 0 0 940 0 f.LT capacity in vph (b+e) : 0 0 940 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? : NO NO				Possible Volume Adjusted Prob- Critical Carryover Critical able Volume to next Volume Phase in vph phase in vph A4B3 20(B3) 130- 20= 110(A4) 20 A3A4 615(A3) OR 110(A4) 615 A1B2 350(B2) OR 294(A1) 350			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 0 TH=1230 RT= 30 Approach 3 2:RT= 0 TH= 0 LT= 0 <--Approach 2 Approach 1--> 1:LT= 350 TH= 0 RT= 420 Approach 4 4: RT= 0 TH= 260 LT= 20		Step 5. ASSIGN LANE VOLUMES, in vph 6 6 3 1 1 0 5 5 < v v 350 -^ 420 -v < ^ ^ 1 1 2 3 3 0 0 0				Step 7. SUM OF CRITICAL VOLUMES 635(B3A3)+350(B2)+0()+0() = 985 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) B			
Step 3. IDENTIFY PHASING < ^ A4B3 ^ A3A4 v --^ A1B2 -->		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = .692			
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >									

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND BRADSHAW RD

Design Hour: AM

Problem Statement: CUMULATIVE PLUS PROJECT (MITIGATED)

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD		Approach				Possible Volume Adjusted			
1 3 2 ^		: -1- -2- -3- -4-				Critical Carryover Critical			
R L N		: 0 0 0 0				Phase in vph to next phase in vph			
FLORIN RD		a.No. of change intervals/hour				Prob- able Critical Volume			
R T T T L		: 0 0 0 0				B2B1 17(B1) 413- 17= 396(B2) 17			
T H H H T		b.LT capacity on change (vph)				A1B2 396(B2) 63- 396= 0(A1) 396			
Approach 1 << >> ^-RT 2		c.G/C ratio				A1A2 337(A2) OR 0(A1) 337			
2 LT--^ v v v <^-RTH		: 0 0 0 0				B4B3 6(B4) 440- 6= 434(B3) 6			
LTH-> <--TH 3		d.Opposing volume in vph				A4B3 434(B3) 827- 434= 393(A4) 434			
3 TH--> <v-LTH		e.LT capacity on green (vph)				A3A4 393(A4) OR 237(A3) 393			
RTH-v> ^ ^ ^ v--LT 2		: 0 0 0 0							
1 RT--v << >> Approach 2		f.LT capacity in vph (b+e)							
L L T R R		: 0 0 0 0							
T T H T T FLORIN RD		g.Left turn volume in vph							
H H		: 0 0 0 0							
2 3 1		h.Is volume > cap. (g>f) ?							
Approach 4: BRADSHAW RD									

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		2 2 2 2		413(B1B2)+337(A2)+440(B4B3)+393(A4)	
3: LT= 10		6 3 3 3		= 1583 vph	
TH= 710		0 7 7 7 5 6			
RT= 260					
v		< v v v >			
		^ 360			
		^ 360			
		< 337			
		< 337			
		< 337			
		v 14			
		v 17			
		<< ^ ^ ^ ^ >			
		4 3 8 8 8 4			
		4 6 2 2 2 0			
		0 0 7 7 7 0			
				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)	
				F	
				Step 9. RECALCULATE	
				Geometric Change:	
				Signal Change:	
				Volume Change:	

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1		Approach 3		
--^ AND <-- A1B2 AND				
--> OR v-- /OR A2B1				
--> <-- A1A2				
< B4B3		Approach 1		See Step 6b.
>				
AND < ^ A3B4 AND		Approach 2		
v > OR /OR A4B3				
^ A3A4				
v				
1 --> A3 B1 v-- B3 <		Approach 4		Exclusive right turns reduced 30 % V/C Ratio = 1.15
A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND BRADSHAW RD

Design Hour: PM

Problem Statement: CUMULATIVE PLUS PROJECT (MITIGATED)

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: BRADSHAW RD 1 3 2 ^ R L N FLORIN RD R T T T L T H H H T Approach 1 << >> ^--RT 2 2 LT--^ v v v <^--RTH LTH--> <--TH 3 3 TH--> <v--LTH RTH--v> ^ ^ ^ v--LT 2 1 RT--v << >> Approach 2 L L T R R FLORIN RD T T H T T H H 2 3 1 Approach 4: BRADSHAW RD		-----Approach----- : -1- -2- -3- -4- a.No. of change : 0 0 0 0 intervals/hour : b.LT capacity on : 0 0 0 0 change (vph) : c.G/C ratio : 0 0 0 0 d.Opposing volume : 0 0 0 0 in vph : e.LT capacity on : 0 0 0 0 green (vph) : f.LT capacity in : 0 0 0 0 vph (b+e) : g.Left turn volume : 0 0 0 0 in vph : h.Is volume > cap. : (g>f) ? :				Possible Volume Adjusted Critical Carryover Critical Volume to next Volume in vph phase in vph B2B1 165(B2) 193- 165= 28(B1) 165 A2B1 28(B1) 103- 28= 75(A2) 28 A1A2 476(A1) OR 75(A2) 476 B4B3 72(B3) 385- 72= 313(B4) 72 A3B4 313(B4) 850- 313= 537(A3) 313 A3A4 537(A3) OR 407(A4) 537			
Step 2. IDENTIFY VOLUMES, in vph 3: LT= 700 Approach 3 2:RT= 200 TH=2550 TH= 310 RT= 770 v LT= 350 <--Approach 2 165 -^ 135 -^ 343 -> 343 -> 343 -> 680 -v Approach 1--> 1:LT= 300 ^ 4: RT= 40 TH=1030 TH=1220 RT= 680 LT= 130 Approach 4		Step 5. ASSIGN LANE VOLUMES, in vph 7 8 8 8 3 3 7 5 5 5 1 8 0 0 0 0 5 5 < < v v v v > ^- 100 ^- 100 <- 103 <- 103 <- 103 v- 158 v- 193 << ^ ^ ^ ^ > 4 4 4 7 5 0 0 0 4 2 9 7 7 7 0				Step 7. SUM OF CRITICAL VOLUMES 193(B2B1)+476(A1)+385(B3B4)+537(A3) = 1591 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) F Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:			
Step 3. IDENTIFY PHASING --^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >		Step 6a. CRITICAL VOLUMES, in vph (two phase signal) Approach 3 Approach 1 See Step 6b. Approach 2 Approach 4				COMMENTS Exclusive right turns reduced 30 % V/C Ratio = 1.16			

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND EXCELSIOR RD

Design Hour: AM

Problem Statement: CUMULATIVE PLUS PROJECT (MITIGATED)

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: EXCELSIOR RD 1 1 1 ^ R L N FLORIN RD R T T T L T H H H T Approach 1 << >> ^-RT 1 2 LT--^ v v v <^-RTH LTH--> <--TH 2 2 TH--> <v-LTH RTH-v> ^ ^ ^ v--LT 1 1 RT--v << >> Approach 2 L L T R R T T H T T FLORIN RD H H 1 2 1 Approach 4: EXCELSIOR RD		-----Approach----- : -1- -2- -3- -4- a.No. of change intervals/hour : 0 0 0 0 b.LT capacity on change (vph) : 0 0 0 0 c.G/C ratio : 0 0 0 0 d.Opposing volume in vph : 0 0 0 0 e.LT capacity on green (vph) : 0 0 0 0 f.LT capacity in vph (b+e) : 0 0 0 0 g.Left turn volume in vph : 0 0 0 0 h.Is volume > cap. (g>f) ? :				Prob- able Phase Possible Critical Volume in vph Volume Carryover to next phase Adjusted Critical Volume in vph B2B1 50(B1) 336- 50= 286(B2) 50 A1B2 286(B2) 425- 286= 139(A1) 286 A1A2 290(A2) OR 139(A1) 290 B4B3 10(B4) 30- 10= 20(B3) 10 A4B3 20(B3) 405- 20= 385(A4) 20 A3A4 385(A4) OR 50(A3) 385			

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES			
Approach 3 3: LT= 10 2:RT= 20 TH= 50 TH= 580 RT= 50 v LT= 50 <--Approach 2		----- 5 5 1 0 0 0 < v >		----- ^- 20 <- 290 <- 290 v- 50 336 -^ 275 -^ 425 -> 425 -> < ^ ^ > 10 -v ----- 4 4 1 3 0 0 1 0 5 5 0		336(B1B2)+290(A2)+30(B4B3)+385(A4) = 1041 vph Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6) ----- C ----- Step 9. RECALCULATE Geometric Change: Signal Change: Volume Change:	
Approach 1--> 1:LT= 610 ^ 4: RT= 110 TH= 850 TH= 810 RT= 10 LT= 30 Approach 4							

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1 --^ AND <-- A1B2 AND --> OR v-- /OR A2B1 --> <-- A1A2 < B4B3 > AND < ^ A3B4 AND v > OR /OR A4B3 ^ A3A4 v		Approach 3 ----- Approach 1 See Step 6b. ----- Approach 2 ----- Approach 4		
A1 --> A3 B1 v-- B3 < v ^ A2 <-- A4 B2 --^ B4 >				

Critical Movement Analysis: PLANNING
Calculation Form 1

Intersection: FLORIN RD AND EXCELSIOR RD

Design Hour: PM

Problem Statement: CUMULATIVE PLUS PROJECT (MITIGATED)

Step 1. IDENTIFY LANE GEOMETRY		Step 4. LEFT TURN CHECK				Step 6b. VOLUME ADJUSTMENT FOR MULTIPHASE SIGNAL OVERLAP			
Approach 3: EXCELSIOR RD		Approach				Possible Volume Adjusted			
1 1 1 ^		: -1- -2- -3- -4-				Critical Volume Critical			
R L N		a.No. of change				Prob- able Phase			
R T T T L		: 0 0 0 0				Critical Volume Adjusted			
T H H H T		: 0 0 0 0				in vph			
Approach 1 << >> ^--RT 1		b.LT capacity on change (vph)				B2B1 66(B2) 210- 66= 144(B1) 66			
2 LT--^ v v v <^--RTH		: 0 0 0 0				A2B1 144(B1) 305- 144= 161(A2) 144			
LTH-->		: 0 0 0 0				A1A2 265(A1) OR 161(A2) 265			
2 TH-->		c.G/C ratio				B4B3 10(B4) 20- 10= 10(B3) 10			
RTH--v ^ ^ ^ <v--LTH		: 0 0 0 0				A4B3 10(B3) 70- 10= 60(A4) 10			
1 RT--v << >> v--LT 1		e.LT capacity on green (vph)				A3A4 610(A3) OR 60(A4) 610			
L L T R R		: 0 0 0 0							
T H T T FLORIN RD		f.LT capacity in vph (b+e)							
H H		: 0 0 0 0							
1 2 1		g.Left turn volume in vph							
Approach 4: EXCELSIOR RD		: 0 0 0 0							
		h.Is volume > cap. (g>f) ?							

Step 2. IDENTIFY VOLUMES, in vph		Step 5. ASSIGN LANE VOLUMES, in vph		Step 7. SUM OF CRITICAL VOLUMES	
Approach 3		6 6		210(B2B1)+265(A1)+20(B4B3)+610(A3)	
3: LT= 10		4 1 1		= 1105 vph	
TH= 610		0 0 0			
RT= 640		< v >			
v				Step 8. INTERSECTION LEVEL OF SERVICE (compare step 7 with table 6)	
<--Approach 2				D	
Approach 1-->		66 -^			
		54 -^			
		265 ->		Step 9. RECALCULATE	
1:LT= 120		< ^ ^ >		Geometric Change:	
TH= 530				Signal Change:	
RT= 30		265 ->		Volume Change:	
Approach 4		30 -v			
		2 7 7 5			
		0 0 0 0			

Step 3. IDENTIFY PHASING		Step 6a. CRITICAL VOLUMES, in vph (two phase signal)		COMMENTS
--^ v-- B2B1		Approach 3		
--^ AND <-- A1B2 AND				
--> OR v-- /OR A2B1				Exclusive right turns reduced 30 % V/C Ratio = .8
--> <-- A1A2		Approach 1		
< B4B3		See Step 6b.		
>		Approach 2		
AND < ^ A3B4 AND				
v > OR /OR A4B3				
^ A3A4				
v				
A1 --> A3 B1 v-- B3 <		Approach 4		
A2 <-- A4 B2 --^ B4 >				

APPENDIX F

Trip Generation Tables - Proposed Alternatives

Table F-1 NORTH VINEYARD SPECIFIC PLAN DAILY TRIP GENERATION FOR 10 PERCENT LOW DENSITY ALTERNATIVE											
Land Use	Quantity	Units	Daily Trip Rate	Daily Vehicle Trips	Inter-nalization	Internal Trips	External Trips	Pass-By Factor	Net External Trips	External Person Trips	Transit Trips
Low Density Residential	5,122	D.U.	9.55	48,915	14%	6,848	42,067	0%	42,067	48,377	2,419
Medium Density Residential	0	D.U.	9.45	0	14%	0	0	0%	0	0	0
High Density Residential	637	D.U.	6.47	4,121	14%	577	3,544	0%	3,544	4,076	204
Neighborhood Commercial	49	ksf GLA	varies	6,555	40%	2,622	3,933	20%	3,146	3,618	181
Community Commercial	262	ksf GLA	varies	16,603	30%	4,981	11,622	30%	8,135	9,356	468
Professional/Offices	103	ksf GFA	14.37	1,480	15%	222	1,258	0%	1,258	1,447	72
Elementary School	600	Students	1.09	654	90%	589	65	0%	65	75	4
Neighborhood Park	64	Acres	2.23	143	90%	128	14	0%	14	16	1
Community Park	27	Acres	2.99	81	50%	40	40	0%	40	46	2
			Totals:	78,552	20%	16,007	62,545	N/A	58,271	67,012	3,351

**Table F-2
NORTH VINEYARD SPECIFIC PLAN DAILY TRIP GENERATION
FOR 15 PERCENT LOW DENSITY ALTERNATIVE**

Land Use	Quantity	Units	Daily Trip Rate	Daily Vehicle Trips	Inter-modalization	Internal Trips	External Trips	Pass-By Factor	Net External Trips	External Person Trips	Transit Trips
Low Density Residential	4,747	D.U.	9.55	45,334	14%	6,347	38,987	0%	38,987	44,835	2,242
Medium Density Residential	0	D.U.	9.45	0	14%	0	0	0%	0	0	0
High Density Residential	637	D.U.	6.47	4,121	14%	577	3,544	0%	3,544	4,076	204
Neighborhood Commercial	49	ksf GLA	varies	6,555	40%	2,622	3,933	20%	3,146	3,618	181
Community Commercial	262	ksf GLA	varies	16,603	30%	4,981	11,622	30%	8,135	9,356	468
Professional/Offices	103	ksf GFA	14.37	1,480	15%	222	1,258	0%	1,258	1,447	72
Elementary School	600	Students	1.09	654	90%	589	65	0%	65	75	4
Neighborhood Park	64	Acres	2.23	143	90%	128	14	0%	14	16	1
Community Park	27	Acres	2.99	81	50%	40	40	0%	40	46	2
Totals:				74,971	21%	15,506	59,465	N/A	55,192	63,470	3,174

DEPARTMENT OF ENVIRONMENTAL REVIEW & ASSESSMENT

VOLUME III TECHNICAL STUDY PREPARERS

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